May 8, 2015

STORMWATER REPORT

For

2 WELLS AVENUE Newton, Massachusetts

Prepared for:

TWO WELLS AVENUE LLC C/O BAL MANAGEMENT LLC

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Nitsch Project #10444

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1.0 INTRODUCTION

Nitsch Engineering has prepared this Stormwater Report for review by the City of Newton Engineering Division to support permitting applications for the proposed redevelopment project at 2 Wells Avenue in Newton, Massachusetts. The Project consists of the demolition of a portion of a 1-story existing building, renovation of the remaining portion of the 1-story existing building, and construction of a new 3-story office building. The new 3-story office building will connect to the remaining portion of the 1-story building. Exterior site improvements include installation of building services, enlargement and reconfiguration of the existing parking lots, enhanced landscaping, a new stormwater management system providing water quality treatment and groundwater recharge, and a retaining wall along the northwest site boundary. The stormwater management system has been designed to meet and exceed the requirements of the City of Newton Engineering Division and the Massachusetts Department of Environmental Protection (DEP) Stormwater Management Standards.

2.0 EXISTING CONDITIONS

The site is located at 2 Wells Avenue in Newton, Massachusetts (Figure 1). The site is bounded by Wells Avenue to the south and west, wooded residential areas to the north, and wetlands to the east.

The existing development consists of a 68,740± square-foot commercial building and associated driveways, parking, and building utilities. Access to the building is provided by two curbs cuts off of Wells Avenue, on the east and west sides of the building. Existing sewer, water, and gas services connect to the building from the mains located within Wells Avenue.

2.1 Existing Topography and Drainage Infrastructure

The topography in the eastern portion of the site slopes towards an existing wetland system. Existing stormwater runoff for the driveway and parking spaces located north of the existing building flows into a swale that wraps around the parking lot to the east and discharges into the wetland. The swale also collects drainage from the parking lot on the eastern side of the existing building prior to discharging into the wetland over a riprap spillway.

West of the existing building, topography generally slopes down from the northern and southern property lines towards the center of the site. Ledge outcroppings are apparent in the northwest corner of the site that slopes steeply to the south. Wells Avenue is at a higher elevation than the site and the northern portion of the roadway and associated shoulder also slope towards the site. An existing drainage swale crosses the southern portion of the site from west to east and flows into a headwall located near the western driveway. A small portion of the site flows north and offsite. Stormwater runoff from the existing western parking lot flows overland into this swale. The swale is connected to the closed drainage system in Wells Avenue by a 12-inch reinforced concrete pipe and controlled by a headwall and catch basin inlets.

Two catch basins, located south of the existing building, appear to collect rooftop runoff and runoff from the adjacent grass area. These catch basins also appear to connect to the closed drainage system in Wells Avenue. The existing stormwater management system was constructed prior to the 2008 MassDEP Stormwater Management Standards, and the site provides minimal peak flow attenuation, water quality treatment, and groundwater recharge. The existing site does not comply with the current guidelines.

2.2 NRSC Soil Designations

The Soil Classification Summary (Table 1) outlines the Natural Resources Conservation Services (NRCS) designation of the soil series at the site. The majority of soils are classified as Udorthents-

Urban Land Complex. The soils in the northwestern corner of the site are classified as Charlton-Hollis Rock Outcrop Complex, 3-8% slope with a hydrologic soil group (HSG) rating of B, indicating that the soils have a moderate infiltrative capacity (Figure 7). The Freetown muck series are located on the eastern portion of the site in the wetland and are HSG A/D (minimal infiltration).

Soil Unit	Soil Series	Hydrologic Soil Group
52A	Freetown Muck, 0-1% slopes	A/D
103B	Charlton Hollis Rock outcrop complex, 3-8% slopes	В
656	Udorthents-Urban Land complex	

2.3 On-Site Soil Investigations

Sixteen (16) test pits were performed on the Project site by Nitsch Engineering on August 26 and 27, 2014. The soil evaluation was performed by a Register Professional Engineer and Certified Soil Evaluator to obtain the seasonal high groundwater elevation and rate of percolation

The results of the test pits were consistent throughout the Site and indicated sandy loam or loamy sand, which are classified as HSG B and A, respectively. Ledge outcrops were observed west of the existing parking lot and ledge was encountered in the test pits located in the northern and western portions of the site. Indicators of estimated seasonal high groundwater were observed in several of the test pits ranging between three (3) and seven (7) feet below grade. In the vicinity of the proposed subsurface infiltration system, seasonal high groundwater was estimated to be at elevation 96.8 feet.

As required by the City of Newton, a percolation test was performed at Test Pit #5. The percolation rate in this test hole was determined to be 6.40 minutes per inch.

The test pit logs and percolation test results are provided in Appendix E and included in the site plan set.

2.4 Wetland Resource Areas

On August 15, 2014 Environmental Consulting & Restoration LLC delineated the leeward limits of Bordering Vegetated Wetlands (BVW) and the Inland Bank of a Perennial Stream located east of the Project site. The 200-foot Riverfront Area associated with the perennial stream and 100-foot Buffer Zone associated with Bank and BVW encroach on the eastern edge of the existing parking lot. There is no proposed work within any of the jurisdictional wetland resource areas.

2.5 100-Year Floodplain

As determined from the City of Newton Zoning Ordinance, Section 22-22 Floodplain/Watershed Protection Provisions, the 100-year floodplain for the Nahanton Street Swamp is at elevation 102.2 feet, and extends onto the easternmost portion of the existing parking lot. There is no proposed above grade work within the floodplain.

2.6 Total Maximum Daily Load (TMDL)

The Project site is located within the middle Charles River watershed and is subject to the Total Maximum Daily Load for Nutrients in the Upper/Middle Charles River, Massachusetts, dated May

2011, and the Final Pathogen TMDL for the Charles River Watershed, dated January 2007. To meet the intent of the TMDLs, the Project has been designed to significantly improve the stormwater quality over the existing condition and uses biofiltration and infiltration to target pathogen and nutrient pollutants.

3.0 PROPOSED CONDITIONS

3.1 **Project Description**

The Project consists of the demolition of a portion of a 1-story existing building, renovation of the remaining portion of the 1-story existing building, and construction of a new 3-story office building. The new 3-story office building will connect to the remaining portion of the 1-story building. There is a decrease in the building footprint of 0.09 acres. The two existing vehicular access driveways off of Wells Avenue will be maintained.

Exterior site improvements include installation of building services, enlargement and reconfiguration of the existing western parking lot, construction of a new parking area in the northeast portion of the site in place of a portion of the demolished building, and the construction of a small 18-space parking area near the eastern access driveway. A retaining wall will be constructed in the northwest corner of the site to allow for the expansion of the western parking lot.

The existing sewer and roof drain services will be maintained for the portion of renovated existing building. However, the new 3-story building will have new drain, water, sewer, gas, and telecommunication services. A new stormwater management system is proposed to provide mitigation of stormwater quantity through recharge and detention and significantly improve the quality of water being discharged off-site.

The Project is anticipated to increase the overall impervious area (site pavement) by approximately 0.35 acres. Refer to Table 2 for a comparison of the existing and proposed land use for the Site.

Land Use	Existing Site (acres)	Proposed Site (acres)	Change
Buildings	1.58	1.49	-0.09
Site Pavement	2.67	3.11	+ 0.44
Grass/Landscaping	1.93	2.56	+ 0.63
Woods	2.05	1.07	- 0.98
Total	8.23	8.23	

Table 2. Proposed land cover for 2 Wells Avenue (in acres)

3.2 Stormwater Management System

The proposed work includes the installation of a stormwater management system that is being designed to meet and exceed the MassDEP Stormwater Management Standards and the City of Newton Engineering Division requirements to the greatest extent practicable. Although the vast majority of the project is a redevelopment, the increase in impervious area resulting from the parking lot expansion is classified as "new development" work under the MassDEP Stormwater Management

Standards. Therefore, the Project will provide peak flow and volume mitigation consistent with City of Newton requirements and provide water quality treatment and groundwater recharge.

The Project has been designed using environmentally-sensitive site design and low impact development (LID) techniques. This design prevents the generation of stormwater and non-point source pollution by disconnecting flow paths, treating and infiltrating stormwater at its source, and protecting natural processes. Stormwater systems have been designed to model natural hydrologic features, including the use of a tree box filter, biofilter swales, an infiltration trench, and subsurface infiltration. Deep sump, hooded catch basins and proprietary water quality structures will provide treatment where LID measures are not feasible due to site constraints. Refer to the TSS Removal spreadsheets in Appendix A for TSS removal summaries for each treatment train implemented at the site.

As required by the City of Newton, runoff from the new impervious parking areas will be infiltrated on-site, and there will be no increase in stormwater volumes discharged from the site for the 100-year, 6.5-inch depth storm. In the western parking lot, runoff is collected and infiltrated in the proposed subsurface infiltration system. In the new small parking lot located east of the building, runoff is collected and infiltrated in a tree box filter. There is no increase in stormwater runoff volume or flow rate to abutting properties.

Biofilter Swales

Five (5) biofilter swales are proposed to collect, treat, and convey stormwater runoff generated by the parking lot, including new and existing impervious areas. Biofilter swales treat stormwater through sedimentation and gravity separation. Pretreatment for the biofilter swales will be provided by either linear peastone sediment traps (for parking areas without curbing) or riprap sediment sumps located at curb breaks. The biofilter swales will overflow into area drains that contain a four-foot sump for additional treatment.

Tree Box Filters

One (1) tree box filter is proposed to provide water quality treatment and groundwater recharge for stormwater runoff generated by the new eastern parking area. The tree box filter design includes a deep sump and hooded catch basin that overflows to an open bottom tree pit. Water quality treatment within the tree pit is facilitated by filtration through the soil media and nutrient uptake by the tree. Overflow from the tree box filters will discharge into an existing connection to the Wells Avenue closed drainage system.

Subsurface Infiltration System

Stormwater from a significant portion of the northeastern and western parking lot and 2,500 square feet of the new 3-story roof will be collected and infiltrated using a subsurface infiltration system. Site impervious area that is tributary to these systems will be pretreated using biofilter swales and area drains with sumps.

The system consists of StormTech SC-740 chambers configured as shown on the design plans. The chambers will be enveloped within a crushed stone base that extends 6 inches above and below the chambers and 12 inches around the perimeter of the chambers. The subsurface infiltration system is designed to infiltrate the 1-inch storm event and the required recharge volume under the MassDEP Stormwater Management Standards and significantly reduces the peak rate and runoff volumes in the 2-, 10-, 25- and 100-year design storms. The system is set at elevation 98.8, which is a minimum of two feet above the seasonal high groundwater elevation estimated in the nearby test pits.

As required by the City of Newton, there are details of the subsurface infiltration system provided in the plan set. These details outline the elevation of the bottom of the stone (98.8 feet), bottom of the chambers (99.3 feet), and the depth of the groundwater table (96.8 feet). The stone envelope surrounding the subsurface infiltration system shall be constructed with crushed washed stone. Additionally, the system will be covered with a layer of filter fabric plus a 3-inch layer of peastone on top of the system, and then covered over with filter fabric.

Deep Sump and Hooded Catch Basins

Deep sump and hooded catch basins are proposed to provide pretreatment in the portions of the parking lot and access driveway that do not drain to the proposed biofilter swales. As required by the City of Newton Engineering Division, the catch basins will contain the Neenah R-3705 gas trap to provide additional water quality protection.

Stormceptor® STC 900 Water Quality Structures

Two Stormceptor® STC water quality structures and two Stormceptor® STI water quality inlets are proposed for water quality pretreatment in areas of the site where stormwater cannot be routed to the infiltration system due to site elevations, or where there was not space for pretreatment prior to the infiltration system. These STCs have been designed to remove greater than 80% TSS in conjunction with their associated deep sump and hooded catch basins.

Infiltration Trench

A portion of the proposed new parking area in the northeastern portion of the site will sheet flow into a crushed stone infiltration trench. The infiltration trench was sized to infiltrate the 1-inch storm. The trench is 114.5 linear feet of 12-inch perforated pipe with 1.5 feet of crushed stone below and on either side and 1 foot of crushed stone above, brought to grade in a 2-foot wide trench.

3.3 Stormwater Management During Construction

The Site Contractor will be responsible for stormwater management of the active construction site and is required to adhere to the conditions of the 2012 Construction General Permit under the Environmental Protection Agency. Erosion and Sediment Control Plans and Details are included in the plan set and provide recommended locations for the perimeter erosion control barrier, inlet protection for existing and proposed drainage structures, steep slope protection, and stabilized construction entrances. A draft Stormwater Pollution Prevention Plan (SWPPP) document will be prepared in accordance with the MassDEP Stormwater Management Standards and the 2012 Construction General Permit and submitted prior to the start of construction.

4.0 STORMWATER MANAGEMENT ANALYSIS

4.1 Methodology

Nitsch Engineering completed a hydrologic analysis of the existing project site utilizing Soil Conservation Service (SCS) Runoff Curve Number (CN) methodology. The SCS method calculates the rate at which the runoff reaches the design point considering several factors: the slope and flow lengths of the subcatchment area, the soil type of the subcatchment area, and the type of surface cover in the subcatchment area. HydroCAD Version 10.00 computer modeling software was used in conjunction with the SCS method to determine the peak runoff rates and runoff volumes for the 2-, 10-, 25-, and 100-year, 24-hour storm events. The proposed project site is being analyzed with the same methodology.

The Site was divided into multiple drainage areas, or subcatchments, which drain to the design points along the property boundary and within the site. For each subcatchment area, SCS Runoff Curve Numbers (CNs) were selected by using the cover type and hydrologic soil group of each area. The peak runoff rates and runoff volumes for the 2-, 10-, 25- and 100-year 24-hour storm events were then determined by inputting the drainage areas, CNs, and time of concentration (T_c) paths into the HydroCAD model.

4.2 HydroCAD Version 10.00

The HydroCAD computer program uses SCS and TR-20 methods to model drainage systems. TR-20 (Technical Release 20) was developed by the Soil Conservation Service to estimate runoff and peak discharges in small watersheds. TR-20 is generally accepted by engineers and reviewing authorities as the standard method for estimating runoff and peak discharges.

HydroCAD Version 10.00 uses up to four types of components to analyze the hydrology of a given site: subcatchments, reaches, basins, and links. Subcatchments are areas of land that produce surface runoff. The area, weighted CN, and T_c characterize each individual subcatchment area. Reaches are generally uniform streams, channels, or pipes that convey water from one point to another. A basin is any impoundment that fills with water from one or more sources and empties via an outlet structure. Links are used to introduce hydrographs into a project from another source or to provide a junction for more than one hydrograph within a project. The time span for the model was set for 0-48 hours in order to prevent truncation of the hydrograph.

4.3 **Precipitation Data**

Nitsch Engineering, Inc. used Technical Paper 40 by the National Weather Service to estimate the rainfall for the 2-year, 10-year, 25-year 24-hour storms in Middlesex County. The City of Newton Engineering Division requires the use of 6.5 inches of rainfall for the 100-year, 24-hour storm event.

The rainfall values used in the HydroCAD analysis are as follows:

Storm Event	24–hour Rainfall
2-year	3.2 in.
10-year	4.6 in.
25-year	5.5 in.
100-year	6.5 in.

4.4 Existing Hydrologic Conditions

As summarized in Table 4, Nitsch Engineering delineated the project site into five (5) on-site subcatchment (watershed) areas discharging to three (3) design points utilizing an existing conditions survey and on-site observations (See Figure 1). Table 4 summarizes the design point, location and area of each watershed. The design points (DP) are defined as the northwest property boundary (DP-1), the Wells Avenue closed drainage system (DP-2), and the eastern wetland (DP-3). The HydroCAD model for existing conditions is provided in Appendix B.

Design Point	Watershed	Area (sf)	Description	
1	1S-Sub1	10,968	Woods	
	2S-Sub2	147,391	Paved parking and adjacent woods and grass – Flows to headwall along Wells Avenue	
2	3S-Sub3	47,383	Building roof and adjacent grass- Flows to ex. CB that discharges into Wells Avenue	
	4S-Sub4	38,880	Building roof and adjacent grass- Flows to ex. CB that discharges into Wells Avenue	
3	5S-Sub5	113,982	Paved parking and adjacent woods and grass – Flows to eastern wetland system	
Total Area	a	358,604		

Table 4. Existing Drainage Area Summary

4.5 Proposed Hydrologic Conditions

The proposed project has been designed to mitigate the change in stormwater runoff at each of the design points as required by the DEP Stormwater Management Standards and the City of Newton Engineering Division requirements. The existing watershed areas were modified to reflect the proposed topography, storm drainage structures and BMPs, and roof areas. (See Figure 2 and Table 5). The proposed subsurface infiltration system and infiltrations trench were included as a ponds in the HydroCAD model.

Design Point	Watershed	Area (sf)	Description	Proposed Treatment BMP(s)
1	1S-Sub1	6,521	Woods and grass	Sheet flow (maintain existing)
	2S-Sub2A	28,661	Parking and grass	Biofilter Swale to AD to Subsurface Infiltration
	3S-Sub2B	18,034	Parking and grass	Biofilter Swale to AD to Subsurface Infiltration
	4S-Sub2C	25,380	Parking, woods, and grass	Biofilter Swale to AD to Subsurface Infiltration
0	5S-Sub2D	12,647	Parking and grass	Biofilter Swale to AD
2	6S-Sub2E	10,382	Parking, woods, and grass	Biofilter Swale to AD
	7S-Sub2F	17,153	Parking, woods, and grass	Biofilter Swale to AD
	14S-Sub2G	15,595	Roof and grass	Maintain connection to ex. CB
	20S-Sub3B	16,912	Parking and grass	Subsurface Infiltration System

Table 5. Proposed Drainage Area Summary

	21S	8,198	Parking and grass	Subsurface Infiltration System
	15S	2,500	Roof	Subsurface Infiltration System
	16s	39,556	Roof	-
	8S	14,967	Patio and grass	Area Drains with sumps
	9S-Sub4	37,456	Parking, roof, and grass	Tree Box Filters to ex. CB
3	10S-Sub5	93,190	Parking and adjacent woods and grass	Maintain existing drainage swale to eastern wetland system
	13S	9,052	Parking, landscaped islands	Infiltration Trench
Total Area		358,604		

4.6 Peak Flow Rates

The proposed stormwater management system is expected to reduce the proposed peak runoff rates and runoff volumes to at or below the existing rates for Design Points DP-1, DP-2, and DP-3. Tables 6 through 8 below summarize the existing and proposed hydrologic analyses for the site at each design point.

Table 6 – Peak Rates of Runoff for Design Point DP-1

Storm Event	0.000	10	05 Moor	100 100
Storin Event	2-year	10-year	25-year	100-year
Existing Peak Rate (cfs)	0.03	0.17	0.31	0.51
Proposed Peak Rate (cfs)	0.02	0.13	0.21	0.34
Existing Volume (cf)	229	720	1,133	1,713
Proposed Volume (cf)	168	488	750	1,115

Table 7 – Peak Rates of Runoff for Design Point DP-2

Storm Event	2-year	10-year	25-year	100-year
Existing Peak Rate (cfs)	9.17	16.02	20.59	26.26
Proposed Peak Rate (cfs)	7.54	12.28	18.35	22.61
Existing Volume (cf)	31,011	53,826	69,346	88,872
Proposed Volume (cf)	27,526	51,756	68,171	88,862

Table 8 – Peak Rates of Runoff for Design Point DP-3

Storm Event	2-year	10-year	25-year	100-year
Existing Peak Rate (cfs)	2.33	4.85	6.62	8.87
Proposed Peak Rate (cfs)	1.01	2.69	3.96	5.66
Existing Volume (cf)	9,855	19,468	26,303	35,102
Proposed Volume (cf)	5,000	11,512	16,488	23,154

5.0 MassDEP Stormwater Management Standards

The Project is not located within a jurisdictional water resource area of buffer zone and does not fall under the jurisdiction of the Newton Conservation Commission. Therefore, the MassDEP Stormwater Management Standards are not applicable. However, the Project has been designed to meet and exceed these Standards to the maximum extent practicable as summarized below. This design reflects a significant improvement over the existing condition.

Standard 1: No New Untreated Discharges

The Project will not discharge any untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth. Stormwater from the site will be collected and treated in accordance with the City of Newton requirements and the MassDEP Stormwater Management Standards. The proposed parking areas will provide an increased level of treatment over what currently existing on-site, incorporating substantial low impact development techniques.

Standard 2: Peak Rate Attenuation

The proposed stormwater management system will be designed so that the post-development peak discharge rates do not exceed pre-development peak discharge rates. To prevent storm damage and downstream flooding, the proposed stormwater management practices will mitigate peak runoff rates for the 2-, 10-, 25- and 100-year, 24 hour storm events.

In addition to peak rate attenuation, the City of Newton Engineering Division requires that the peak volume of stormwater runoff leaving the post-development site will not exceed the peak volume leaving the pre-development site for the 2-, 10-, 25-, and 100-year storm events. The proposed stormwater management system is expected to reduce or maintain the post-development volumes of runoff to at or below the pre-development volumes. Therefore, the proposed system will exceed the DEP Stormwater Management Standards.

Standard 3: Groundwater Recharge

The site was designed using environmentally-sensitive site design, low impact development techniques, and stormwater BMP treatment trains to minimize the loss of annual recharge to groundwater. The annual recharge from the post-development site will approximate the annual recharge from pre-development conditions based on soil type using the guidelines provided in the MassDEP Stormwater Management Handbook.

New Impervious Area	= 0.35 acres
Rv (Recharge Volume)	= 0.35 in. / (12 inches/ft) x 0.35 acres x 43,560 sf/acre
	= 445 cubic feet

The subsurface infiltration system was designed to maximize the amount of groundwater recharge provided by the Project. The volume provided below the outlet in the subsurface infiltration system is **8,394 cubic feet**, which <u>exceeds</u> the recharge volume required under the MassDEP Stormwater Management Standards. The proposed tree box filter and infiltration trench will provide additional groundwater recharge.

The HydroCAD reports provided in Appendix C indicate that the subsurface infiltration system will drain within 48 hours for the 2-, 10-, 25-, and 100-year storm events, exceeding the 72-hour MassDEP drawdown requirement.

Standard 4: Water Quality Treatment

The proposed stormwater management system will be designed to remove greater than 80% of the average annual post-construction load of Total Suspended Solids (TSS) to the maximum extent practicable. Structural stormwater BMPs including biofilter swales, tree box filters, deep sump and hooded catch basins, and Stormceptor[®] water quality units are sized to capture the required water quality volume (1 inch over the project site) and remove a minimum of 80% of total suspended solids. TSS removal calculation spreadsheets, Stormceptor[®] sizing calculations, and dry well sizing calculations are provided in Appendix A.

Source control and pollution prevention measures, such as vacuum cleaning, street sweeping, proper snow management, and stabilization of eroded surfaces, are included in the Long-Term Pollution Prevention Plan and Operation and Maintenance Plan (Appendix D).

Standard 5: Land Uses with Higher Potential Pollutant Loads

The Project is anticipated to be classified as a Land Use with Higher Potential Pollutant Loads (LUHPPLs) as defined by MassDEP. The parking areas of the project are classified as a LUHPPL based on the anticipated average daily vehicle trips to be generated by the proposed Project (greater than 1,000 vehicle trips per day). As previously noted, through the incorporation of several LID techniques, the proposed stormwater management system will provide a significant improvement to the water quality being discharged off of the Project site. The proposed BMPs, including biofilter swales, deep sump and hooded catch basins and area drains, and tree box filter provide a high level of sediment and oil and grease removal. Therefore, the Project complies with this Standard.

Standard 6: Critical Areas

The Project is not located within any critical areas. Therefore, this standard is not applicable.

Standard 7: Redevelopments

The Project is a mix of redevelopment and new development under the MassDEP Stormwater Management Standards. Where the new development proposes expansion of impervious cover, the standards for new development are met. In areas of redevelopment, the standards will be met to the greatest extent practicable.

Standard 8: Construction Period Pollution Prevention and Sedimentation Control

A plan to control construction-related impacts, including erosion, sedimentation, and other pollutant sources during construction and land disturbance activities (construction period erosion, sedimentation, and pollution prevention plan) will be developed prior to the start of construction.

Since the Project will disturb more than one (1) acre of land, a Notice of Intent will be submitted to the Environmental Protection Agency (EPA) for coverage under the National Pollution Discharge Elimination System (NPDES) Construction General Permit. As part of this application the Applicant

is required to prepare a Stormwater Pollution Prevention Plan (SWPPP) and implement the measures in the SWPPP. The SWPPP, which is to be kept on site, includes erosion and sediment controls (stabilization practices and structural practices), temporary and permanent stormwater management measures, Contractor inspection schedules and reporting of all SWPPP features, materials management, waste disposal, off-site vehicle tracking, spill prevention and response, sanitation, and non-stormwater discharges. A draft SWPPP will be provided prior to the start of construction.

Standard 9: Operation and Maintenance Plan

A post-construction operation and maintenance plan has been prepared and will be implemented to ensure that stormwater management systems function as designed. Source control and stormwater BMP operation requirements for the academic campus are summarized in the Long-Term Pollution Prevention Plan and Operation and Maintenance Plan provided in Appendix D.

Standard 10: Prohibition of Illicit Discharges

There will be no illicit discharges to the stormwater management system associated with the Project. An Illicit Discharge Compliance Statement is provided in Appendix A.

6.0 CLOSED DRAINAGE SYSTEM DESIGN

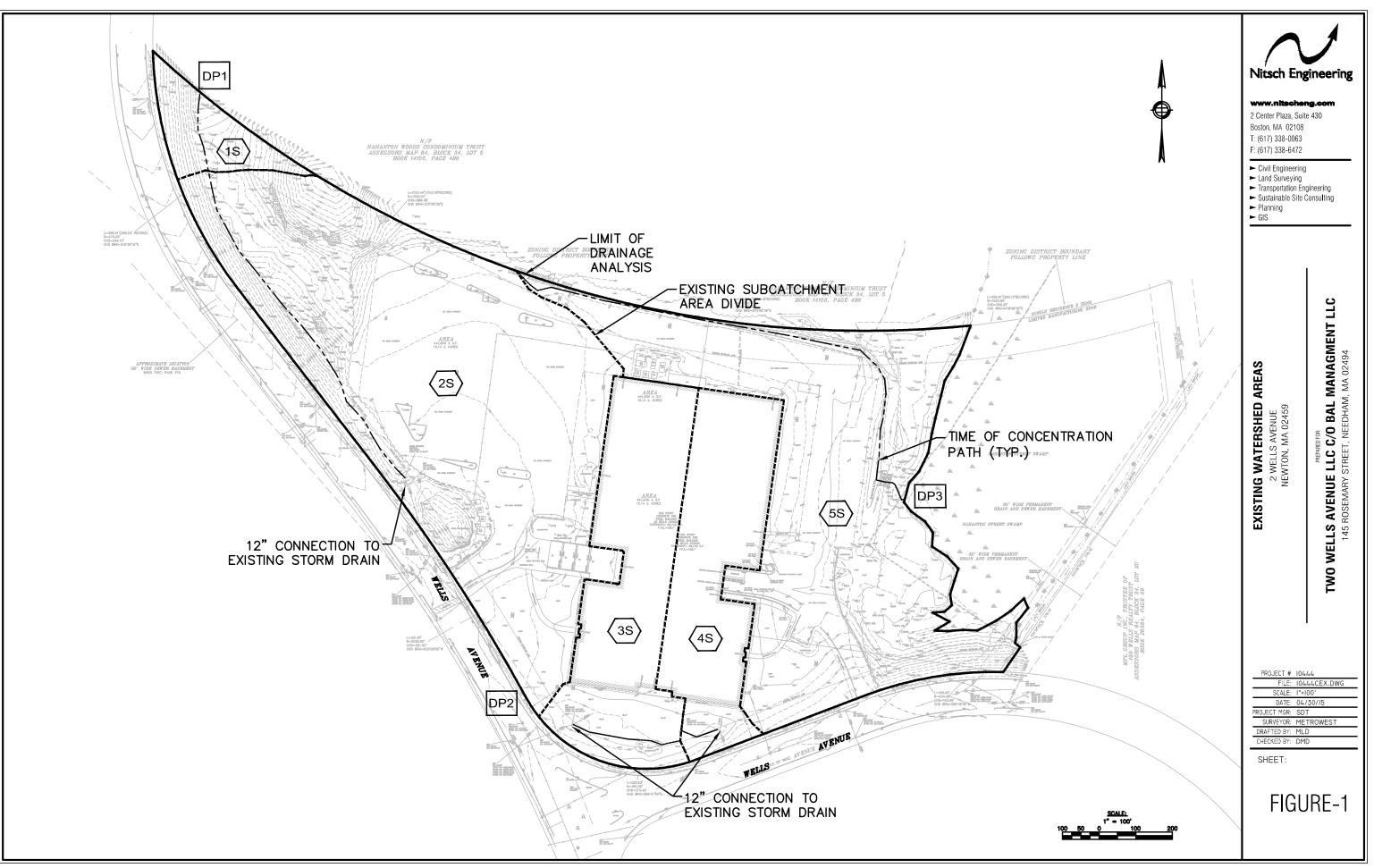
The proposed closed drainage system consists of deep sump and hooded catch basins, drainage manholes, infiltration structures, and proprietary water quality treatment units connected with corrugated polyethylene pipe. The closed drainage system was designed to convey the 10-year storm event using the Rational method. The pipe connection out of the subsurface infiltration system outlet control structure was designed to convey the 100-year storm outflow to prevent any backup into the system.

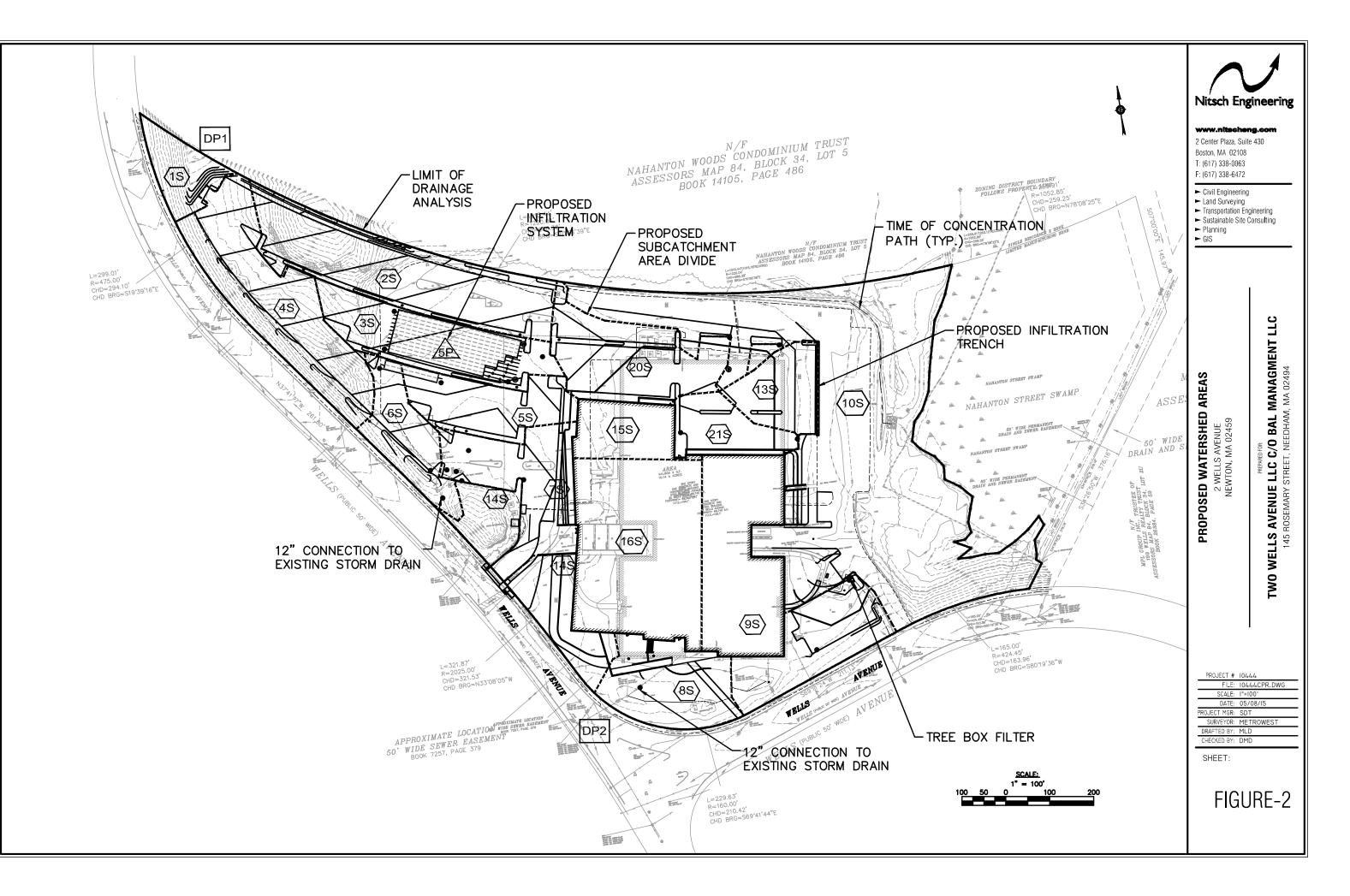
7.0 CONCLUSION

In conclusion, the existing site does not meet current MassDEP Stormwater Standards. The Project's proposed stormwater management system will reduce or maintain peak runoff rates and volumes through the widespread use of infiltration BMPs and improve the water quality of stormwater being discharged from the Site. Environmentally sensitive site design and low impact development techniques will be implemented throughout the Site, and no work is proposed within the site's sensitive areas. The Project is being designed to meet and exceed the MassDEP Stormwater Management Standards and the City of Newton Engineering Division requirements to the maximum extent practicable. The proposed Project incorporates a significant improvement over the existing condition with regards to stormwater management.

FIGURES

Figure 1	Existing Watershed Areas
Figure 2	Proposed Watershed Areas





5/6/2015 3:06 P

APPENDIX A

Stormwater Management Standards Documentation

Standard 4: TSS Removal Calculations Standard 4: Proprietary Water Quality Structure Design Standard 4: Infiltration Trench Sizing for Water Quality Storm Standard 10: Illicit Discharge Compliance Statement



2 WELLS AVENUE, NEWTON MASSACHUSETTS WATER QUALITY TREATMENT SUMMARY

Nitsch Engineering has prepared this Water Quality Treatment Summary for the proposed Project located at 2 Wells Avenue in Newton, Masaschusetts. In compliance with MassDEP Stormwater Management Standard #4, the proposed stormwater management system is designed to remove at least 80% of the average annual post-construction load of Total Suspended Solids (TSS).

A summary of treatment trains proposed to provide water quantity control and water quality improvement at the proposed project site is provided below.

Treatment Train 1

Biofilter Swale \rightarrow Area Drain with Deep Sump \rightarrow Subsurface Infiltration System

Treatment Train 2

Biofilter Swale \rightarrow Area Drain with Deep Sump \rightarrow Proprietary Water Quality Structure

 $\frac{\text{Treatment Train 3}}{\text{Deep Sump and Hooded Catch Basins}} \rightarrow \text{Tree Box Filter}$

Treatment Train 4

Deep Sump and Hooded Catch Basins → Proprietary Water Quality Structure

Treatment Train 5

Infiltration Trench



Treatment Train 1

Biofilter Swale \rightarrow Area Drain with Deep Sump \rightarrow Subsurface Infiltration System

Treatment Spreadsheet

В	С	D	E	F
	TSS Removal	Starting TSS	Amount	Remaining
BMP	Rate	Load	Removed (C*D)	Load (D-E)
Subsurface Infiltration System with Pretreatment*	0.80	1.00	0.80	0.20
		Total TSS Removal =	80%	Meets 80% TSS removal requirement

*Pretreatment provided by biofilter swale and area drain with sump.



Treatment Train 2

Biofilter Swale \rightarrow Area Drain with Deep Sump \rightarrow Proprietary Water Quality Structure

Treatment Spreadsheet

В	С	D	Е	F
	TSS Removal	Starting TSS	Amount	Remaining
BMP	Rate ¹	Load*	Removed (C*D)	Load (D-E)
Biofilter Swale	0.50	1.00	0.50	0.50
Area Drain w/ Sump	0.25	.50	.125	0.375
Proprietary WQS	0.80	0.375	0.30	0.075
		Total TSS Removal =	92.5%	Meets and exceeds 80% TSS removal requirement

Refer to attached sizing documentation for proprietary treatment structures.



 $\frac{\text{Treatment Train 3}}{\text{Deep Sump and Hooded Catch Basin} \rightarrow \text{Tree Box Filter}}$

Treatment Spreadsheet

В	C TSS Removal	D Starting TSS	E Amount	F Remaining
BMP	Rate ¹	Load*	Removed (C*D)	Load (D-E)
Tree Box Filter with Pretreatment *	80	1.00	.80	.20
			2007	Meets 80% TSS
		Total TSS Removal =	80%	removal requirement

*Pretreatment provided by deep sump and hooded catch basin



 $\frac{\text{Treatment Train 4}}{\text{Deep Sump and Hooded Catch Basins}} \rightarrow \text{Proprietary Water Quality Structure}$

Treatment Spreadsheet

В	С	D	E	F
	TSS Removal	Starting TSS	Amount	Remaining
BMP	Rate	Load	Removed (C*D)	Load (D-E)
Deep Sump and Hooded Catch Basin	0.25	1.00	0.25	0.75
Proprietary WQS	0.80	0.75	0.60	0.15
		Total TSS Removal =	85%	Meets and exceeds 80% TSS removal requirement

Refer to attached sizing documentation for proprietary treatment structures.



Treatment Train 5 Infiltration Trench

Treatment Spreadsheet

В	С	D	E	F
	TSS Removal	Starting TSS	Amount	Remaining
BMP	Rate	Load	Removed (C*D)	Load (D-E)
Infiltration Trench	0.55	1.00	0.55	0.45
				1
		Total TSS Removal =	55%	



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1" Calculation Sheet

Nitsch Job #	10444
Calc:	MLD
Date:	8-May-15

This spreadsheet should be used to convert water quality volume to an equivilent water quality peak flow rate as outlined in the new MA DEP guidelines that take effect on October 15, 2013.

Glossary

Water Quality Flow Rate =	WQF
Water Quality Volume =	WQV*
unit peak discharge (csm/in) =	qu**
Impervious Area in watershed (square miles) =	Ai

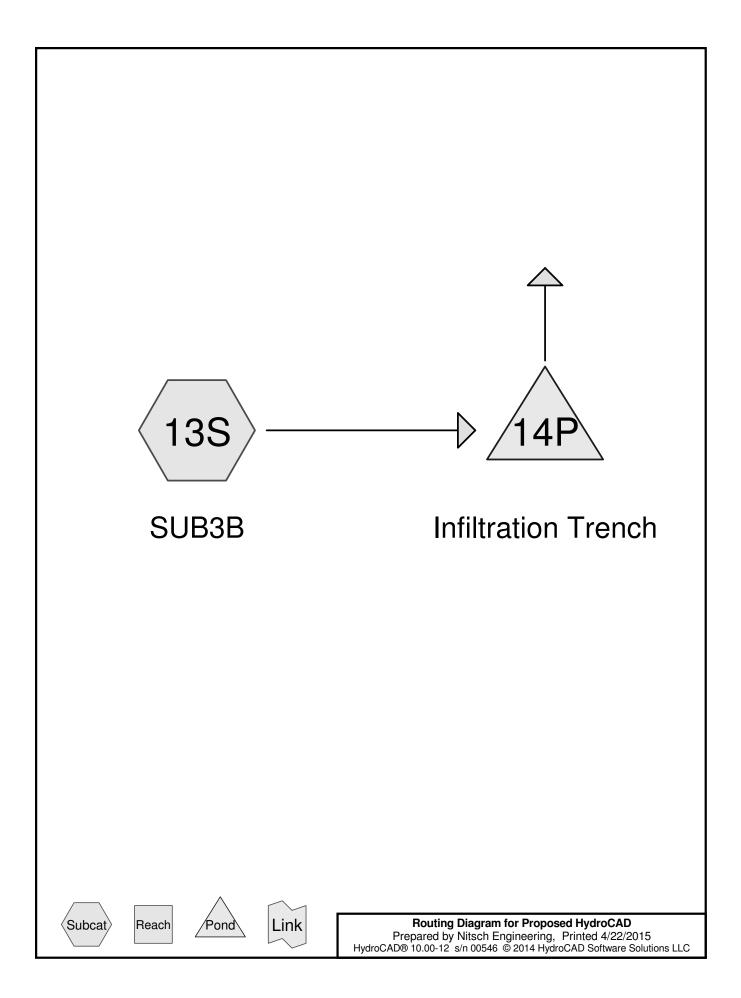
*WQV is expressed in watershed inches (you must use 1.0-inches in all cases with this method and not 0.5-inches) ** calculate the qu based on the time of concentration (see 1" - qu Table)

Compute Water Quality Flow with the following Equation

WQF = (qu)(A)(WQV)

Input Information (in colored cells only)

Site Plan Callout		Enter qu (from 1" - qu Table)	Enter Impervious Area (SF)	Ai (sq/mi)	WQV		WQF	
WQU-500	=	774	16029	0.000575	1	=	0.45	cfs
WQU-208	=	774	9073	0.000325	1	=	0.25	cfs
STC-100	=	774	7657	0.000275	1	ш	0.21	cfs
STC-102	=	774	7024	0.000252	1	=	0.20	cfs
	=			0.000000	1	ш	0.00	cfs
	=			0.000000	1	=	0.00	cfs
	=			0.000000	1	=	0.00	cfs
	=			0.000000	1	=	0.00	cfs
	=			0.000000	1	=	0.00	cfs
	=			0.000000	1	=	0.00	cfs



Proposed HydroCAD Prepared by Nitsch Engineering HydroCAD® 10.00-12 s/n 00546 © 2014 HydroCAD Software Solutions LLC Printed 4/22/2015 Page 2

Area Listing (selected nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
109	61	>75% Grass cover, Good, HSG B (13S)
8,943	98	Roofs, HSG B (13S)
9,052	98	TOTAL AREA

Proposed HydroCAD Prepared by Nitsch Engineering HydroCAD® 10.00-12 s/n 00546 © 2014 HydroCAD Software Solutions LLC Printed 4/22/2015 Page 3

Soil Listing (selected nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
9,052	HSG B	13S
0	HSG C	
0	HSG D	
0	Other	
9,052		TOTAL AREA

Proposed HydroCAD

Prepared by Nitsch Engineering HydroCAD® 10.00-12 s/n 00546 © 2014 HydroCAD Software Solutions LLC Printed 4/22/2015 Page 4

Ground Covers (selected nodes)								
	HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Su
	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	Cover	Nu
	0	109	0	0	0	109	>75% Grass	
							cover, Good	
	0	8,943	0	0	0	8,943	Roofs	
	0	9,052	0	0	0	9,052	TOTAL AREA	

Ground Covers (selected nodes)

	Infiltration Trench Sizing for Water Quality Storm
Proposed HydroCAD	Type III 24-hr 1-inch storm Rainfall=1.00"
Prepared by Nitsch Engineering	Printed 4/22/2015
HydroCAD® 10.00-12 s/n 00546 © 2014 HydroCAD Softwa	re Solutions LLC Page 5

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 13S: SUB3B

Runoff Area=9,052 sf 98.80% Impervious Runoff Depth=0.79" Tc=6.0 min CN=98 Runoff=0.18 cfs 597 cf

Pond 14P: Infiltration Trench

Peak Elev=100.97' Storage=269 cf Inflow=0.18 cfs 597 cf Discarded=0.01 cfs 597 cf Primary=0.00 cfs 0 cf Outflow=0.01 cfs 597 cf

Total Runoff Area = 9,052 sf Runoff Volume = 597 cf Average Runoff Depth = 0.79" 1.20% Pervious = 109 sf 98.80% Impervious = 8,943 sf

Proposed HydroCAD

Summary for Subcatchment 13S: SUB3B

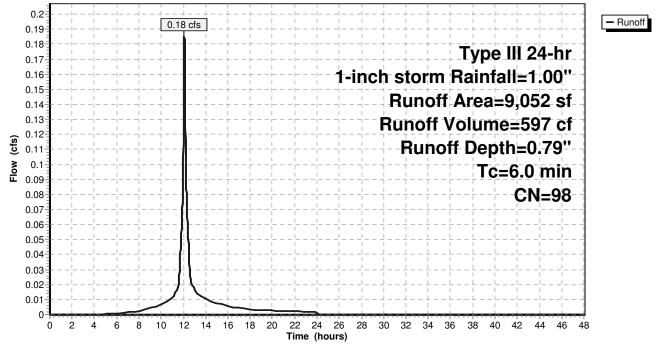
Runoff 0.18 cfs @ 12.08 hrs, Volume= 597 cf, Depth= 0.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 1-inch storm Rainfall=1.00"

A	rea (sf)	CN	Description			
	8,943	98	Roofs, HSC	àВ		
	109	61	>75% Gras	s cover, Go	ood, HSG B	
	9,052	98	Weighted Average			
	109		1.20% Pervious Area			
	8,943		98.80% Imp	pervious Ar	rea	
Tc	Length	Slope		Capacity	Description	
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)		
6.0					Direct Entry,	
					-	

Subcatchment 13S: SUB3B

Hydrograph



Summary for Pond 14P: Infiltration Trench

Inflow Area =	9,052 sf, 98.80% Impervious,	Inflow Depth = 0.79" for 1-inch storm event
Inflow =	0.18 cfs @ 12.08 hrs, Volume=	597 cf
Outflow =	0.01 cfs @ 11.11 hrs, Volume=	597 cf, Atten= 94%, Lag= 0.0 min
Discarded =	0.01 cfs @ 11.11 hrs, Volume=	597 cf
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 100.97' @ 13.88 hrs Surf.Area= 458 sf Storage= 269 cf

Plug-Flow detention time= 218.7 min calculated for 596 cf (100% of inflow) Center-of-Mass det. time= 218.7 min (1,006.5 - 787.9)

Volume	Invert	Avail.Stor	rage	Storage D	Description	
#1	99.50'	60)5 cf			below (Recalc)
#2	101.00'	ç	90 cf	1,603 cf Overall - 90 cf Embedded = 1,513 cf x cf 12.0'' Round Pipe Storage Inside #1 L= 114.5'		· · · · · · · · · · · · · · · · · · ·
		69	95 cf	Total Ava	ilable Storage	
Elevatio (fee		urf.Area (sq-ft)		.Store c-feet)	Cum.Store (cubic-feet)	
99.5	50	458		0	0	
103.0	00	458		1,603	1,603	
Device	Routing	Invert	Outl	et Devices		
#1	Primary	101.00'	12.0	" Round (Culvert	
			Inlet n= 0	= 52.0' CPP, square edge headwall, Ke= 0.500 hlet / Outlet Invert= 101.00' / 100.20' S= 0.0154 '/' Cc= 0.900 = 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf		
#2	Discarded	99.50'	1.02	0 in/hr Exf	iltration over S	Surface area
Discourd		Max. 0.01 af	1	4 4 4 Java J		

Discarded OutFlow Max=0.01 cfs @ 11.11 hrs HW=99.54' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=99.50' (Free Discharge)

Proposed HydroCAD Prepared by Nitsch Engineering

Hydrograph 0.2 - Inflow 0.18 cfs 0.19 Outflow Discarded 0.18 Inflow Area=9,052 sf Primary 0.17 0.16 Peak Elev=100.97' 0.15 0.14 Storage=269 cf 0.13 0.12 (s) 0.12 0.11 Flow 0.1 0.09 0.08 0.07 0.06 0.05 0.04 0.03 0.02 0.01 cfs 0.00 cfs 0-2 ò 6 10 12 14 16 22 24 26 28 30 32 34 36 38 40 42 44 46 4 8 18 20 48

Time (hours)

Pond 14P: Infiltration Trench



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May 8, 2015

2 WELLS AVENUE Newton, Massachusetts

STANDARD 10: Illicit Discharge Compliance Statement

Standard 10 states: All illicit discharges to the stormwater management system are prohibited.

This is to verify:

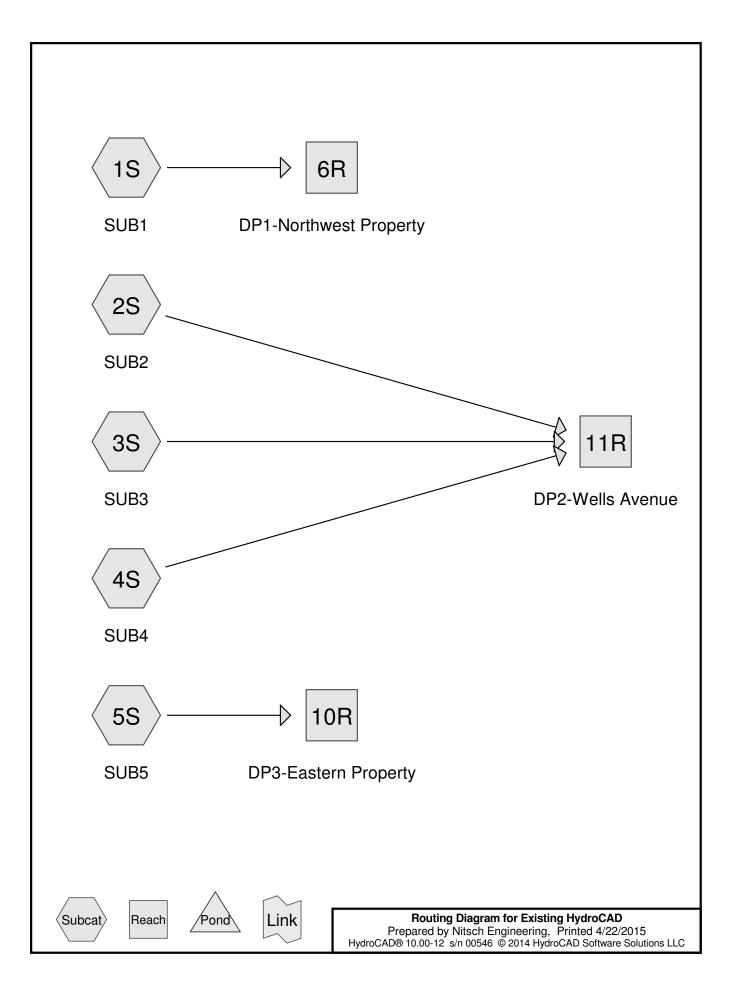
- 1. Based on the information available there are no known or suspected illicit discharges to the stormwater management system at the TWO WELLS AVENUE site as defined in the MassDEP Stormwater Handbook.
- 2. The design of the stormwater system includes no proposed illicit discharges.

Deborah Danik, PE

05/08/15 Date

APPENDIX B

Existing Conditions – HydroCAD Calculations



Printed 4/22/2015 Page 2

Area Listing (selected nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
84,043	61	>75% Grass cover, Good, HSG B (2S, 3S, 4S, 5S)
116,224	98	Paved parking (2S, 3S, 4S, 5S)
68,708	98	Roofs (3S, 4S)
89,629	55	Woods, Good, HSG B (1S, 2S, 5S)
358,604	79	TOTAL AREA

Printed 4/22/2015 Page 3

Soil Listing (selected nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
173,672	HSG B	1S, 2S, 3S, 4S, 5S
0	HSG C	
0	HSG D	
184,932	Other	2S, 3S, 4S, 5S
358,604		TOTAL AREA

					E	xisting Conc	ditions
Existing Hydro Prepared by Nits HydroCAD® 10.00-	ch Engineerii		D Software Solu	utions LLC	F	Printed 4/22 P	2/2015 2age 4
		Ground Co	vers (selected	l nodes)			
HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	
(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	Cover	

0

0

(sq-ft) Cover 84,043 >75% Grass cover, Good

68,708

89,629

358,604

Paved parking

Woods, Good

TOTAL AREA

Roofs

Su Nu

0 0 84,043 0 116,224 116,224

68,708

0

0 0 184,932

0

0

0

0

0

0

0

0

0

84,043

89,629

173,672

0

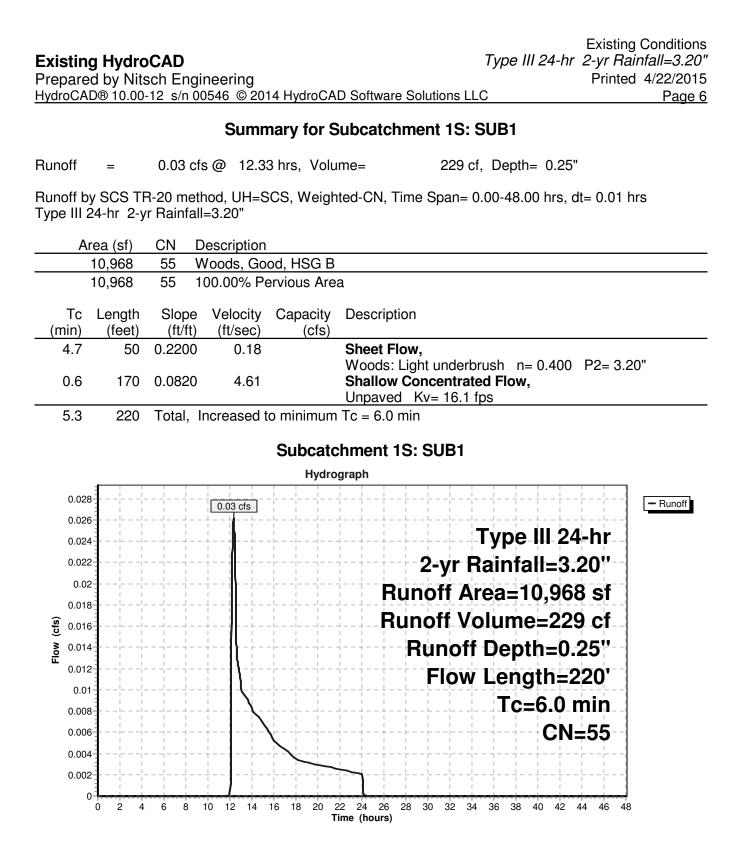
0

	Existing Conditions
Existing HydroCAD	Type III 24-hr 2-yr Rainfall=3.20"
Prepared by Nitsch Engineering	Printed 4/22/2015
HydroCAD® 10.00-12 s/n 00546 © 2014 HydroCAD Software Solutions LL	C Page 5

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Runoff Area=10,968 sf 0.00% Impervious Runoff Depth=0.25" Flow Length=220' Tc=6.0 min CN=55 Runoff=0.03 cfs 229 cf
Runoff Area=147,391 sf 48.22% Impervious Runoff Depth=1.21" Flow Length=559' Tc=7.5 min CN=77 Runoff=4.44 cfs 14,884 cf
Runoff Area=47,383 sf 73.87% Impervious Runoff Depth=2.00" Flow Length=128' Tc=8.7 min CN=88 Runoff=2.31 cfs 7,885 cf
Runoff Area=38,880 sf 89.06% Impervious Runoff Depth=2.54" Flow Length=83' Tc=6.0 min CN=94 Runoff=2.55 cfs 8,242 cf
Runoff Area=113,982 sf 38.80% Impervious Runoff Depth=1.04" Flow Length=627' Tc=13.8 min CN=74 Runoff=2.33 cfs 9,855 cf
Inflow=0.03 cfs 229 cf
Outflow=0.03 cfs 229 cf
Inflow=2.33 cfs 9,855 cf
Outflow=2.33 cfs 9,855 cf
Inflow=9.17 cfs 31,011 cf
Outflow=9.17 cfs 31,011 cf

Total Runoff Area = 358,604 sf Runoff Volume = 41,096 cf Average Runoff Depth = 1.38" 48.43% Pervious = 173,672 sf 51.57% Impervious = 184,932 sf



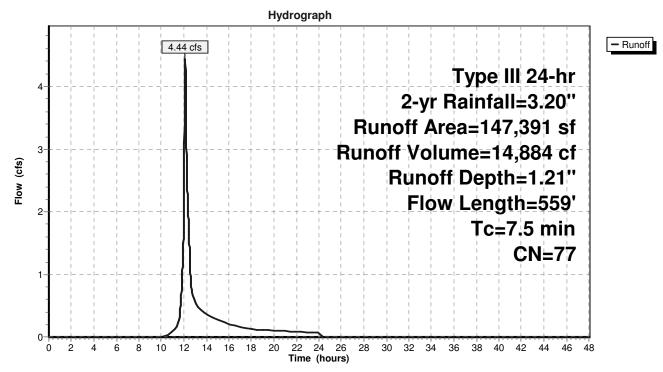
Page 7

Runoff = 4.44 cfs @ 12.11 hrs, Volume= 14,884 cf, Depth= 1
--

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.20"

	A	rea (sf)	CN	Description		
*		71,077	98	Paved park	ing	
		29,455	61	>75% Ġras	s cover, Go	ood, HSG B
		46,859	55	Woods, Go	od, HSG B	
	1	47,391	77	Weighted A	verage	
		76,314	57	51.78% Pe	rvious Area	
		71,077	98	48.22% Imp	pervious Ar	ea
	Тс	Length	Slope	,	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.7	50	0.2200	0.18		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.20"
	2.8	509	0.0350	3.01		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	7.5	559	Total			

Subcatchment 2S: SUB2



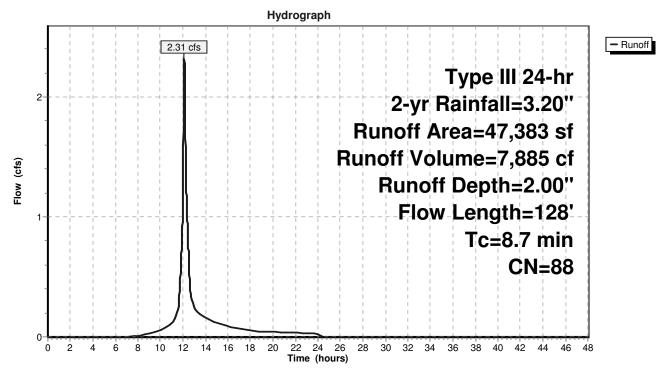
Summary for Subcatchment 3S: SUB3

Runoff	=	2.31 cfs @	12.12 hrs,	Volume=	7,885 cf, Depth= 2.00"
--------	---	------------	------------	---------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.20"

_	A	rea (sf)	CN	Description		
*		686	98	Paved park	ing	
*		34,315	98	Roofs	-	
_		12,382	61	>75% Gras	s cover, Go	ood, HSG B
		47,383	88	Weighted A	verage	
		12,382	61	26.13% Pei	rvious Area	
		35,001	98	73.87% Imp	pervious Ar	ea
	Тс	Length	Slope		Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	8.1	50	0.0080	0.10		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.20"
	0.6	78	0.0200	2.28		Shallow Concentrated Flow,
_						Unpaved Kv= 16.1 fps
	8.7	128	Total			

Subcatchment 3S: SUB3



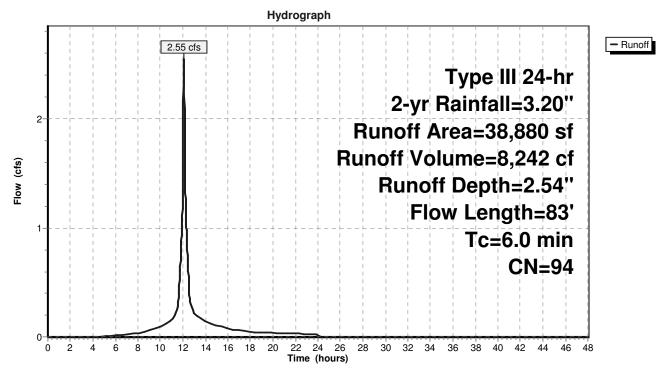
Summary for Subcatchment 4S: SUB4

Runoff	=	2.55 cfs @	12.08 hrs, Volume=	8,242 cf, Depth= 2.54"
--------	---	------------	--------------------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.20"

_	A	rea (sf)	CN I	Description		
*		234	98 I	Paved park	ing	
*		34,393	98 I	Roofs	-	
_		4,253	61 :	>75% Gras	s cover, Go	bod, HSG B
		38,880	94	Neighted A	verage	
		4,253	61 [·]	10.94% Per	vious Area	
		34,627	98 8	39.06% Imp	pervious Ar	ea
	Tc	Length	Slope		Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.4	50	0.0360	0.19		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.20"
	0.2	33	0.0240	2.49		Shallow Concentrated Flow,
_						Unpaved Kv= 16.1 fps
	4.6	83	Total,	Increased t	o minimum	Tc = 6.0 min

Subcatchment 4S: SUB4



Summary for Subcatchment 5S: SUB5

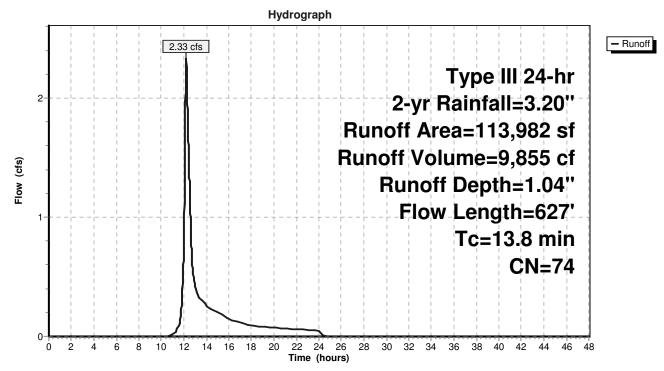
Runoff	=	2.33 cfs @	12.20 hrs, Volume=	9,855 cf, Depth= 1.04"
--------	---	------------	--------------------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.20"

	A	rea (sf)	CN E	Description					
*		44,227	98 F	98 Paved parking					
		37,953	61 >	-75% Gras	s cover, Go	bod, HSG B			
		31,802	55 V	Voods, Go	od, HSG B				
	1	13,982		Veighted A	•				
		69,755			rvious Area				
		44,227	98 3	88.80% Imp	pervious Ar	ea			
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
		· /		· · · · ·	(015)				
	6.5	50	0.0140	0.13		Sheet Flow, Grass: Short $n= 0.150$ P2= 3.20"			
	7.2	.2 542 0.0060 1.25			Shallow Concentrated Flow, Unpaved Kv= 16.1 fps				
	0.1	35	0.0860	4.72		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps			

13.8 627 Total

Subcatchment 5S: SUB5



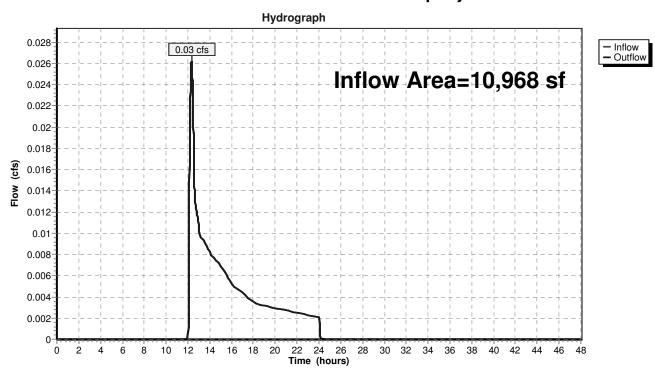
Existing Conditions

Printed 4/22/2015

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Inflow Are	a =	10,968 sf,	0.00% Impervious,	Inflow Depth = 0.25"	for 2-yr event
Inflow	=	0.03 cfs @ 1	12.33 hrs, Volume=	229 cf	
Outflow	=	0.03 cfs @ 1	12.33 hrs, Volume=	229 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach 6R: DP1-Northwest Property

Summary for Reach 10R: DP3-Eastern Property

Page 12

Inflow Are	a =	113,982 sf, 38.80% Impervious, Inflow Depth =	1.04" for 2-yr event
Inflow	=	2.33 cfs @ 12.20 hrs, Volume= 9,855 c	of
Outflow	=	2.33 cfs @ 12.20 hrs, Volume= 9,855 c	cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Hydrograph Inflow Outflow 2.33 cfs Inflow Area=113,982 sf 2 Flow (cfs) 0 22 24 26 Time (hours) 2 12 14 16 18 20 0 4 6 8 10 28 30 32 34 36 38 40 42 44 46 48

Reach 10R: DP3-Eastern Property

Summary for Reach 11R: DP2-Wells Avenue

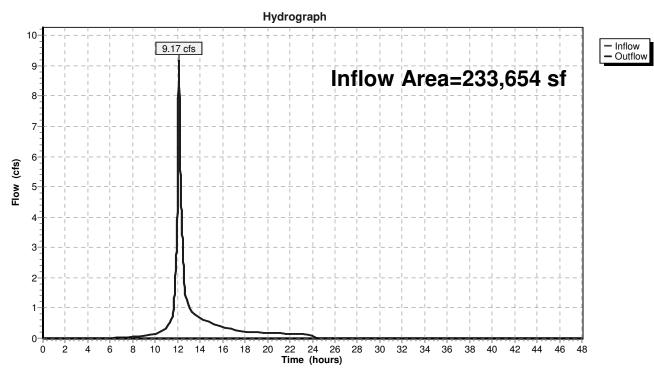
Existing Conditions

Printed 4/22/2015

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Inflow Are	a =	233,654 sf, 60.22% Impervious, Inflow Depth = 1.59" for 2-yr event	
Inflow	=	9.17 cfs @ 12.11 hrs, Volume= 31,011 cf	
Outflow	=	9.17 cfs @ 12.11 hrs, Volume= 31,011 cf, Atten= 0%, Lag= 0.0 min	

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



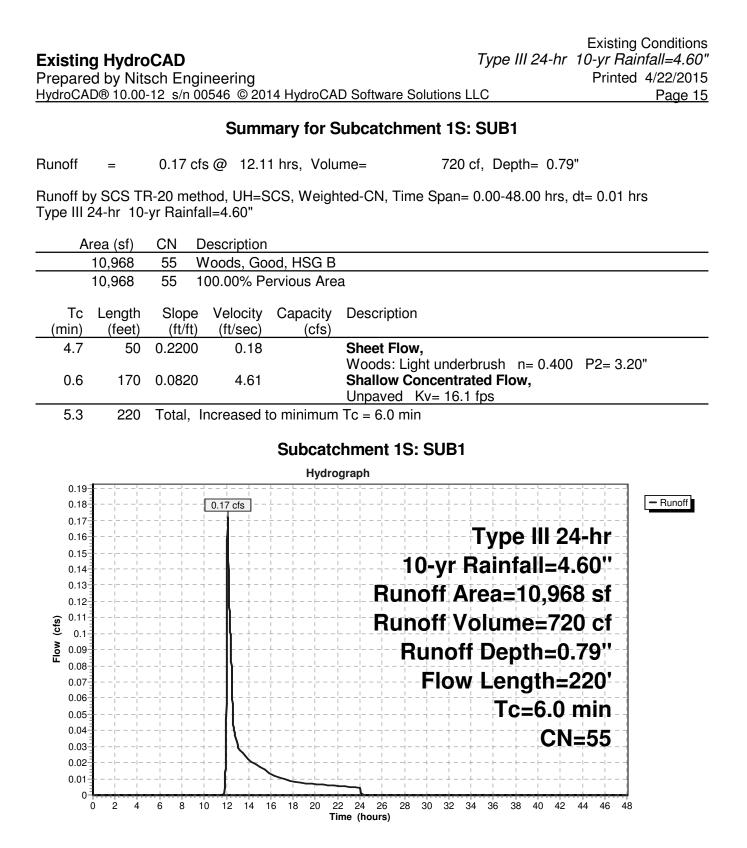
Reach 11R: DP2-Wells Avenue

		Existing Conditions
Existing HydroCAD	Type III 24-hr	10-yr Rainfall=4.60"
Prepared by Nitsch Engineering		Printed 4/22/2015
HydroCAD® 10.00-12 s/n 00546 © 2014 HydroCAD Software Solution	s LLC	Page 14

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: SUB1	Runoff Area=10,968 sf 0.00% Impervious Runoff Depth=0.79" Flow Length=220' Tc=6.0 min CN=55 Runoff=0.17 cfs 720 cf
Subcatchment 2S: SUB2	Runoff Area=147,391 sf 48.22% Impervious Runoff Depth=2.29" Flow Length=559' Tc=7.5 min CN=77 Runoff=8.61 cfs 28,153 cf
Subcatchment 3S: SUB3	Runoff Area=47,383 sf 73.87% Impervious Runoff Depth=3.29" Flow Length=128' Tc=8.7 min CN=88 Runoff=3.76 cfs 12,992 cf
Subcatchment 4S: SUB4	Runoff Area=38,880 sf 89.06% Impervious Runoff Depth=3.91" Flow Length=83' Tc=6.0 min CN=94 Runoff=3.83 cfs 12,681 cf
Subcatchment 5S: SUB5	Runoff Area=113,982 sf 38.80% Impervious Runoff Depth=2.05" Flow Length=627' Tc=13.8 min CN=74 Runoff=4.85 cfs 19,468 cf
Reach 6R: DP1-Northwest Property	Inflow=0.17 cfs 720 cf Outflow=0.17 cfs 720 cf
Reach 10R: DP3-Eastern Property	Inflow=4.85 cfs 19,468 cf Outflow=4.85 cfs 19,468 cf
Reach 11R: DP2-Wells Avenue	Inflow=16.02 cfs 53,826 cf Outflow=16.02 cfs 53,826 cf

Total Runoff Area = 358,604 sf Runoff Volume = 74,014 cf Average Runoff Depth = 2.48" 48.43% Pervious = 173,672 sf 51.57% Impervious = 184,932 sf



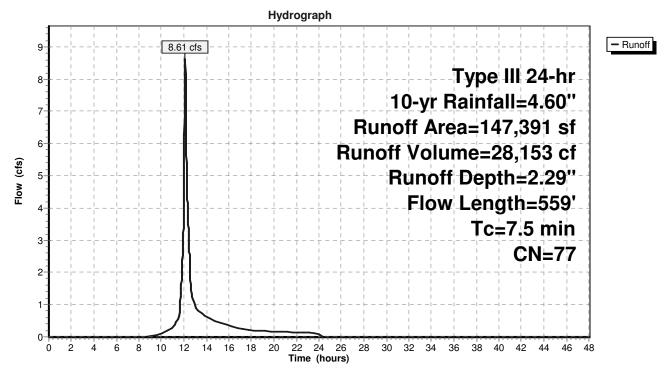
Summary for Subcatchment 2S: SUB2

Runoff	=	8.61 cfs @	12.11 hrs,	Volume=	28,153 cf, De	epth= 2.29"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=4.60"

	А	rea (sf)	CN	Desc	cription		
*		71,077	98	Pave	ed parki	ing	
		29,455	61	>75%	% Ġras	s cover, Go	ood, HSG B
_		46,859	55	Woo	ds, Go	od, HSG B	
	1	47,391	77	Weig	ghted A	verage	
		76,314	57			vious Area	
		71,077	98	48.2	2% Imp	ervious Are	ea
	Tc (min)	Length (feet)	Slop (ft/f		elocity ft/sec)	Capacity (cfs)	Description
	4.7	50	0.220	0	0.18		Sheet Flow,
	2.8	509	0.035	0	3.01		Woods: Light underbrush n= 0.400 P2= 3.20" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
	7.5	559	Total				

Subcatchment 2S: SUB2



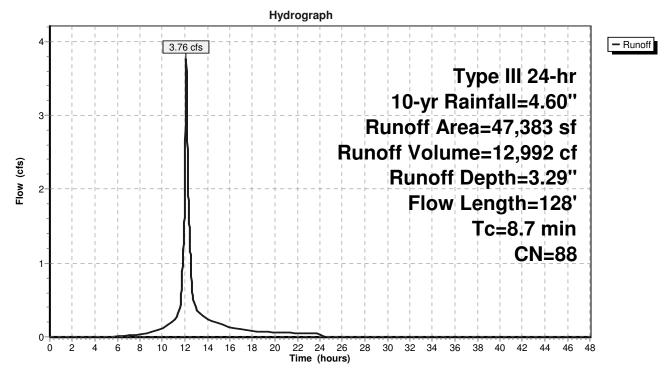
Summary for Subcatchment 3S: SUB3

Runoff = 3.76 cfs @ 12.12 hrs, Volume= 12,992 cf, Depth= 3.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=4.60"

	A	rea (sf)	CN	Des	cription		
*		686	98	Pav	ed parki	ing	
*		34,315	98	Roo	ofs	•	
_		12,382	61	>75	% Grass	s cover, Go	ood, HSG B
		47,383	88	Wei	ighted A	verage	
		12,382	61	26.1	13% Per	vious Area	
		35,001	98	73.8	37% Imp	ervious Are	ea
	Tc (min)	Length (feet)	Slop (ft/ft		/elocity (ft/sec)	Capacity (cfs)	Description
_	8.1	50	0.008	0	0.10		Sheet Flow,
	0.6	78	0.020	0	2.28		Grass: Short n= 0.150 P2= 3.20" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
	8.7	128	Total				

Subcatchment 3S: SUB3



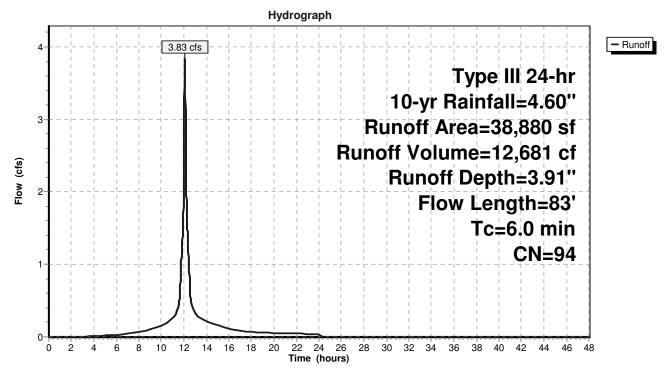
Summary for Subcatchment 4S: SUB4

Runoff = 3.83 cfs @ 12.08 hrs, Volume= 12,681 cf, Depth= 3.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=4.60"

_	A	vrea (sf)	CN	Description		
*		234	98	Paved park	ing	
*		34,393	98	Roofs	-	
_		4,253	61 :	>75% Gras	s cover, Go	bod, HSG B
		38,880	94	Weighted A	verage	
		4,253	61	10.94% Per	rvious Area	
		34,627	98	89.06% Imp	pervious Ar	ea
	Тс	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.4	50	0.0360	0.19		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.20"
	0.2	33	0.0240	2.49		Shallow Concentrated Flow,
_						Unpaved Kv= 16.1 fps
	4.6	83	Total,	Increased t	o minimum	ı Tc = 6.0 min

Subcatchment 4S: SUB4



Summary for Subcatchment 5S: SUB5

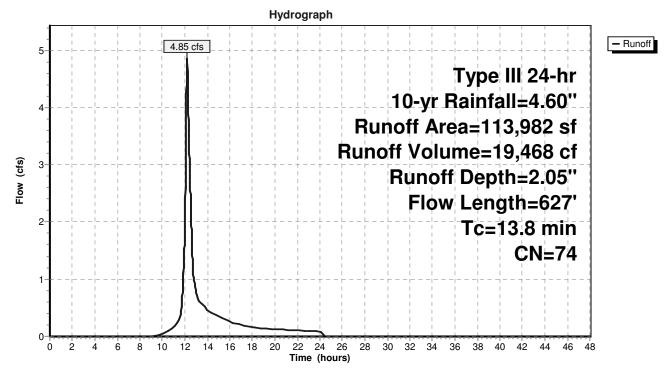
Runoff	=	4.85 cfs @	12.19 hrs,	Volume=	19,468 cf, Depth= 2.05"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=4.60"

_	A	rea (sf)	CN E	Description					
*		44,227	98 F	98 Paved parking					
		37,953	61 >	75% Gras	s cover, Go	bod, HSG B			
_		31,802	55 V	Voods, Go	od, HSG B				
	1	13,982	74 V	Veighted A	verage				
		69,755			vious Area				
		44,227	98 3	88.80% Imp	pervious Ar	ea			
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	6.5	50	0.0140	0.13		Sheet Flow,			
	7.2	542	0.0060	1.25		Grass: Short n= 0.150 P2= 3.20" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps			
	0.1	35	0.0860	4.72		Shallow Concentrated Flow,			
_						Unpaved Kv= 16.1 fps			
	100	~~~	— · ·						

13.8 627 Total

Subcatchment 5S: SUB5



Existing Conditions

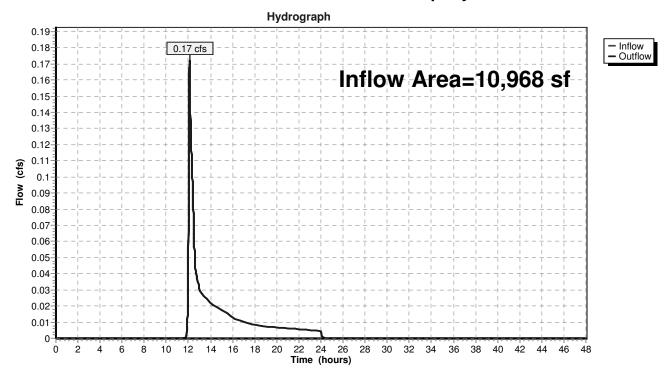
Printed 4/22/2015

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Inflow Area =		10,968 sf, (0.00% Impervious,	Inflow Depth = 0.79"	for 10-yr event
Inflow	=	0.17 cfs @ 12	2.11 hrs, Volume=	720 cf	
Outflow	=	0.17 cfs @ 12	2.11 hrs, Volume=	720 cf, Atter	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach 6R: DP1-Northwest Property



Summary for Reach 10R: DP3-Eastern Property

Inflow Are	a =	113,982 sf, 38.80% Imperviou	s, Inflow Depth = 2.05" for 10-yr event
Inflow	=	4.85 cfs @ 12.19 hrs, Volume	= 19,468 cf
Outflow	=	4.85 cfs @ 12.19 hrs, Volume	= 19,468 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Hydrograph Inflow Outflow 4.85 cfs 5 Inflow Area=113,982 sf 4 Flow (cfs) 3 2 1 0 22 24 26 Time (hours) 2 10 12 14 0 4 6 8 16 18 20 28 30 32 34 36 38 40 42 44 46 48

Reach 10R: DP3-Eastern Property

Summary for Reach 11R: DP2-Wells Avenue

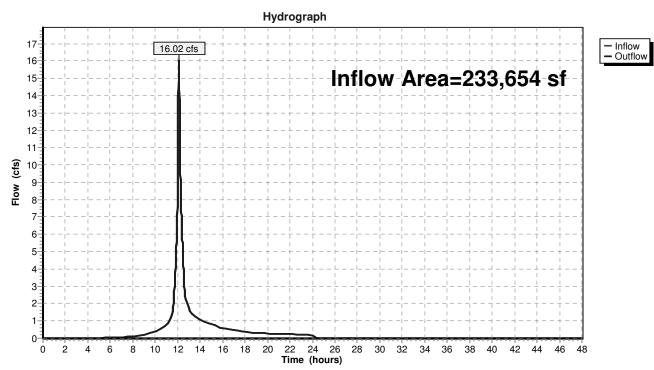
Existing Conditions

Printed 4/22/2015

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Inflow Are	ea =	233,654 sf, 60.22% Impervious, Inflow Depth = 2.76" for 10-yr event
Inflow	=	16.02 cfs @ 12.11 hrs, Volume= 53,826 cf
Outflow	=	16.02 cfs @ 12.11 hrs, Volume= 53,826 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



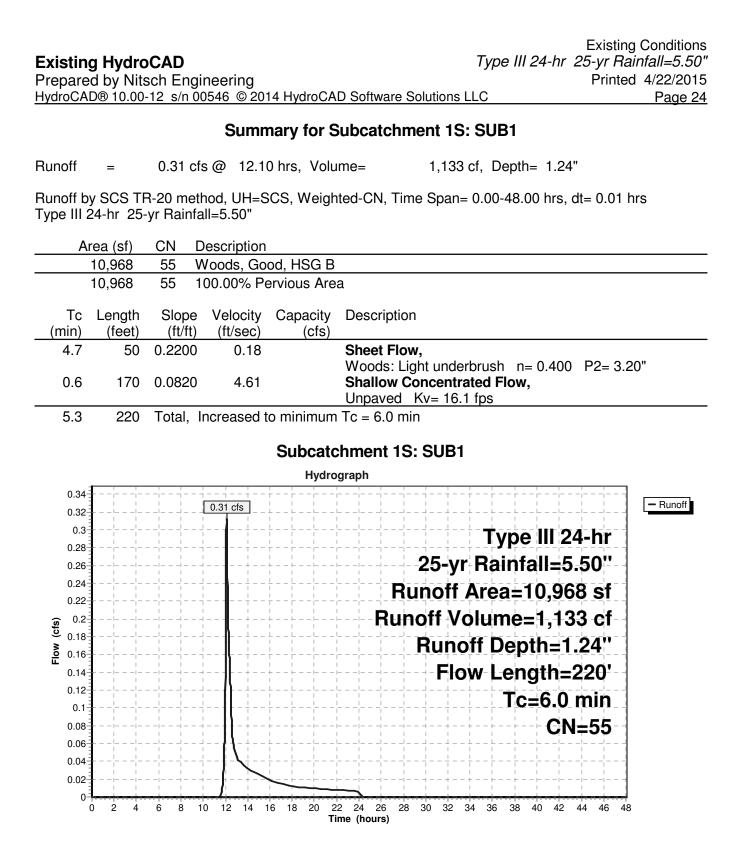
Reach 11R: DP2-Wells Avenue

	Existing Conditions
Existing HydroCAD	Type III 24-hr 25-yr Rainfall=5.50"
Prepared by Nitsch Engineering	Printed 4/22/2015
HydroCAD® 10.00-12 s/n 00546 © 2014 HydroCAD Software Solutions	LLC Page 23

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: SUB1	Runoff Area=10,968 sf 0.00% Impervious Runoff Depth=1.24" Flow Length=220' Tc=6.0 min CN=55 Runoff=0.31 cfs 1,133 cf
Subcatchment 2S: SUB2	Runoff Area=147,391 sf 48.22% Impervious Runoff Depth=3.05" Flow Length=559' Tc=7.5 min CN=77 Runoff=11.48 cfs 37,419 cf
Subcatchment 3S: SUB3	Runoff Area=47,383 sf 73.87% Impervious Runoff Depth=4.15" Flow Length=128' Tc=8.7 min CN=88 Runoff=4.69 cfs 16,370 cf
Subcatchment 4S: SUB4	Runoff Area=38,880 sf 89.06% Impervious Runoff Depth=4.80" Flow Length=83' Tc=6.0 min CN=94 Runoff=4.64 cfs 15,558 cf
Subcatchment 5S: SUB5	Runoff Area=113,982 sf 38.80% Impervious Runoff Depth=2.77" Flow Length=627' Tc=13.8 min CN=74 Runoff=6.62 cfs 26,303 cf
Reach 6R: DP1-Northwest Property	Inflow=0.31 cfs 1,133 cf Outflow=0.31 cfs 1,133 cf
Reach 10R: DP3-Eastern Property	Inflow=6.62 cfs 26,303 cf Outflow=6.62 cfs 26,303 cf
Reach 11R: DP2-Wells Avenue	Inflow=20.59 cfs 69,346 cf Outflow=20.59 cfs 69,346 cf

Total Runoff Area = 358,604 sf Runoff Volume = 96,782 cf Average Runoff Depth = 3.24" 48.43% Pervious = 173,672 sf 51.57% Impervious = 184,932 sf



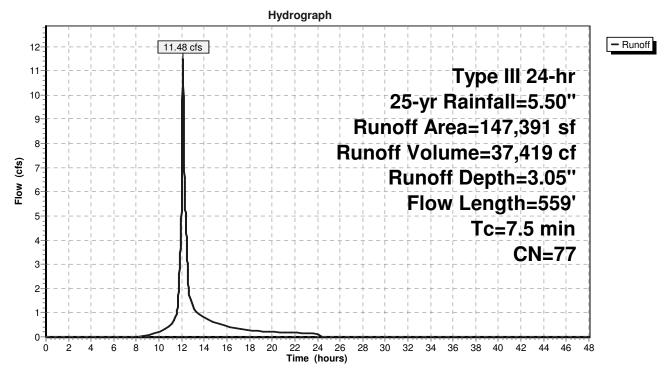
Summary for Subcatchment 2S: SUB2

Runoff =	= 11.48 cfs (2 12.11 hrs, Volume=	37,419 cf, Depth= 3.05"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.50"

	Δ	rea (sf)	CN	Description		
*		71,077	98	Paved park	ing	
		29,455	61	>75% Gras	s cover, Go	bod, HSG B
		46,859	55	Woods, Go	od, HSG B	
_	1	47,391	77	Weighted A	verage	
	76.314 57 51.78% Pervious Area					
		71,077	98	48.22% Imp	pervious Ar	ea
	Tc	Length	Slope	e Velocity	Capacity	Description
	(min)	(feet)	(ft/ft		(cfs)	
	4.7	50	0.2200	0.18		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.20"
	2.8	509	0.0350) 3.01		Shallow Concentrated Flow,
	1.0					Unpaved Kv= 16.1 fps
	7.5	559	Total			· · ·

Subcatchment 2S: SUB2



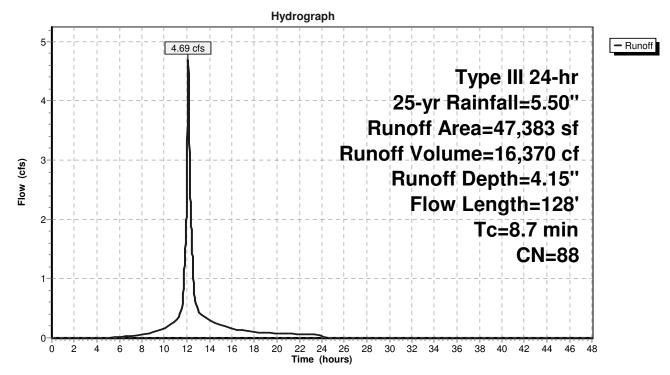
Summary for Subcatchment 3S: SUB3

Runoff = 4.69 cfs @ 12.12 hrs, Volume= 16,370 cf, Depth= 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.50"

	A	rea (sf)	CN	Descriptior	ı	
*		686	98	Paved park	king	
*		34,315	98	Roofs	-	
_		12,382	61	>75% Gras	s cover, Go	bod, HSG B
		47,383	88	Weighted A	Average	
		12,382	61	26.13% Pe	rvious Area	
		35,001	98	73.87% lm	pervious Ar	ea
	Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description
	8.1	50	0.008	0 0.10	· · ·	Sheet Flow,
	0.6	78	0.020	0 2.28		Grass: Short n= 0.150 P2= 3.20" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
	8.7	128	Total			

Subcatchment 3S: SUB3



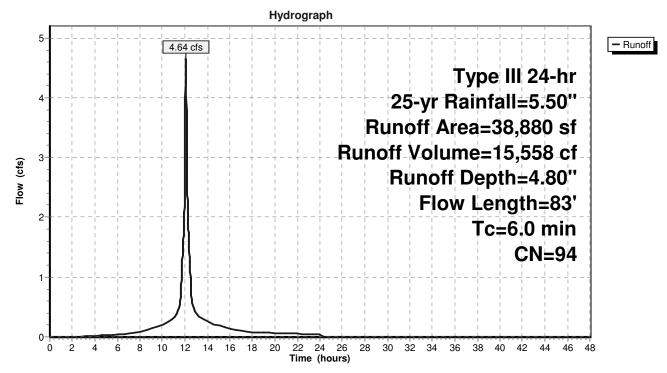
Summary for Subcatchment 4S: SUB4

Runoff = 4.64 cfs @ 12.08 hrs, Volume= 15,558 cf, Depth= 4.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.50"

	A	rea (sf)	CN	Description		
*		234	98	Paved park	ing	
*		34,393	98	Roofs	-	
_		4,253	61 :	>75% Gras	s cover, Go	bod, HSG B
		38,880	94	Weighted A	verage	
		4,253	61	10.94% Pei	rvious Area	
		34,627	98 8	89.06% Imp	pervious Ar	ea
	Tc	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.4	50	0.0360	0.19		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.20"
	0.2	33	0.0240	2.49		Shallow Concentrated Flow,
_						Unpaved Kv= 16.1 fps
	4.6	83	Total,	Increased t	o minimum	ı Tc = 6.0 min

Subcatchment 4S: SUB4



Summary for Subcatchment 5S: SUB5

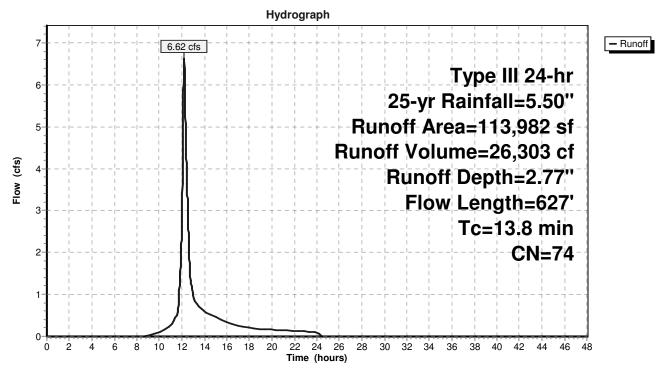
Runoff	=	6.62 cfs @	12.19 hrs,	Volume=	26,303 cf, Depth= 2.77"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.50"

_	A	rea (sf)	CN E	Description		
*		44,227	98 F	aved park	ing	
		37,953	61 >	75% Gras	s cover, Go	bod, HSG B
		31,802	55 V	Voods, Go	od, HSG B	
	113,982 74 Weighted Average					
	69,755 58 61.20% Pervious Area					
	44,227 98 38.80% Impervious Are			8.80% Imp	pervious Ar	ea
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	6.5	50	0.0140	0.13	()	Sheet Flow,
	0.0		0.0.0			Grass: Short n= 0.150 P2= 3.20"
	7.2	542	0.0060	1.25		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	0.1	35	0.0860	4.72		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps

13.8 627 Total

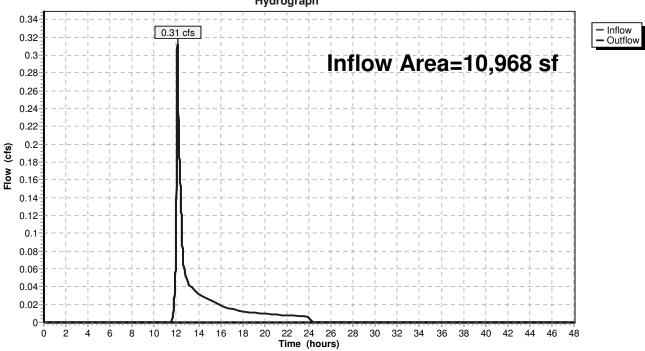
Subcatchment 5S: SUB5



Inflow Area =		10,968 sf,	0.00% Impervious,	Inflow Depth = 1.24"	for 25-yr event
Inflow	=	0.31 cfs @ 1	12.10 hrs, Volume=	1,133 cf	
Outflow	=	0.31 cfs @ 1	12.10 hrs, Volume=	1,133 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach 6R: DP1-Northwest Property Hydrograph

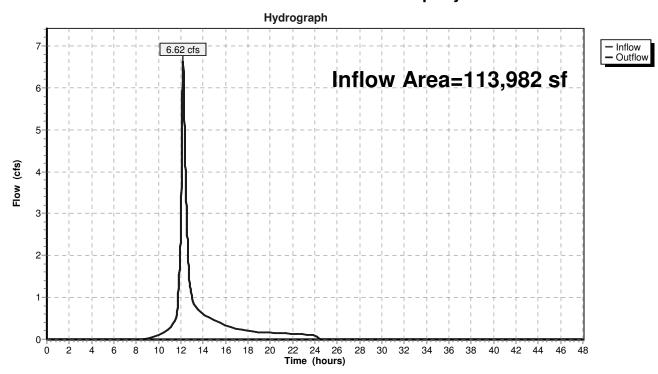


Existing Conditions

Summary for Reach 10R: DP3-Eastern Property

Inflow Are	a =	113,982 sf, 38.80% Impervious, Inflow Depth = 2.77" for 25-yr event	
Inflow	=	6.62 cfs @ 12.19 hrs, Volume= 26,303 cf	
Outflow	=	6.62 cfs @ 12.19 hrs, Volume= 26,303 cf, Atten= 0%, Lag= 0.0 r	min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach 10R: DP3-Eastern Property

Summary for Reach 11R: DP2-Wells Avenue

Existing Conditions

Printed 4/22/2015

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Inflow Are	ea =	233,654 sf, 60.22% Impervious, Inflow Depth = 3.56" for 25-yr event	233,654 sf,	
Inflow	=	20.59 cfs @ 12.10 hrs, Volume= 69,346 cf	20.59 cfs @	
Outflow	=	20.59 cfs @ 12.10 hrs, Volume= 69,346 cf, Atten= 0%, Lag= 0.0 min	20.59 cfs @	min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Hydrograph 23 22 Inflow Outflow 20.59 cfs 21 20 Inflow Area=233,654 sf 19-18-17 16-15-14 (sj) 13 12 **o I I I** 9-8-7. 6-5-4-3-2 1 0-10 22 24 26 Time (hours) 12 14 16 18 20 28 30 32 0 2 4 6 8 34 36 38 40 42 44 46 48

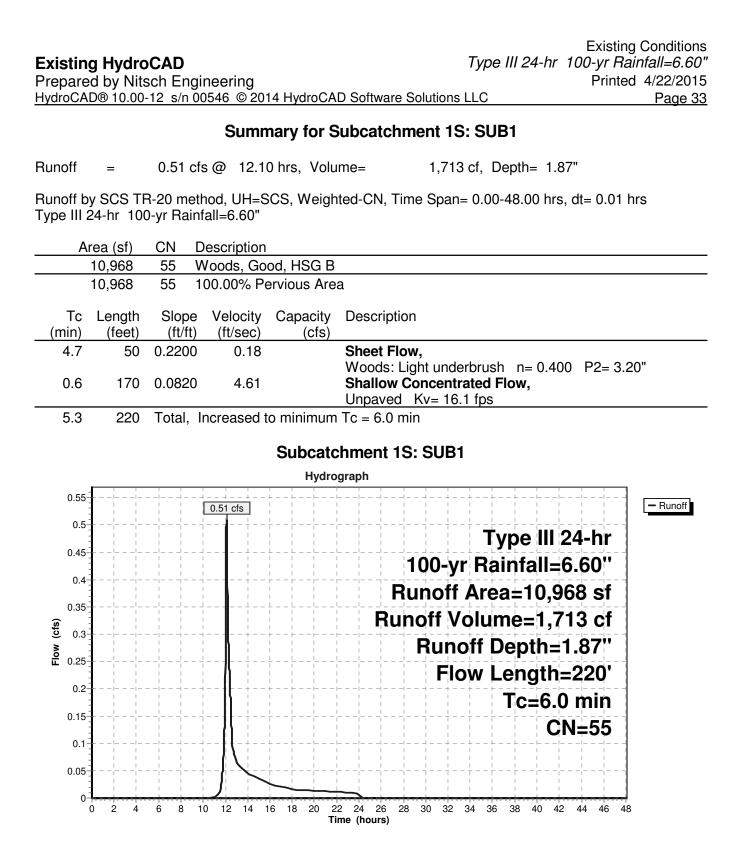
Reach 11R: DP2-Wells Avenue

		Existing (Conditions
Existing HydroCAD	Type III 24-hr	100-yr Rain	nfall=6.60"
Prepared by Nitsch Engineering		Printed	4/22/2015
HydroCAD® 10.00-12 s/n 00546 © 2014 HydroCAD Software Solutions	S LLC		Page 32

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: SUB1	Runoff Area=10,968 sf 0.00% Impervious Runoff Depth=1.87" Flow Length=220' Tc=6.0 min CN=55 Runoff=0.51 cfs 1,713 cf
Subcatchment 2S: SUB2	Runoff Area=147,391 sf 48.22% Impervious Runoff Depth=4.01" Flow Length=559' Tc=7.5 min CN=77 Runoff=15.07 cfs 49,230 cf
Subcatchment 3S: SUB3	Runoff Area=47,383 sf 73.87% Impervious Runoff Depth=5.21" Flow Length=128' Tc=8.7 min CN=88 Runoff=5.82 cfs 20,554 cf
Subcatchment 4S: SUB4	Runoff Area=38,880 sf 89.06% Impervious Runoff Depth=5.89" Flow Length=83' Tc=6.0 min CN=94 Runoff=5.63 cfs 19,088 cf
Subcatchment 5S: SUB5	Runoff Area=113,982 sf 38.80% Impervious Runoff Depth=3.70" Flow Length=627' Tc=13.8 min CN=74 Runoff=8.87 cfs 35,102 cf
Reach 6R: DP1-Northwest Property	Inflow=0.51 cfs 1,713 cf Outflow=0.51 cfs 1,713 cf
Reach 10R: DP3-Eastern Property	Inflow=8.87 cfs 35,102 cf Outflow=8.87 cfs 35,102 cf
Reach 11R: DP2-Wells Avenue	Inflow=26.26 cfs 88,872 cf Outflow=26.26 cfs 88,872 cf

Total Runoff Area = 358,604 sf Runoff Volume = 125,687 cf Average Runoff Depth = 4.21" 48.43% Pervious = 173,672 sf 51.57% Impervious = 184,932 sf



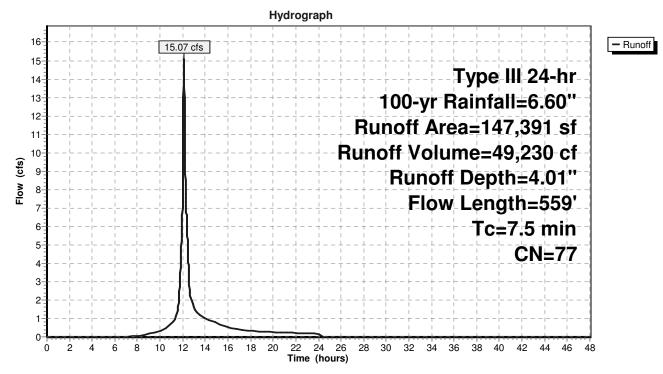
Summary for Subcatchment 2S: SUB2

Runoff	=	15.07 cfs @	12.11 hrs,	Volume=	49,230 cf, Depth= 4.01"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=6.60"

	А	rea (sf)	CN	De	escription						
*		71,077	98	Pa	Paved parking						
		29,455	61		>75% Grass cover, Good, HSG B						
		46,859	55	W	oods, Go	od, HSG B					
	1	47,391	77	W	eighted A	verage					
		76,314	57	-		vious Area					
		71,077 98 48.22% Impervious Are				pervious Ar	ea				
	Tc (min)	Length (feet)	Slop (ft/f		Velocity (ft/sec)	Capacity (cfs)	Description				
	4.7	50	0.220	0	0.18		Sheet Flow,				
	2.8	509	0.035	0	3.01		Woods: Light underbrush n= 0.400 P2= 3.20" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps				
	7.5	559	Total								

Subcatchment 2S: SUB2



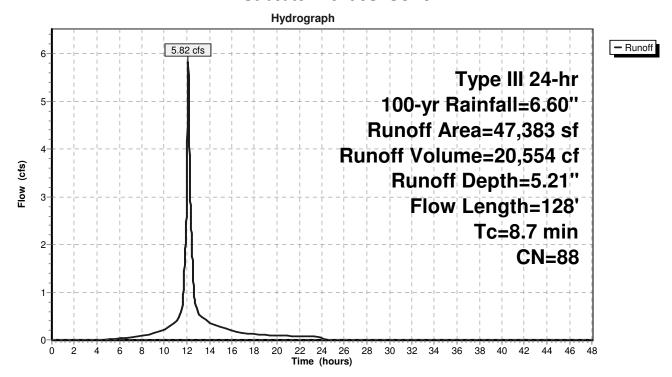
Summary for Subcatchment 3S: SUB3

Runoff = 5.82 cfs @ 12.12 hrs, Volume= 20,554 cf, Depth= 5.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=6.60"

	A	rea (sf)	CN	Description					
*		686	98	Paved parking					
*		34,315	98	Roofs	-				
		12,382	61	>75% Gras	s cover, Go	bod, HSG B			
		47,383	88	Weighted A	verage				
12,382 61 26.13% Pervious Area					vious Area				
		35,001	98	73.87% Imp	pervious Ar	ea			
					-				
	Тс	Length	Slope	,	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	8.1	50	0.0080	0.10		Sheet Flow,			
						Grass: Short n= 0.150 P2= 3.20"			
	0.6	78	0.0200	2.28		Shallow Concentrated Flow,			
_						Unpaved Kv= 16.1 fps			
	8.7	128	Total						

Subcatchment 3S: SUB3



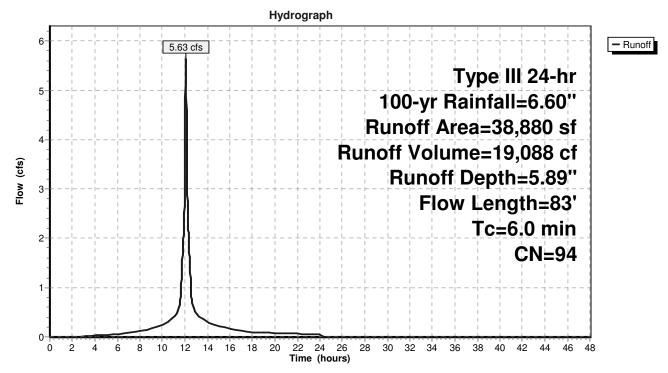
Summary for Subcatchment 4S: SUB4

Runoff = 5.63 cfs @ 12.08 hrs, Volume= 19,088 cf, Depth= 5.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=6.60"

_	A	rea (sf)	CN [Description		
*		234	98 F	Paved park	ing	
*		34,393	98 F	Roofs	-	
_		4,253	61 >	>75% Gras	s cover, Go	bod, HSG B
		38,880	94 \	Neighted A	verage	
		4,253	61 1	10.94% Per	vious Area	
		34,627	98 8	39.06% Imp	pervious Ar	ea
	Tc	Length	Slope		Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.4	50	0.0360	0.19		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.20"
	0.2	33	0.0240	2.49		Shallow Concentrated Flow,
_						Unpaved Kv= 16.1 fps
	4.6	83	Total,	Increased t	o minimum	ı Tc = 6.0 min

Subcatchment 4S: SUB4



Summary for Subcatchment 5S: SUB5

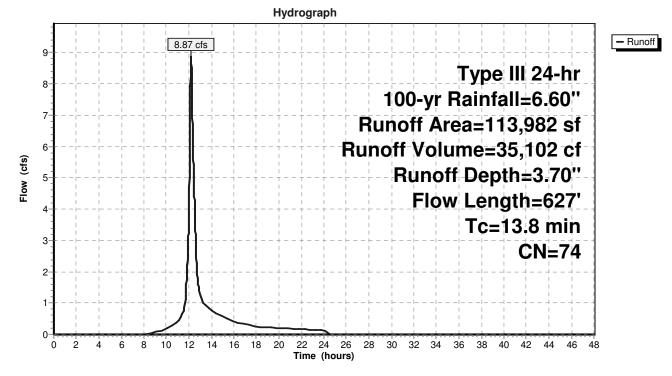
Runoff	=	8.87 cfs @	12.19 hrs,	Volume=	35,102 cf, Depth= 3.70"
--------	---	------------	------------	---------	-------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=6.60"

_	A	rea (sf)	CN D	escription		
*		44,227	98 F	aved park	ing	
		37,953	61 >	75% Gras	s cover, Go	bod, HSG B
		31,802	55 V	Voods, Go	od, HSG B	
	1	13,982	74 V	Veighted A	verage	
	69,755 58 61.20% Pervious Area					
	44,227 98 38.80% Impervious Are					ea
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.5	50	0.0140	0.13		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.20"
	7.2	542	0.0060	1.25		Shallow Concentrated Flow,
		-		-		Unpaved Kv= 16.1 fps
	0.1	35	0.0860	4.72		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps

13.8 627 Total

Subcatchment 5S: SUB5



Summary for Reach 6R: DP1-Northwest Property

Inflow Are	a =	10,968 sf,	0.00% Impervious,	Inflow Depth = 1.87"	for 100-yr event
Inflow	=	0.51 cfs @ 1	12.10 hrs, Volume=	1,713 cf	
Outflow	=	0.51 cfs @ 1	12.10 hrs, Volume=	1,713 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

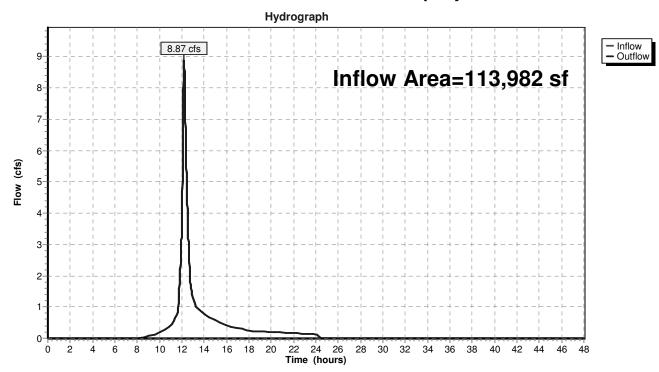
Hydrograph 0.55 Inflow Outflow 0.51 cfs 0.5 Inflow Area=10,968 sf 0.45 0.4 0.35 (cfs) 0.3 **NO I** 0.25 0.2 0.15 0.1 0.05 0-22 24 26 Time (hours) ż 12 0 4 6 8 10 14 16 18 20 28 30 32 34 36 38 40 42 44 46 48

Reach 6R: DP1-Northwest Property

Summary for Reach 10R: DP3-Eastern Property

Inflow Are	a =	113,982 sf, 38.80% Impervious, Inflow Depth = 3.70" for	100-yr event
Inflow	=	8.87 cfs @ 12.19 hrs, Volume= 35,102 cf	
Outflow	=	8.87 cfs @ 12.19 hrs, Volume= 35,102 cf, Atten= 0%	6, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

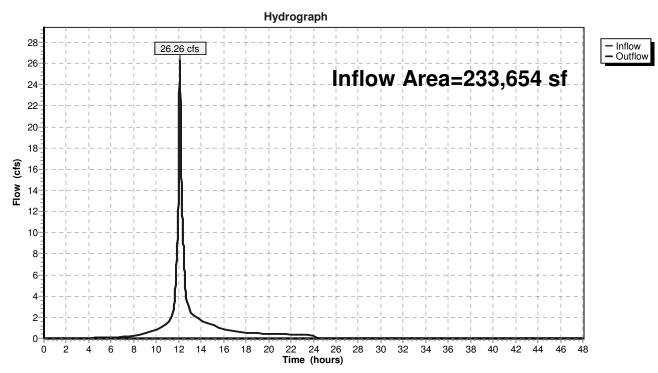


Reach 10R: DP3-Eastern Property

Summary for Reach 11R: DP2-Wells Avenue

Inflow Are	a =	233,654 sf, 60.22% Impervious, Inflow Depth = 4.56" for 100-yr event	
Inflow	=	26.26 cfs @ 12.10 hrs, Volume= 88,872 cf	
Outflow	=	26.26 cfs @ 12.10 hrs, Volume= 88,872 cf, Atten= 0%, Lag= 0.0 m	nin

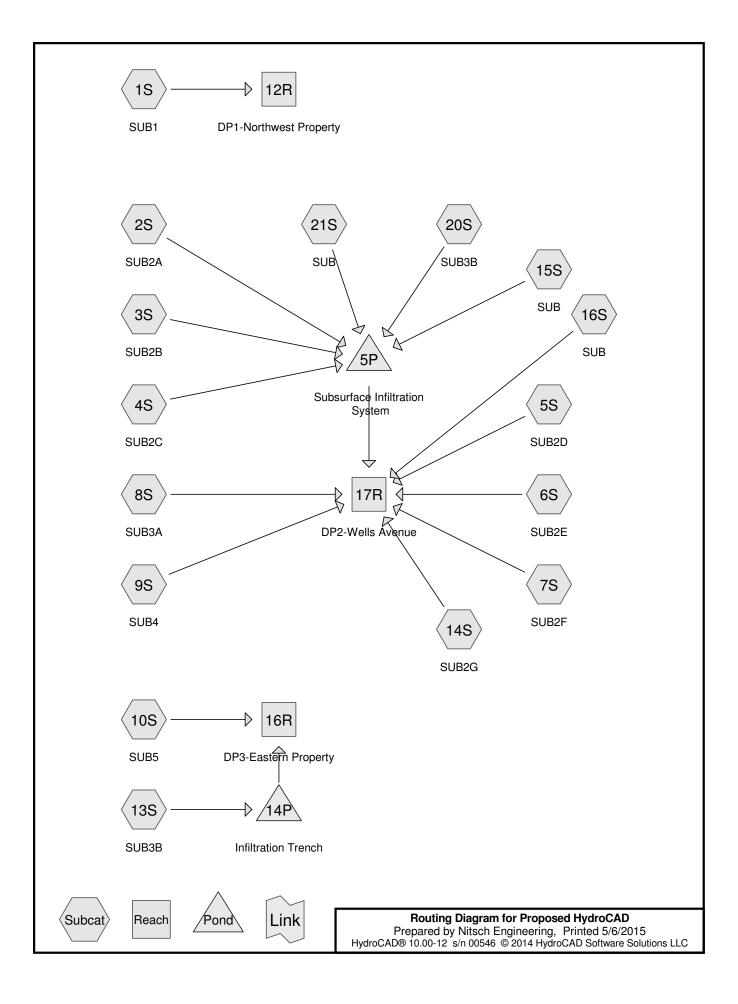
Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach 11R: DP2-Wells Avenue

APPENDIX C

Proposed Conditions – HydroCAD Calculations



Area Listing (selected nodes)

Area	ι CN	Description
(sq-ft)		(subcatchment-numbers)
111,518	8 61	>75% Grass cover, Good, HSG B (1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S, 9S, 10S, 13S, 14S, 20S, 21S)
4,541	98	Impervious (8S)
99,299	98	Paved parking (2S, 3S, 4S, 5S, 6S, 7S, 9S, 10S, 14S)
6,330	98	Paved parking, HSG B (21S)
16,489	98	Pavement (20S)
22,782	98	Roofs (9S)
50,999	98	Roofs, HSG B (13S, 15S, 16S)
46,646	55	Woods, Good, HSG B (1S, 4S, 6S, 10S, 14S)
358,604	81	TOTAL AREA

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Soil Listing (selected nodes)

Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
0	HSG A	
215,493	HSG B	1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S, 9S, 10S, 13S, 14S, 15S, 16S, 20S, 21S
0	HSG C	
0	HSG D	
143,111	Other	2S, 3S, 4S, 5S, 6S, 7S, 8S, 9S, 10S, 14S, 20S
358,604		TOTAL AREA

	Proposed Conditions
Proposed HydroCAD Prepared by Nitsch Engineering	Printed 5/6/2015
HydroCAD® 10.00-12 s/n 00546 © 2014 HydroCAD Software Solutions LLC	Page 4
Ground Covers (selected nodes)	

I	HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover	Sı Nı
	0	111,518	0	0	0	111,518	>75% Grass	
		_	_				cover, Good	
	0	0	0	0	4,541	4,541	Impervious	
	0	6,330	0	0	99,299	105,629	Paved parking	
	0	0	0	0	16,489	16,489	Pavement	
	0	50,999	0	0	22,782	73,781	Roofs	
	0	46,646	0	0	0	46,646	Woods, Good	
	0	215,493	0	0	143,111	358,604	TOTAL AREA	

Proposed HydroCAD	Proposed Conditions Type III 24-hr 2-yr Rainfall=3.20"
Prepared by Nitsch Engineering HydroCAD® 10.00-12 s/n 00546 © 2014 Hydro	Printed 5/6/2015
Runoff by SCS T	0-48.00 hrs, dt=0.01 hrs, 4801 points R-20 method, UH=SCS, Weighted-CN Trans method - Pond routing by Stor-Ind method
Subcatchment 1S: SUB1	Runoff Area=6,521 sf 0.00% Impervious Runoff Depth=0.31" Tc=6.0 min CN=57 Runoff=0.02 cfs 168 cf
Subcatchment 2S: SUB2A	Runoff Area=28,661 sf 77.18% Impervious Runoff Depth=2.17" Tc=6.0 min CN=90 Runoff=1.65 cfs 5,180 cf
Subcatchment 3S: SUB2B	Runoff Area=18,034 sf 89.00% Impervious Runoff Depth=2.54" Tc=6.0 min CN=94 Runoff=1.18 cfs 3,823 cf
Subcatchment 4S: SUB2C	Runoff Area=25,380 sf 63.98% Impervious Runoff Depth=1.68" Tc=6.0 min CN=84 Runoff=1.15 cfs 3,558 cf
Pond 5P: Subsurface Infiltration System Discarded=0.19 cfs	Peak Elev=100.48' Storage=8,888 cf Inflow=5.87 cfs 18,596 cf s 16,233 cf Primary=0.40 cfs 2,364 cf Outflow=0.58 cfs 18,596 cf
Subcatchment 5S: SUB2D	Runoff Area=12,647 sf 81.62% Impervious Runoff Depth=2.26" Tc=6.0 min CN=91 Runoff=0.76 cfs 2,380 cf
Subcatchment 6S: SUB2E	Runoff Area=10,382 sf 54.97% Impervious Runoff Depth=1.47" Tc=6.0 min CN=81 Runoff=0.41 cfs 1,271 cf
Subcatchment 7S: SUB2F	Runoff Area=17,153 sf 86.38% Impervious Runoff Depth=2.45" Tc=6.0 min CN=93 Runoff=1.09 cfs 3,496 cf
Subcatchment 8S: SUB3A	Runoff Area=14,967 sf 30.34% Impervious Runoff Depth=0.93" Tc=8.7 min CN=72 Runoff=0.31 cfs 1,160 cf
Subcatchment 9S: SUB4	Runoff Area=37,456 sf 75.33% Impervious Runoff Depth=2.08" Tc=6.0 min CN=89 Runoff=2.09 cfs 6,497 cf
Subcatchment 10S: SUB5	Runoff Area=93,190 sf 8.24% Impervious Runoff Depth=0.48" Tc=13.8 min CN=62 Runoff=0.63 cfs 3,735 cf
Reach 12R: DP1-Northwest Property	Inflow=0.02 cfs 168 cf Outflow=0.02 cfs 168 cf
Subcatchment 13S: SUB3B	Runoff Area=9,052 sf 98.80% Impervious Runoff Depth=2.97" Tc=6.0 min CN=98 Runoff=0.65 cfs 2,238 cf
Pond 14P: Infiltration Trench Discarded=0.0	Peak Elev=101.39' Storage=366 cf Inflow=0.65 cfs 2,238 cf 11 cfs 974 cf Primary=0.61 cfs 1,264 cf Outflow=0.62 cfs 2,238 cf
Subcatchment 14S: SUB2G	Runoff Area=15,595 sf 5.98% Impervious Runoff Depth=0.44" Tc=6.0 min CN=61 Runoff=0.12 cfs 577 cf
Subcatchment 15S: SUB	Runoff Area=2,500 sf 100.00% Impervious Runoff Depth=2.97" Tc=6.0 min CN=98 Runoff=0.18 cfs 618 cf

Page 6

Subcatchment 16S: SUB Runoff Area=39,556 sf 100.00% Impervious Runoff Depth=2.97" Tc=6.0 min CN=98 Runoff=2.82 cfs 9,782 cf

Reach 17R: DP2-Wells Avenue

Reach 16R: DP3-Eastern Property

Subcatchment 20S: SUB3B

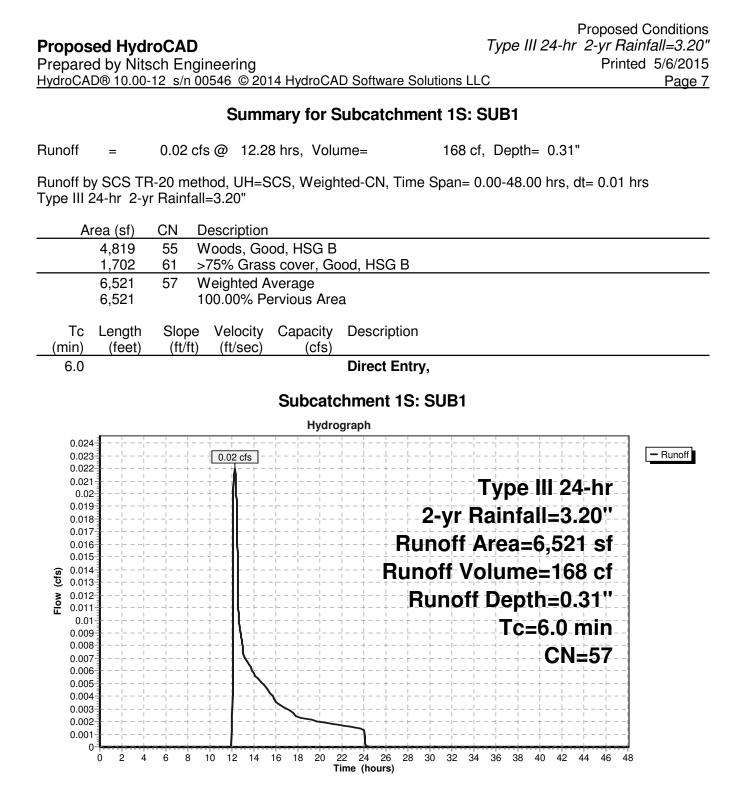
Subcatchment 21S: SUB

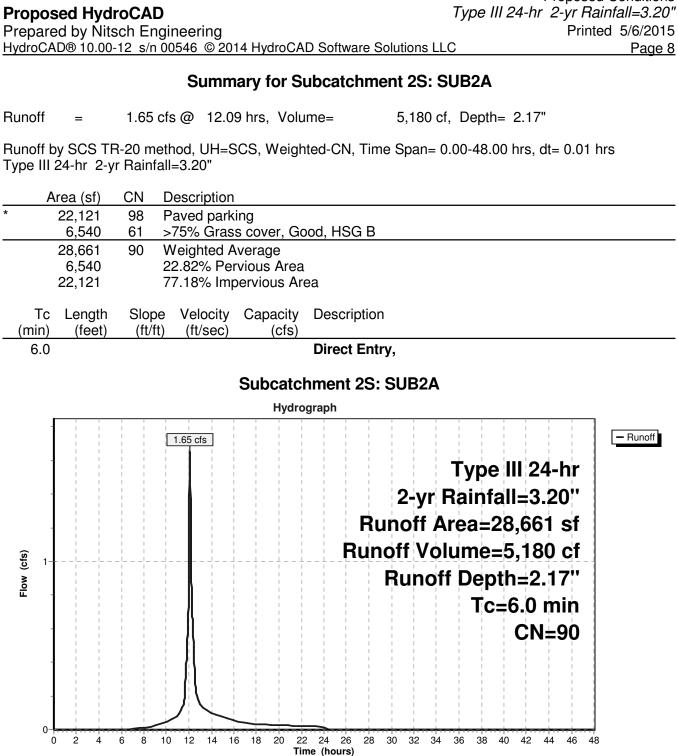
Inflow=7.54 cfs 27,526 cf Outflow=7.54 cfs 27,526 cf

Runoff Area=19,312 sf 85.38% Impervious Runoff Depth=2.45" Tc=6.0 min CN=93 Runoff=1.23 cfs 3,936 cf

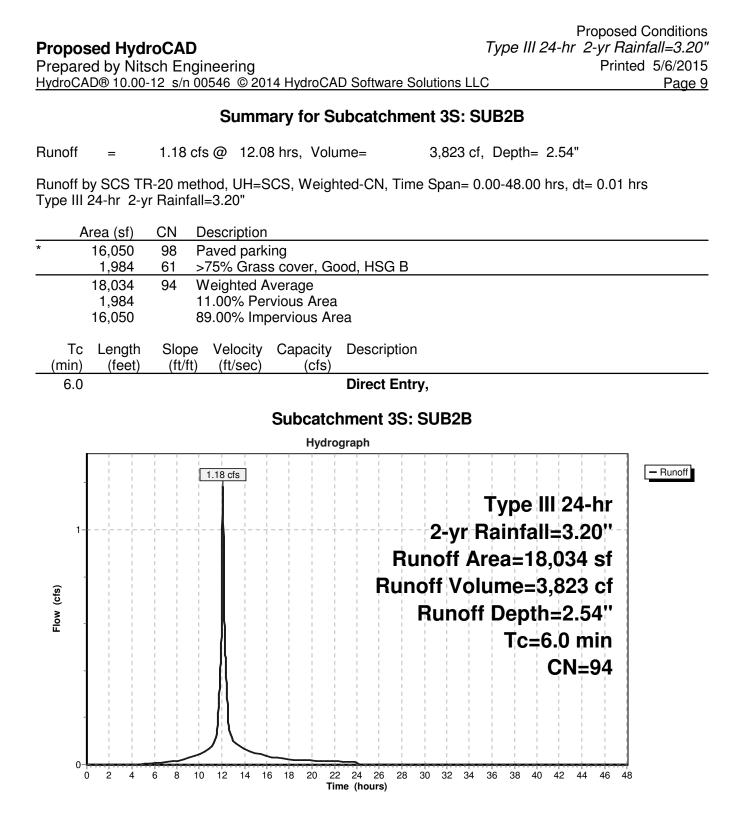
Runoff Area=8,198 sf 77.21% Impervious Runoff Depth=2.17" Tc=6.0 min CN=90 Runoff=0.47 cfs 1,482 cf

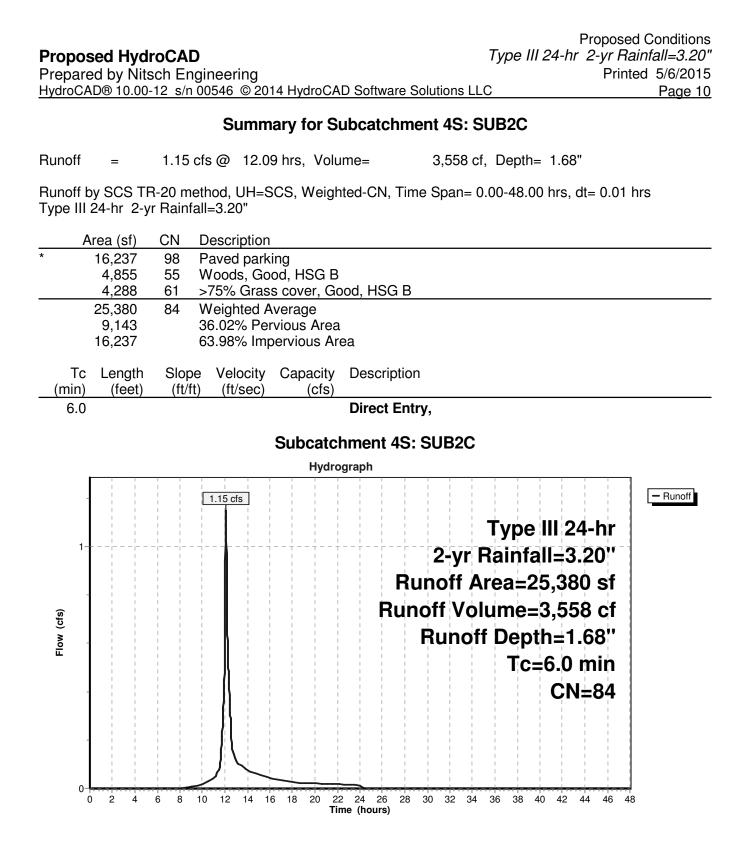
Total Runoff Area = 358,604 sf Runoff Volume = 49,901 cf Average Runoff Depth = 1.67" 44.11% Pervious = 158,164 sf 55.89% Impervious = 200,440 sf





Proposed Conditions





Summary for Pond 5P: Subsurface Infiltration System

Proposed Conditions

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Inflow Area =	102,085 sf, 78.10% Impervious,	Inflow Depth = 2.19" for 2-yr event
Inflow =	5.87 cfs @ 12.09 hrs, Volume=	18,596 cf
Outflow =	0.58 cfs @ 12.93 hrs, Volume=	18,596 cf, Atten= 90%, Lag= 50.7 min
Discarded =	0.19 cfs @ 10.33 hrs, Volume=	16,233 cf
Primary =	0.40 cfs @ 12.93 hrs, Volume=	2,364 cf

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 100.48' @ 12.93 hrs Surf.Area= 7,912 sf Storage= 8,888 cf

Plug-Flow detention time= 389.0 min calculated for 18,593 cf (100% of inflow) Center-of-Mass det. time= 389.0 min (1,191.5 - 802.5)

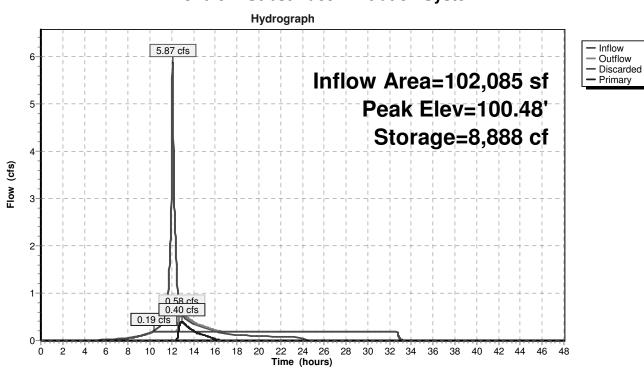
Volume	Invert	Avail.Storage	Storage Description
#1A	98.80'	6,077 cf	68.00'W x 116.36'L x 3.50'H Field A
			27,694 cf Overall - 10,330 cf Embedded = 17,363 cf x 35.0% Voids
#2A	99.30'	10,330 cf	ADS_StormTech SC-740 x 224 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			Row Length Adjustment= +0.44' x 6.45 sf x 14 rows
		16,407 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	98.80'	1.020 in/hr Exfiltration over Horizontal area
#2	Primary	99.30'	12.0" Round Culvert out of OCS
			L= 95.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 99.30' / 98.40' S= 0.0095 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	100.40'	5.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Discarded OutFlow Max=0.19 cfs @ 10.33 hrs HW=98.84' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.19 cfs)

Primary OutFlow Max=0.39 cfs @ 12.93 hrs HW=100.48' (Free Discharge) -2=Culvert out of OCS (Passes 0.39 cfs of 2.76 cfs potential flow) **1**-3=Sharp-Crested Rectangular Weir (Weir Controls 0.39 cfs @ 0.95 fps)



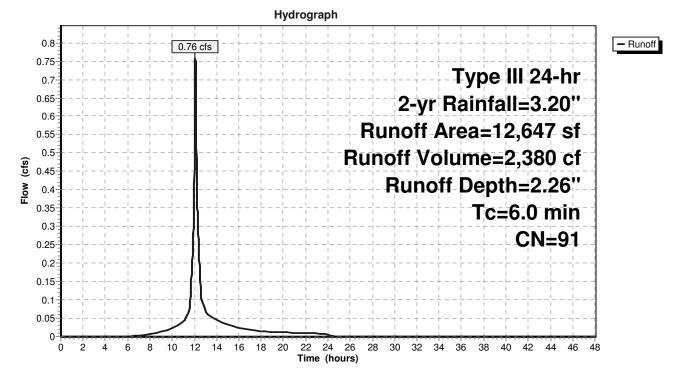
Pond 5P: Subsurface Infiltration System

Runoff	_	0.76 cfs @	12 09 hrs	Volume-	2 380 cf	Depth= 2.26"
nulioli	=	0.70 CIS @	12.09 115,	volume=	2,300 01,	Deptn= 2.20

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.20"

	А	rea (sf)	CN	Description		
*		10,322	98	Paved park	ing	
_		2,325	61	>75% Ġras	s cover, Go	bod, HSG B
		12,647	91	Weighted A	verage	
		2,325		18.38% Pe	rvious Area	
		10,322		81.62% Imp	pervious Ar	ea
	Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description
	6.0					Direct Entry,

Subcatchment 5S: SUB2D



Proposed Conditions

Proposed Conditions Type III 24-hr 2-yr Rainfall=3.20" Proposed HydroCAD Prepared by Nitsch Engineering Printed 5/6/2015 HydroCAD® 10.00-12 s/n 00546 © 2014 HydroCAD Software Solutions LLC Page 14 Summary for Subcatchment 6S: SUB2E Runoff 0.41 cfs @ 12.09 hrs, Volume= 1,271 cf, Depth= 1.47" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.20" Area (sf) CN Description 5,707 98 Paved parking 4,001 61 >75% Grass cover, Good, HSG B 674 55 Woods, Good, HSG B 10,382 81 Weighted Average 4,675 45.03% Pervious Area 5,707 54.97% Impervious Area Velocity Capacity Тс Length Slope Description (feet) (ft/ft) (ft/sec) (cfs) (min) **Direct Entry**, 6.0 Subcatchment 6S: SUB2E Hydrograph 0.44 - Runoff 0.41 cfs 0.42 0.4 Type III 24-hr 0.38 0.36 2-yr Rainfall=3.20" 0.34 0.32 Runoff Area=10,382 sf 0.3 0.28 Runoff Volume=1,271 cf 0.26 (cfs) 0.24 Runoff Depth=1.47" Flow 0.22 0.2 Tc=6.0 min 0.18 0.16 **CN=81** 0.14 0.12 0.1 0.08 0.06 0.04 0.02 0

28 30 32 34

40 42

44 46 48

36 38

2

4 6 8

0

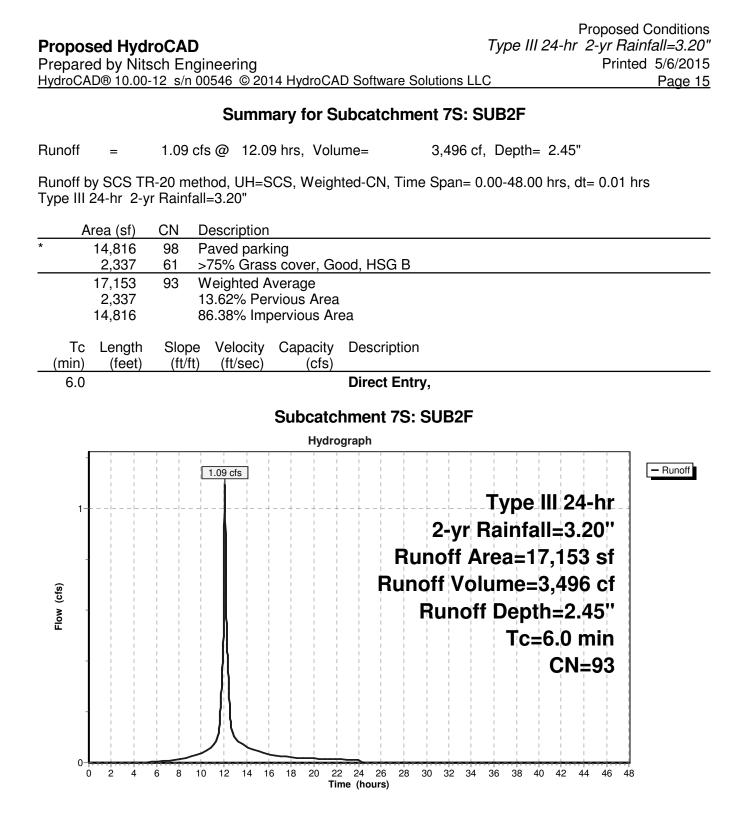
10

12 14 16 18

20

22 24 26

Time (hours)

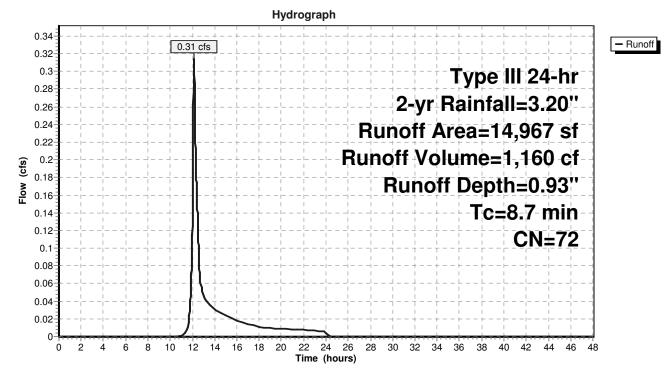


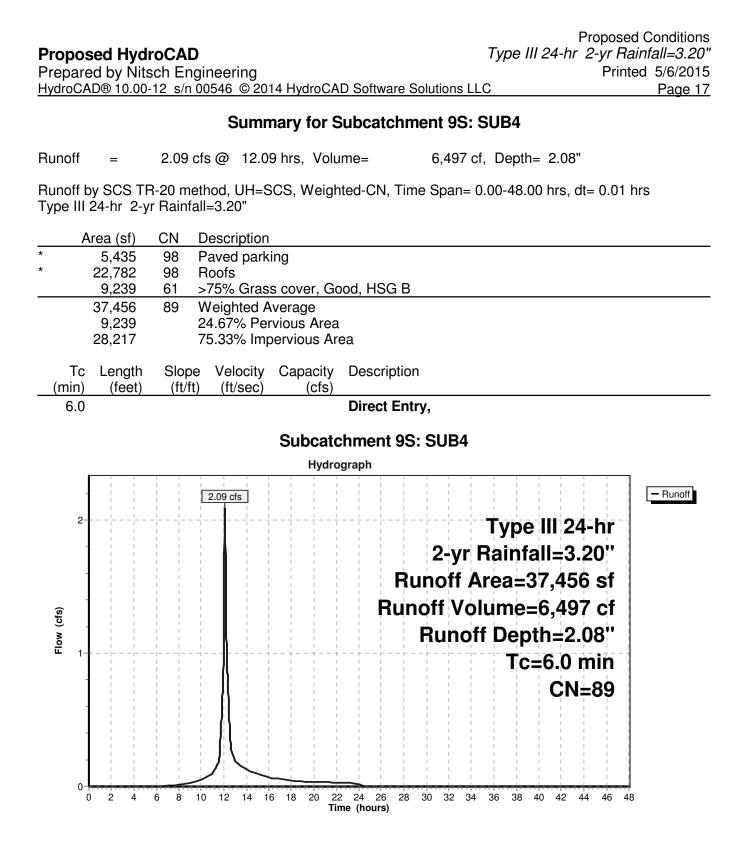
Runoff	=	0.31 cfs @	12.13 hrs.	Volume=	1.160 cf.	Depth= 0.93"
riunon	_	0.01 013 @	12.101113,	volume-	1,100 01,	Dopin= 0.00

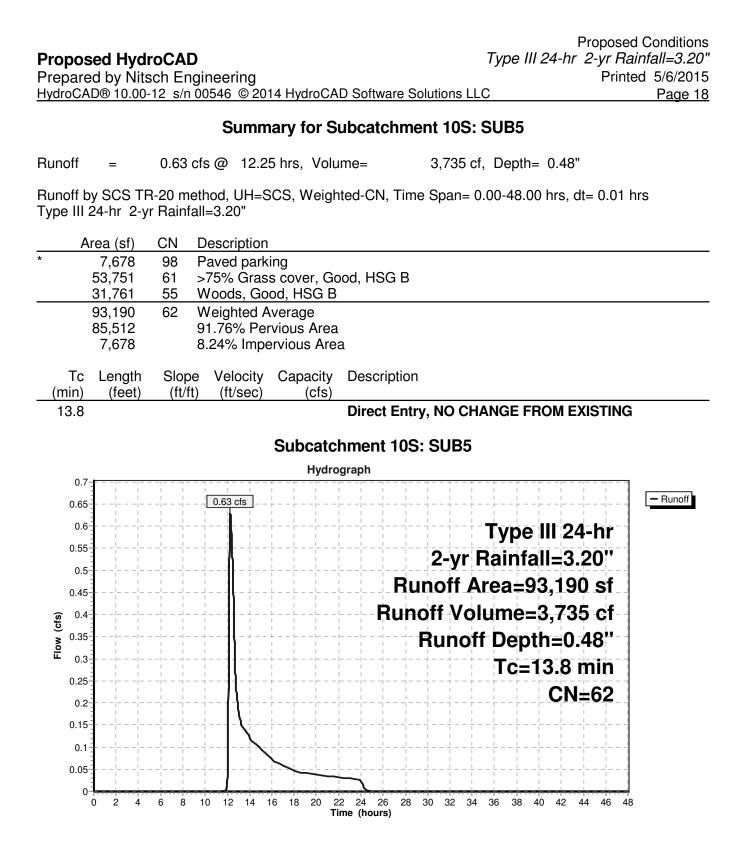
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.20"

_	A	Area (sf)	CN	Description		
*		4,541	98	Impervious		
_		10,426	61	>75% Gras	s cover, Go	bod, HSG B
		14,967	72	Weighted A	verage	
		10,426		69.66% Pe	rvious Area	
		4,541		30.34% Imp	pervious Ar	ea
	-		01		A	
	Tc	- 3-	Slope	,	Capacity	Description
_	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	8.7					Direct Entry, NO CHANGE FROM EXISTING
						-

Subcatchment 8S: SUB3A







Summary for Reach 12R: DP1-Northwest Property

Inflow Area =		6,521 sf,	0.00% Impervious,	Inflow Depth = 0.31" for 2-yr event	
Inflow	=	0.02 cfs @ 1	12.28 hrs, Volume=	168 cf	
Outflow	=	0.02 cfs @ 1	12.28 hrs, Volume=	168 cf, Atten= 0%, Lag= 0.0 min	

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

12 14

16 18

20

0.001 0

0 2 4 6 8 10

Hydrograph 0.024 Inflow Outflow 0.023 0.02 cfs 0.022 Inflow Area=6,521 sf 0.021 0.02 0.019 0.018-0.017 0.016 0.015 0.014 (s) 0.014 0.013 **8** 0.012 0.011 0.01 0.009-0.008 0.007 0.006 0.005 0.004 0.003 0.002-

22 24 26 Time (hours)

28

30 32

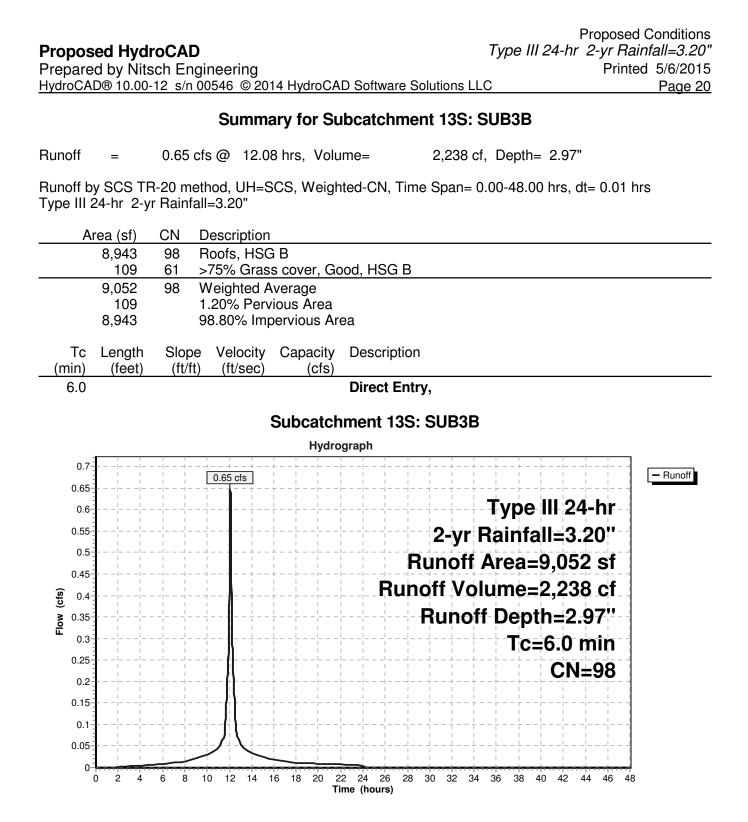
34 36 38 40 42

44 46 48

Reach 12R: DP1-Northwest Property

Proposed Conditions

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Summary for Pond 14P: Infiltration Trench

Inflow Area =	9,052 sf, 98.80% Impervious,	Inflow Depth = 2.97" for 2-yr event
Inflow =	0.65 cfs @ 12.08 hrs, Volume=	2,238 cf
Outflow =	0.62 cfs @ 12.11 hrs, Volume=	2,238 cf, Atten= 4%, Lag= 1.4 min
Discarded =	0.01 cfs @ 7.14 hrs, Volume=	974 cf
Primary =	0.61 cfs @ 12.11 hrs, Volume=	1,264 cf

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 101.39' @ 12.11 hrs Surf.Area= 458 sf Storage= 366 cf

Plug-Flow detention time= 118.1 min calculated for 2,238 cf (100% of inflow) Center-of-Mass det. time= 118.2 min (874.6 - 756.4)

Volume	Invert	Avail.Stor	rage	Storage I	Description		
#1	99.50'	60)5 cf	Stone (P	Stone (Prismatic) Listed below (Recalc)		
#2	#2 101.00' 90 c		90 cf	,	ound Pipe Stora	mbedded = 1,513 cf x 40.0% Voids ige Inside #1	
		69	95 cf	Total Ava	ailable Storage		
Elevatio (fee		urf.Area (sq-ft)		.Store c-feet)	Cum.Store (cubic-feet)		
99.5	50	458		0	0		
103.0	0	458		1,603	1,603		
Device	Routing	Invert	Outle	et Devices	5		
#1	Primary	101.00' 99.50'	L= 52 Inlet n= 0.	/ Outlet In .013 Corr	, square edge h ivert= 101.00' /	eadwall, Ke= 0.500 100.20' S= 0.0154 '/' Cc= 0.900 both interior, Flow Area= 0.79 sf Surface area	
			~ -				

Discarded OutFlow Max=0.01 cfs @ 7.14 hrs HW=99.54' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.61 cfs @ 12.11 hrs HW=101.39' (Free Discharge) **1=Culvert** (Inlet Controls 0.61 cfs @ 2.13 fps)

Proposed HydroCAD

0-

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2

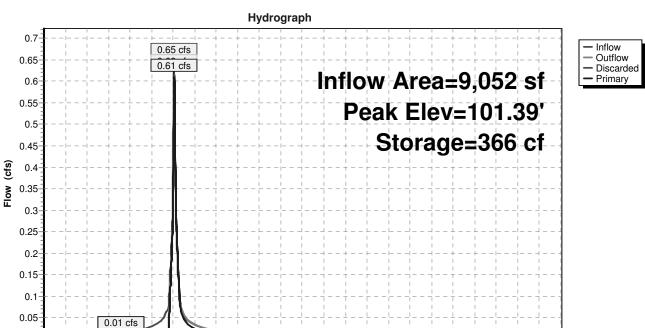
4 6 8

10

12 14

16 18 20

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22 24 26 Time (hours)

28 30

32 34

36 38

40 42 44 46

48

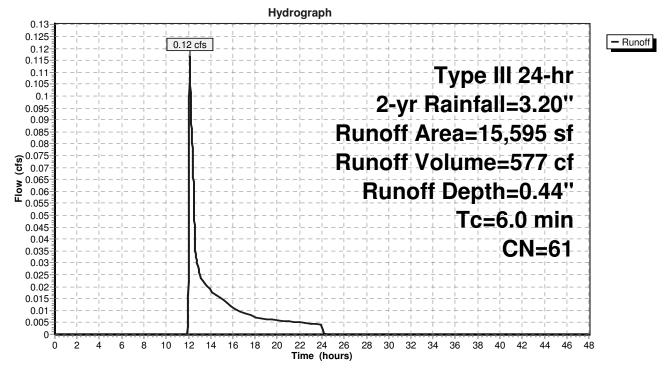
Pond 14P: Infiltration Trench

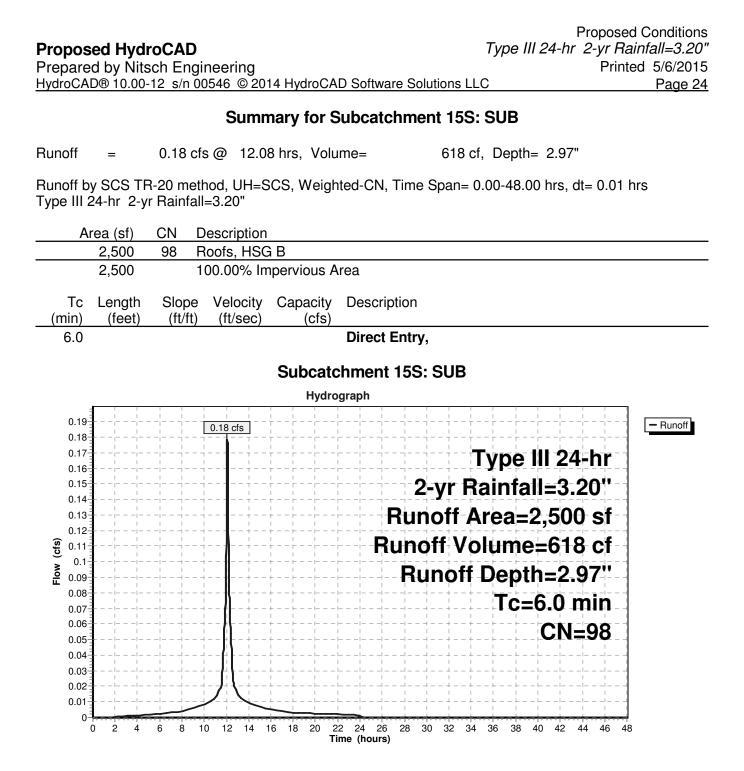
Summary for Subcatchment 14S: SUB2G

Runoff = 0.12 cfs @ 12.12 hrs, Volume= 577 cf, Depth= 0.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.20"

	A	rea (sf)	CN	Description	1			
*		933	98	Paved park	Paved parking			
		10,125	61	>75% Gras	s cover, Go	ood, HSG B		
		4,537	55	Woods, Go	od, HSG B	3		
		15,595	61	Weighted Average				
		14,662		94.02% Pe	rvious Area	1		
		933		5.98% Impe	ervious Area	a		
	_							
,	Tc	Length	Slop	•	Capacity	Description		
(n	nin)	(feet)	(ft/f	t) (ft/sec)	(cfs)			
	6.0					Direct Entry,		
Subcatchment 14S: SUB2G								



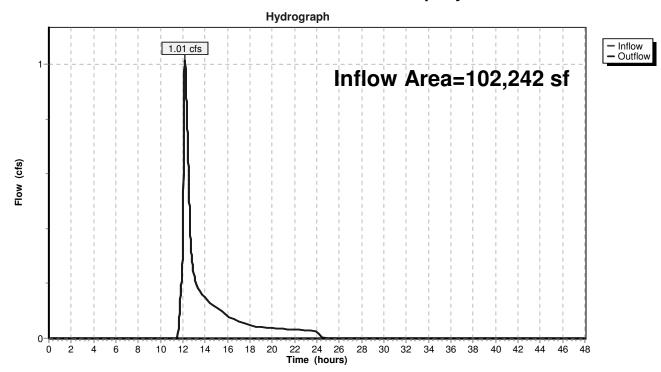


Summary for Reach 16R: DP3-Eastern Property

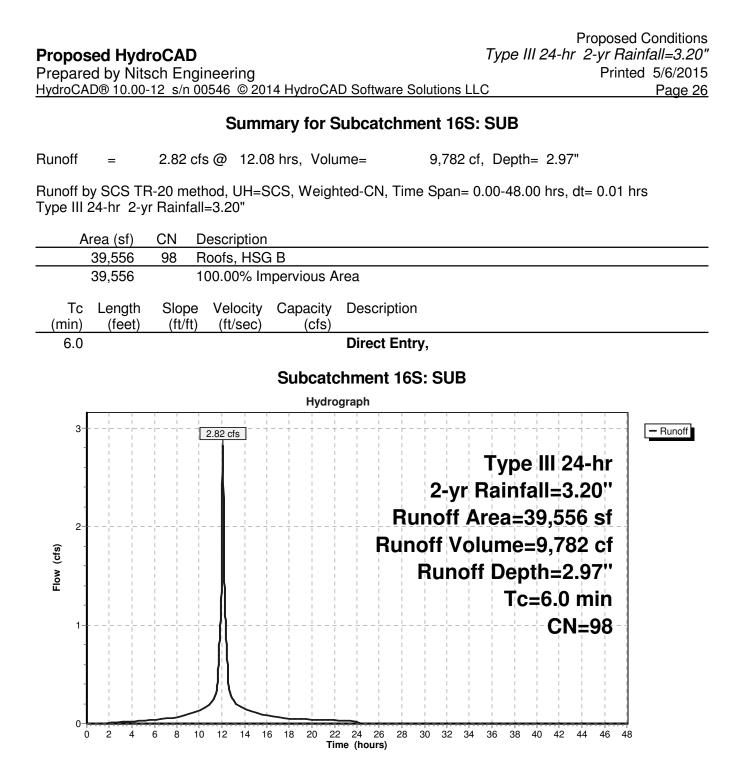
Page 25

Inflow Are	a =	102,242 sf,	16.26% Imperviou	s, Inflow Depth =	0.59" for 2-yr event
Inflow	=	1.01 cfs @	12.17 hrs, Volume	= 5,000 cf	
Outflow	=	1.01 cfs @	12.17 hrs, Volume	= 5,000 cl	f, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach 16R: DP3-Eastern Property



Summary for Reach 17R: DP2-Wells Avenue

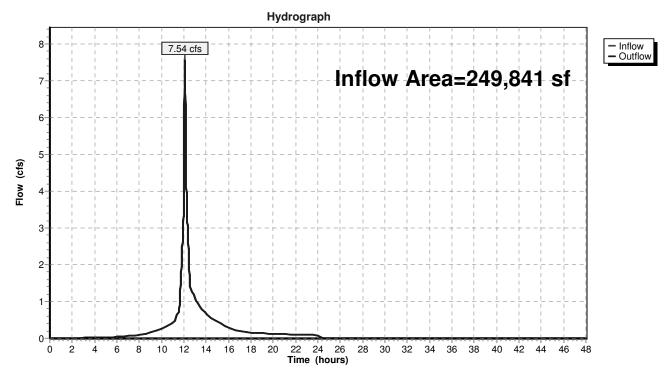
Proposed Conditions

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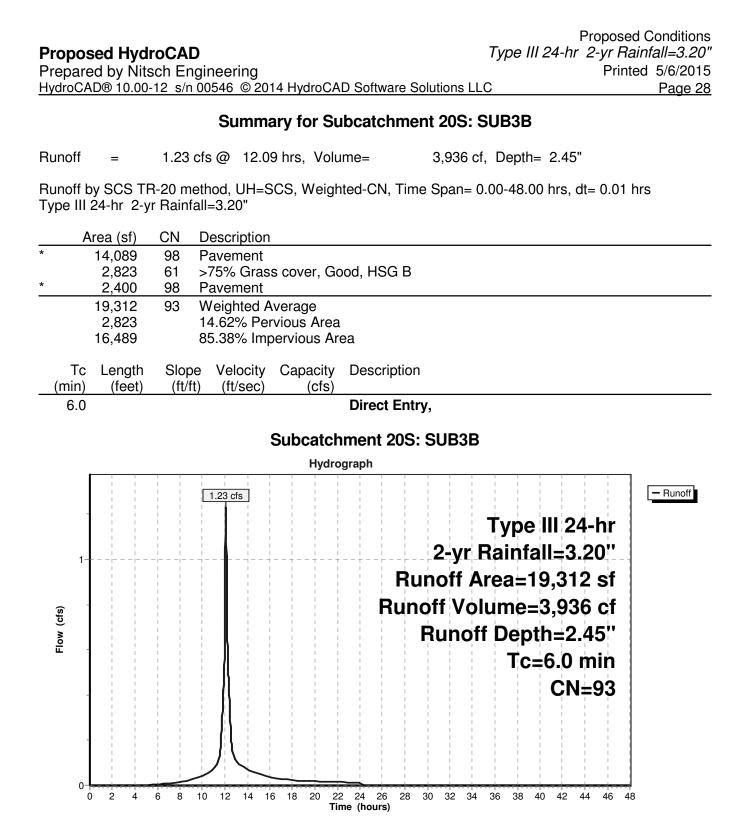
Page 27

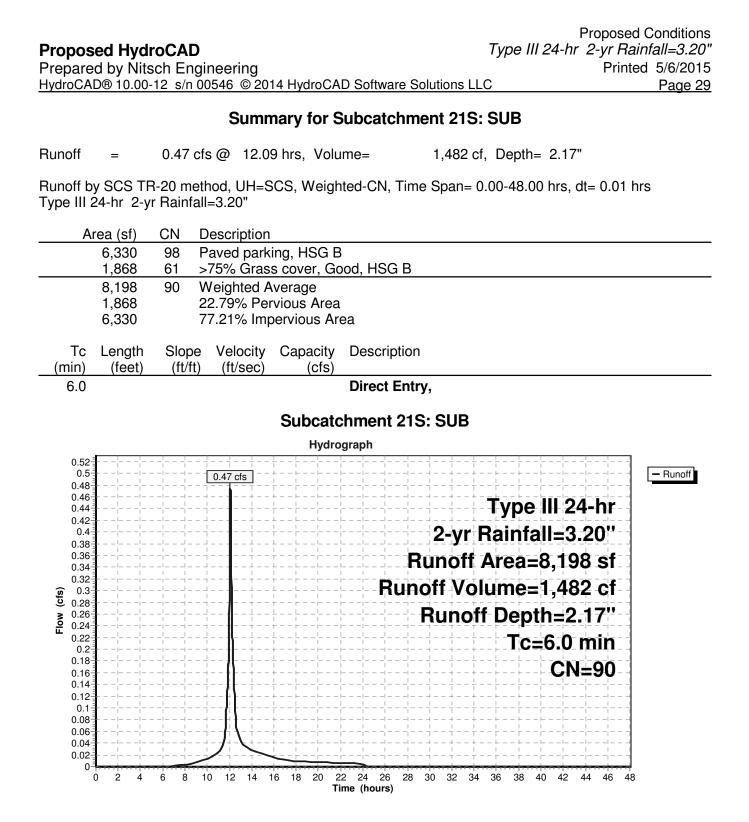
Inflow Area =	249,841 sf,	73.57% Impervious,	Inflow Depth = 1.32"	for 2-yr event
Inflow =	7.54 cfs @	12.09 hrs, Volume=	27,526 cf	
Outflow =	7.54 cfs @	12.09 hrs, Volume=	27,526 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach 17R: DP2-Wells Avenue

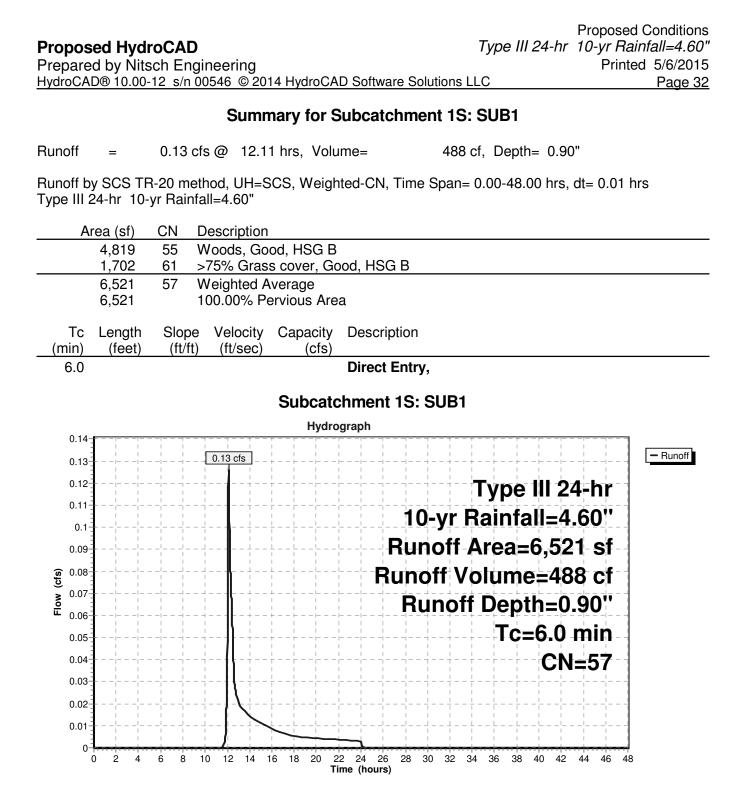


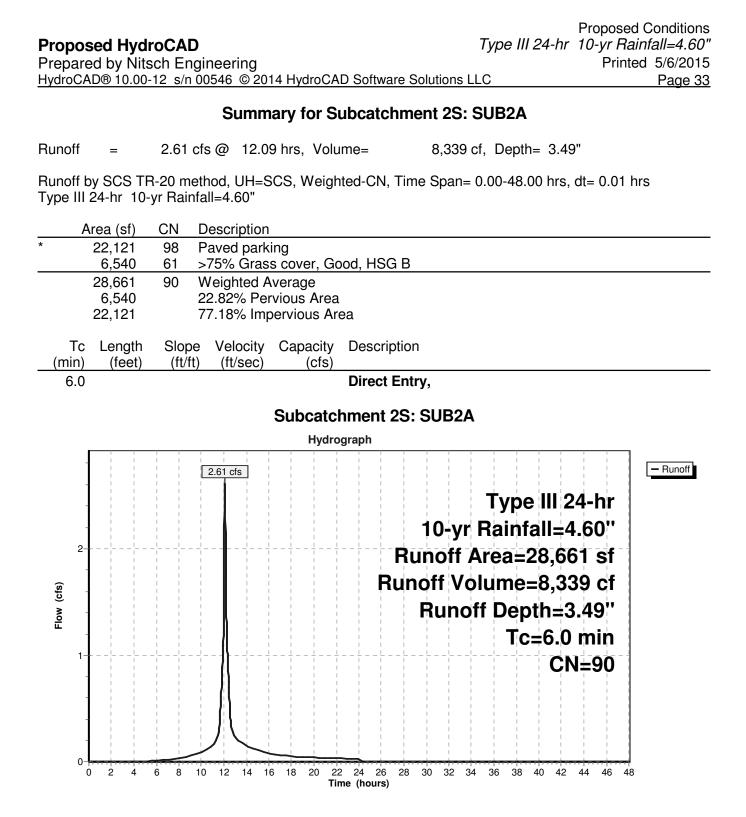


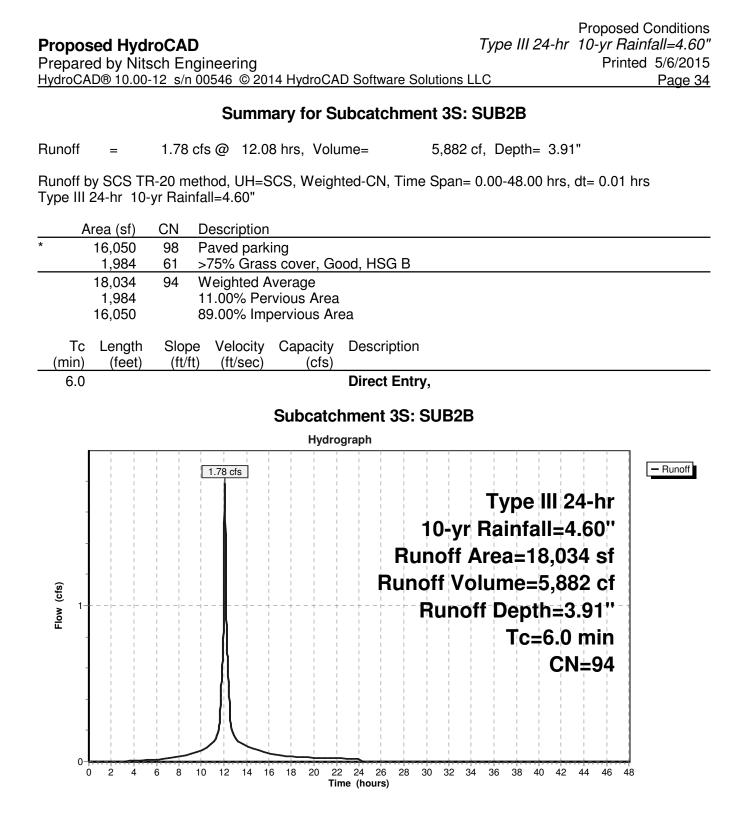
Proposed HydroCAD Prepared by Nitsch Engineering HydroCAD® 10.00-12 s/n 00546 © 2014 Hy	Proposed Conditions <i>Type III 24-hr 10-yr Rainfall=4.60"</i> Printed 5/6/2015 ydroCAD Software Solutions LLC Page 30
Runoff by SCS	00-48.00 hrs, dt=0.01 hrs, 4801 points TR-20 method, UH=SCS, Weighted-CN +Trans method - Pond routing by Stor-Ind method
Subcatchment 1S: SUB1	Runoff Area=6,521 sf 0.00% Impervious Runoff Depth=0.90" Tc=6.0 min CN=57 Runoff=0.13 cfs 488 cf
Subcatchment 2S: SUB2A	Runoff Area=28,661 sf 77.18% Impervious Runoff Depth=3.49" Tc=6.0 min CN=90 Runoff=2.61 cfs 8,339 cf
Subcatchment 3S: SUB2B	Runoff Area=18,034 sf 89.00% Impervious Runoff Depth=3.91" Tc=6.0 min CN=94 Runoff=1.78 cfs 5,882 cf
Subcatchment 4S: SUB2C	Runoff Area=25,380 sf 63.98% Impervious Runoff Depth=2.91" Tc=6.0 min CN=84 Runoff=1.98 cfs 6,148 cf
Pond 5P: Subsurface Infiltration System Discarded=0.19 cfs	n Peak Elev=100.83' Storage=10,855 cf Inflow=9.24 cfs 29,788 cf s 18,345 cf Primary=3.39 cfs 11,443 cf Outflow=3.57 cfs 29,788 cf
Subcatchment 5S: SUB2D	Runoff Area=12,647 sf 81.62% Impervious Runoff Depth=3.59" Tc=6.0 min CN=91 Runoff=1.18 cfs 3,788 cf
Subcatchment 6S: SUB2E	Runoff Area=10,382 sf 54.97% Impervious Runoff Depth=2.63" Tc=6.0 min CN=81 Runoff=0.74 cfs 2,279 cf
Subcatchment 7S: SUB2F	Runoff Area=17,153 sf 86.38% Impervious Runoff Depth=3.81" Tc=6.0 min CN=93 Runoff=1.66 cfs 5,440 cf
Subcatchment 8S: SUB3A	Runoff Area=14,967 sf 30.34% Impervious Runoff Depth=1.89" Tc=8.7 min CN=72 Runoff=0.68 cfs 2,363 cf
Subcatchment 9S: SUB4	Runoff Area=37,456 sf 75.33% Impervious Runoff Depth=3.39" Tc=6.0 min CN=89 Runoff=3.34 cfs 10,582 cf
Subcatchment 10S: SUB5	Runoff Area=93,190 sf 8.24% Impervious Runoff Depth=1.20" Tc=13.8 min CN=62 Runoff=2.08 cfs 9,304 cf
Reach 12R: DP1-Northwest Property	Inflow=0.13 cfs 488 cf Outflow=0.13 cfs 488 cf
Subcatchment 13S: SUB3B	Runoff Area=9,052 sf 98.80% Impervious Runoff Depth=4.36" Tc=6.0 min CN=98 Runoff=0.93 cfs 3,292 cf
Pond 14P: Infiltration Trench Discarded=0.0	Peak Elev=101.48' Storage=389 cf Inflow=0.93 cfs 3,292 cf 1 cfs 1,084 cf Primary=0.89 cfs 2,208 cf Outflow=0.90 cfs 3,292 cf
Subcatchment 14S: SUB2G	Runoff Area=15,595 sf 5.98% Impervious Runoff Depth=1.14" Tc=6.0 min CN=61 Runoff=0.42 cfs 1,476 cf
Subcatchment 15S: SUB	Runoff Area=2,500 sf 100.00% Impervious Runoff Depth=4.36" Tc=6.0 min CN=98 Runoff=0.26 cfs 909 cf

Reach 16R: DP3-Eastern Property	Inflow=2.69 cfs 11,512 cf Outflow=2.69 cfs 11,512 cf
Subcatchment 16S: SUB	Runoff Area=39,556 sf 100.00% Impervious Runoff Depth=4.36" Tc=6.0 min CN=98 Runoff=4.08 cfs 14,385 cf
Reach 17R: DP2-Wells Avenue	Inflow=12.28 cfs 51,756 cf Outflow=12.28 cfs 51,756 cf
Subcatchment 20S: SUB3B	Runoff Area=19,312 sf 85.38% Impervious Runoff Depth=3.81" Tc=6.0 min CN=93 Runoff=1.87 cfs 6,125 cf
Subcatchment 21S: SUB	Runoff Area=8,198 sf 77.21% Impervious Runoff Depth=3.49" Tc=6.0 min CN=90 Runoff=0.75 cfs 2,385 cf

Total Runoff Area = 358,604 sf Runoff Volume = 83,185 cf Average Runoff Depth = 2.78" 44.11% Pervious = 158,164 sf 55.89% Impervious = 200,440 sf







	d by Ňits	roCAD sch Engineer 12 s/n 00546	•	e Solutions L	Type III 24-hr LC	Printed	all=4.60"	
Summary for Subcatchment 4S: SUB2C								
Runoff	=	1.98 cfs @	12.09 hrs, V	/olume=	6,148	cf, Depth= 2.9 ⁻	"	
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs								

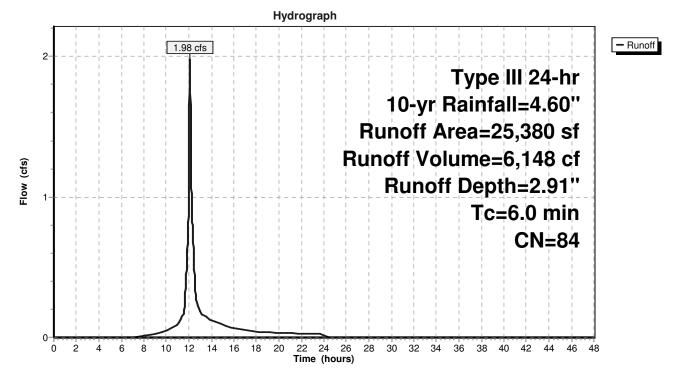
	Are	ea (sf)	CN	Description					
*	1	16,237	98	Paved park	ing				
		4,855	55	Woods, Good, HSG B					
_		4,288	61	>75% Grass cover, Good, HSG B					
	2	5,380 84 Weighted Average							
		9,143		36.02% Pervious Area					
	1	16,237		63.98% Imp	pervious Ar				
	То	Longth	Slope	Volooity	Consoity	Description			
		Length	Slope (ft/ft)		Capacity (cfs)	Description			
_	(min)	(feet)	(11/11	(II/Sec)	(CIS)				
	60					Direct Entry			



Type III 24-hr 10-yr Rainfall=4.60"

Direct Entry,

Subcatchment 4S: SUB2C



Summary for Pond 5P: Subsurface Infiltration System

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Inflow Area =	102,085 sf, 78.10% Impervious,	Inflow Depth = 3.50" for 10-yr event
Inflow =	9.24 cfs @ 12.09 hrs, Volume=	29,788 cf
Outflow =	3.57 cfs @ 12.32 hrs, Volume=	29,788 cf, Atten= 61%, Lag= 14.1 min
Discarded =	0.19 cfs @ 9.03 hrs, Volume=	18,345 cf
Primary =	3.39 cfs @ 12.32 hrs, Volume=	11,443 cf

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 100.83' @ 12.32 hrs Surf.Area= 7,912 sf Storage= 10,855 cf

Plug-Flow detention time= 285.9 min calculated for 29,782 cf (100% of inflow) Center-of-Mass det. time= 286.0 min (1,076.3 - 790.3)

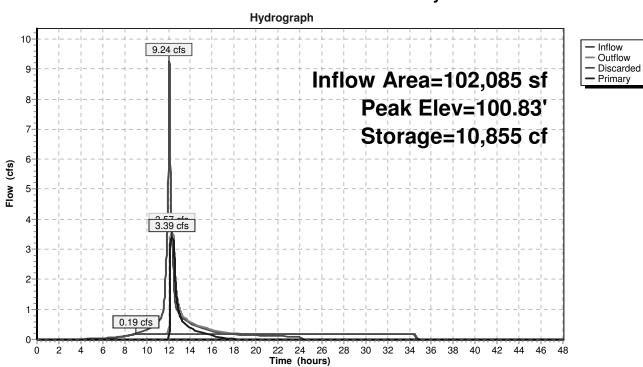
Volume	Invert	Avail.Storage	Storage Description
#1A	98.80'	6,077 cf	68.00'W x 116.36'L x 3.50'H Field A
			27,694 cf Overall - 10,330 cf Embedded = 17,363 cf x 35.0% Voids
#2A	99.30'	10,330 cf	ADS_StormTech SC-740 x 224 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			Row Length Adjustment= +0.44' x 6.45 sf x 14 rows
		16,407 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	98.80'	1.020 in/hr Exfiltration over Horizontal area
#2	Primary	99.30'	12.0" Round Culvert out of OCS
			L= 95.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 99.30' / 98.40' S= 0.0095 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	100.40'	5.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Discarded OutFlow Max=0.19 cfs @ 9.03 hrs HW=98.84' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.19 cfs)

Primary OutFlow Max=3.39 cfs @ 12.32 hrs HW=100.83' (Free Discharge) -2=Culvert out of OCS (Inlet Controls 3.39 cfs @ 4.31 fps) **-3=Sharp-Crested Rectangular Weir** (Passes 3.39 cfs of 4.54 cfs potential flow)



Pond 5P: Subsurface Infiltration System

CN=91

42 44

46 48

Page 38

Summary for Subcatchment 5S: SUB2D

Runoff 1.18 cfs @ 12.08 hrs, Volume= 3,788 cf, Depth= 3.59" =

0-

ò

2 4 6 8 10

12 14 16 18

20

22 24 26

Time (hours)

28 30 32 34 36 38 40

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=4.60"

A	vrea (sf)	CN D	escription					
*	10,322		aved park					
	2,325				ood, HSG B			
	12,647		Veighted A					
	2,325			vious Area				
	10,322	8	1.62% Imp	pervious Ar	ea			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			
Subcatchment 5S: SUB2D								
				Hydro	graph		1	
			1.18 cfs				- Runoff	
						Type III 24-hr		
1-		-¦¦¦ - 1 1 1				-yr Rainfall=4.60"		
	Runoff Area=12,647 sf							
fs)					Runofi	Volume=3,788 cf		
Flow (cfs)					 	inoff Depth=3.59"		
Elo .								
						Tc=6.0 min		

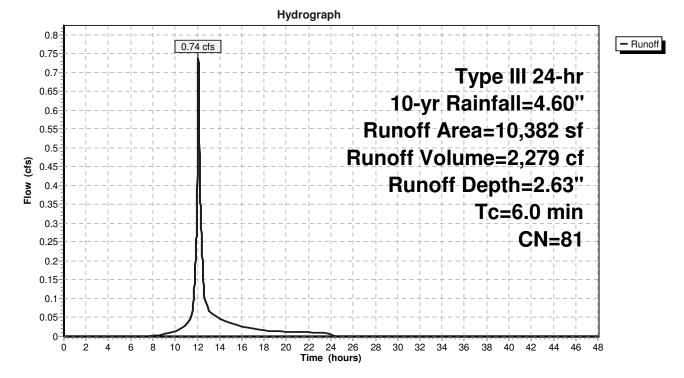
Summary for Subcatchment 6S: SUB2E

Runoff	=	0.74 cfs @	12.09 hrs, Volume=	2,279 cf, Depth= 2.63"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=4.60"

A	Area (sf)	CN	Description				
*	5,707	98	Paved park	ing			
	4,001	61	>75% Gras	s cover, Go	lood, HSG B		
	674	55	Woods, Good, HSG B				
	10,382	81	Weighted Average				
	4,675		45.03% Pervious Area				
	5,707		54.97% Imp	pervious Ar	rea		
-				o ''			
Tc	Length	Slope		Capacity	•		
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)			
6.0					Direct Entry,		

Subcatchment 6S: SUB2E



Proposed Hyd Prepared by Nits HydroCAD® 10.00	
	Summary for Subcatchment 7S: SUB2F
Runoff =	1.66 cfs @ 12.08 hrs, Volume= 5,440 cf, Depth= 3.81"
Runoff by SCS TF Type III 24-hr 10-	R-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs yr Rainfall=4.60"
Area (sf)	CN Description
* 14,816 2,337	 98 Paved parking 61 >75% Grass cover, Good, HSG B
17,153 2,337 14,816	93 Weighted Average 13.62% Pervious Area 86.38% Impervious Area
Tc Length (min) (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)
6.0	Direct Entry,
	Subcatchment 7S: SUB2F Index of the second state in the s

Proposed Conditions

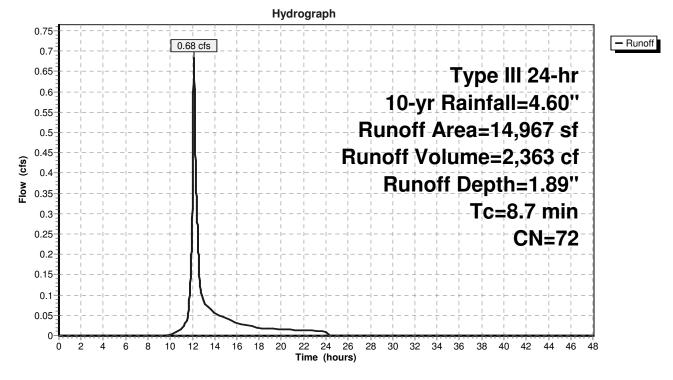
Summary for Subcatchment 8S: SUB3A

Runoff = 0.68 cfs @ 12.13 hrs, Volume= 2,363 cf, Depth= 1.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=4.60"

_	A	rea (sf)	CN	Description				
*		4,541	98	Impervious				
_		10,426	61	>75% Gras	s cover, Go	bod, HSG B		
		14,967	72	Weighted Average				
		10,426		69.66% Pervious Area				
		4,541		30.34% Impervious Area				
	Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description		
	8.7					Direct Entry, NO CHANGE FROM EXISTING		





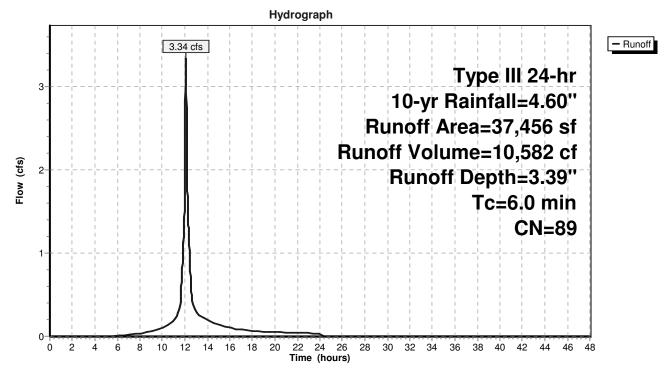
Proposed Conditions Proposed HydroCAD Type III 24-hr 10-yr Rainfall=4.60" Printed 5/6/2015 Prepared by Nitsch Engineering HydroCAD® 10.00-12 s/n 00546 © 2014 HydroCAD Software Solutions LLC Page 42 Summary for Subcatchment 9S: SUB4 Runoff 3.34 cfs @ 12.09 hrs, Volume= 10,582 cf, Depth= 3.39" = Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=4.60" Area (sf) CN Description

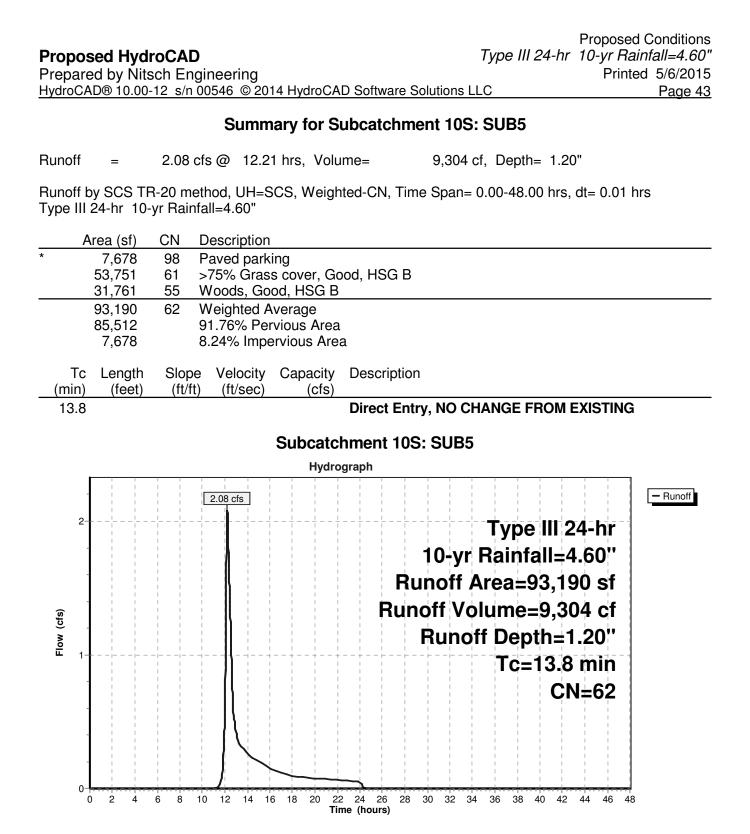
	6.0					Direct Entry				
_	(min)	(feet)	(ft/ft)) (ft/sec)	(cfs)					
		•	•	,		Description				
	Тс	Length	Slope	e Velocity	Capacity	Description				
	28,217 75.33% Impervious Area					ea				
		9,239		24.67% Per						
		37,456	89	Weighted A	verage					
		9,239	61	>75% Gras	75% Grass cover, Good, HSG B					
,	k	22,782	98	Roofs						
	k	5,435	98	Paved park	ing					
-			-							
	Δ	rea (st)	CN	Description						



Direct Entry,

Subcatchment 9S: SUB4





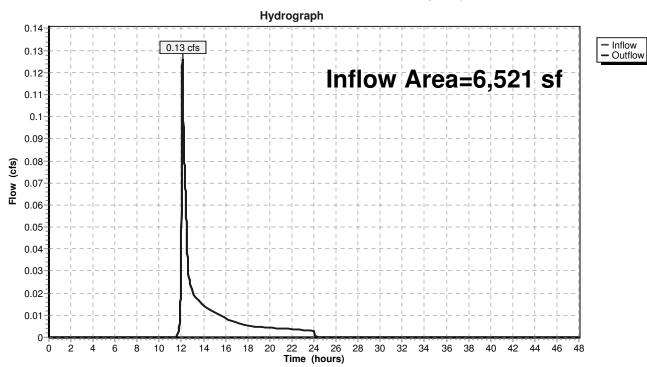
Summary for Reach 12R: DP1-Northwest Property

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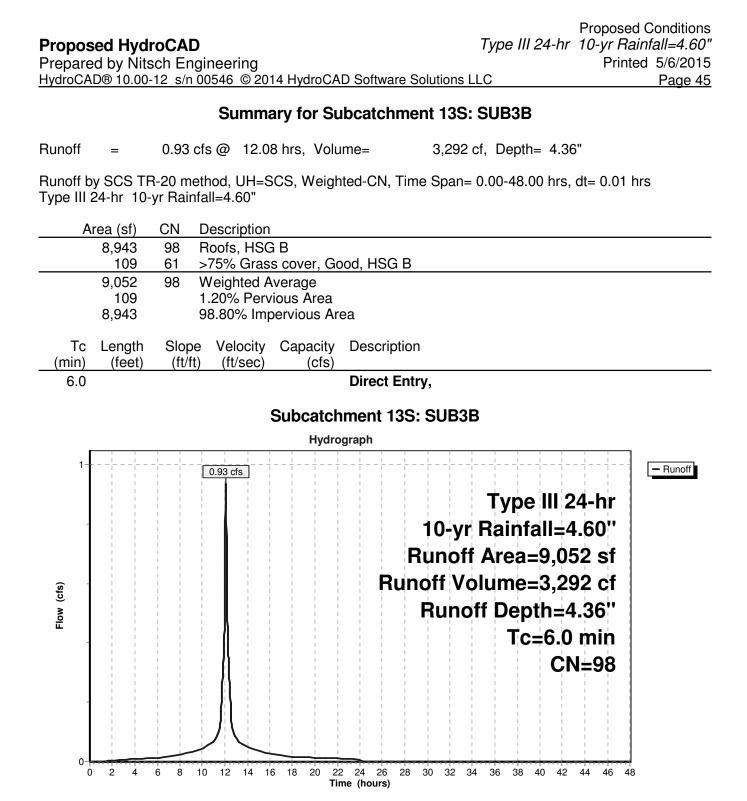
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Inflow Are	a =	6,521 sf,	0.00% Impervious	Inflow Depth = 0.90" for 10-yr event
Inflow	=	0.13 cfs @ 1	12.11 hrs, Volume=	488 cf
Outflow	=	0.13 cfs @ 1	12.11 hrs, Volume=	488 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach 12R: DP1-Northwest Property



Inflow Area =	9,052 sf, 98.80% Impervious,	Inflow Depth = 4.36" for 10-yr event
Inflow =	0.93 cfs @ 12.08 hrs, Volume=	3,292 cf
Outflow =	0.90 cfs @ 12.10 hrs, Volume=	3,292 cf, Atten= 3%, Lag= 1.3 min
Discarded =	0.01 cfs @ 5.30 hrs, Volume=	1,084 cf
Primary =	0.89 cfs @ 12.10 hrs, Volume=	2,208 cf

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 101.48' @ 12.10 hrs Surf.Area= 458 sf Storage= 389 cf

Plug-Flow detention time= 95.8 min calculated for 3,291 cf (100% of inflow) Center-of-Mass det. time= 95.9 min (845.3 - 749.4)

Volume	Inver	t Avail.Sto	rage	Storage	e Description	
#1	99.50)' 60)5 cf	Stone (Prismatic) Listed	l below (Recalc)
#2	101.00)' (90 cf	,	Round Pipe Stora	Embedded = 1,513 cf x 40.0% Voids age Inside #1
		69	95 cf	Total Av	vailable Storage	
Elevatio (fee		Surf.Area (sq-ft)		:.Store c-feet)	Cum.Store (cubic-feet)	
99.5	0	458		0	0	
103.0	0	458		1,603	1,603	
Device	Routing	Invert	Outl	et Device	es	
#1	Primary	101.00'			d Culvert	
						neadwall, Ke= 0.500
						100.20' S= 0.0154 '/' Cc= 0.900
#2	Discarded	99.50'			xfiltration over S	both interior, Flow Area= 0.79 sf Surface area
			-			

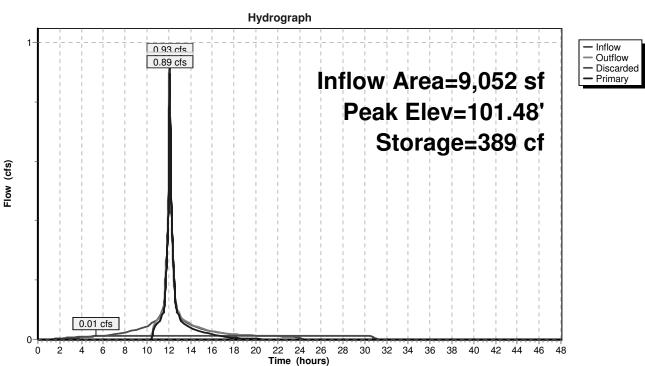
Discarded OutFlow Max=0.01 cfs @ 5.30 hrs HW=99.54' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.89 cfs @ 12.10 hrs HW=101.48' (Free Discharge) **1=Culvert** (Inlet Controls 0.89 cfs @ 2.37 fps)

Proposed Conditions

Proposed HydroCAD

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Pond 14P: Infiltration Trench

Proposed Conditions *Type III 24-hr 10-yr Rainfall=4.60"* Printed 5/6/2015 <u>C Page 47</u>

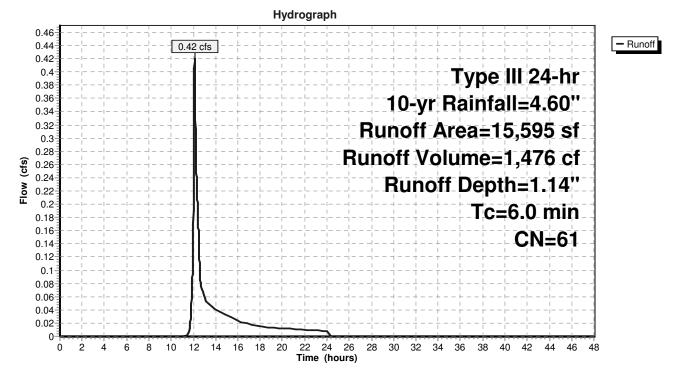
Summary for Subcatchment 14S: SUB2G

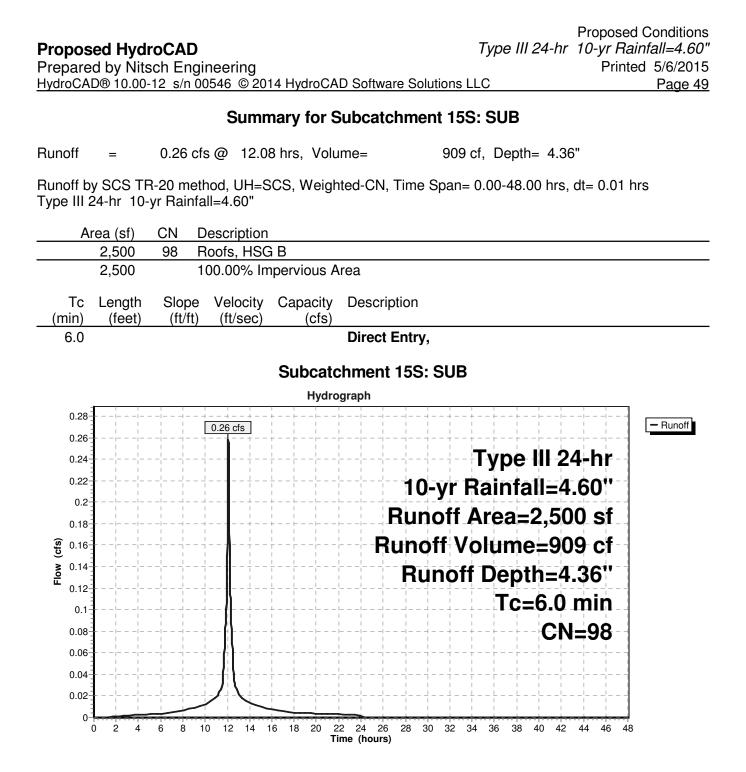
Runoff = 0.42 cfs @ 12.10 hrs, Volume= 1,476 cf, Depth= 1.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=4.60"

	Area (sf)	CN	Description					
*	933	98	Paved park	ing				
	10,125	61	>75% Gras	s cover, Go	ood, HSG B			
	4,537	55	Woods, Go	od, HSG B	3			
	15,595	61	Weighted A	verage				
	14,662		94.02% Pe	rvious Area	1			
	933		5.98% Impe	ervious Are	a			
۲ miı)	c Length	Slop (ft/f	,	Capacity (cfs)	Description			
6	/ / /	(10/1	(1 <i>1</i> ,000)	(010)	Direct Entry,			
Ŭ	•				2			

Subcatchment 14S: SUB2G

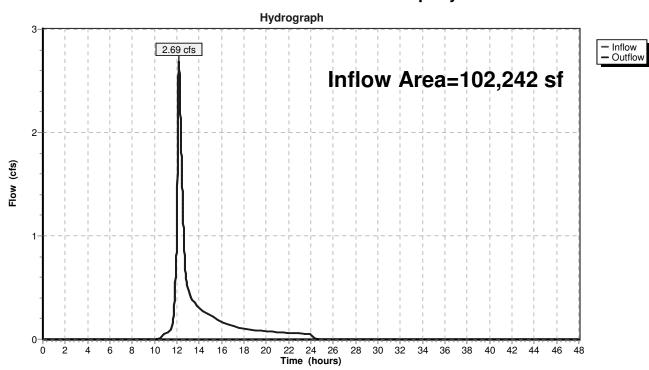




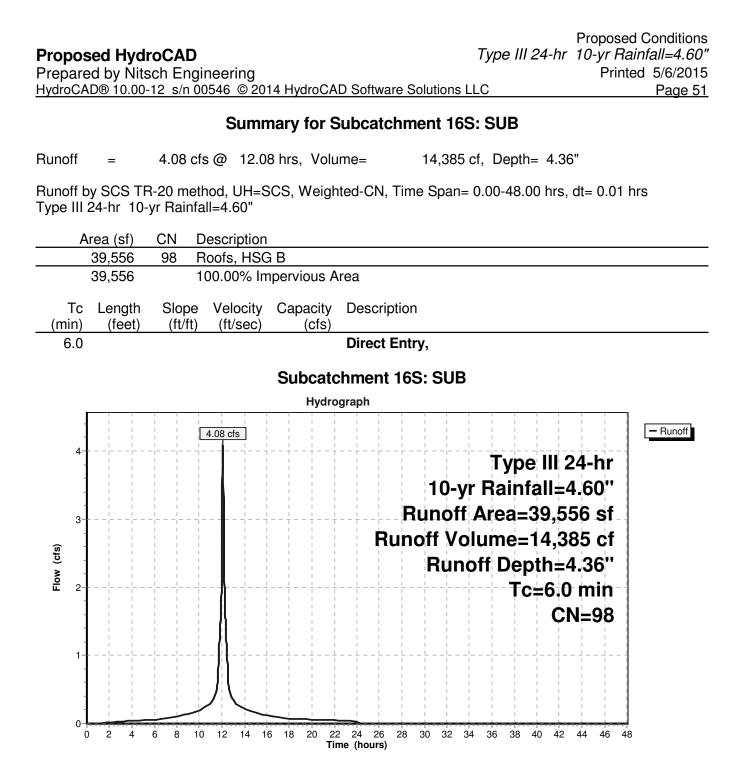
Summary for Reach 16R: DP3-Eastern Property

Inflow Are	a =	102,242 sf, 16.26% Impervious, Inflow Depth = 1.35" for 10-yr	^r event
Inflow	=	2.69 cfs @ 12.18 hrs, Volume= 11,512 cf	
Outflow	=	2.69 cfs @ 12.18 hrs, Volume= 11,512 cf, Atten= 0%, La	g= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach 16R: DP3-Eastern Property



Summary for Reach 17R: DP2-Wells Avenue

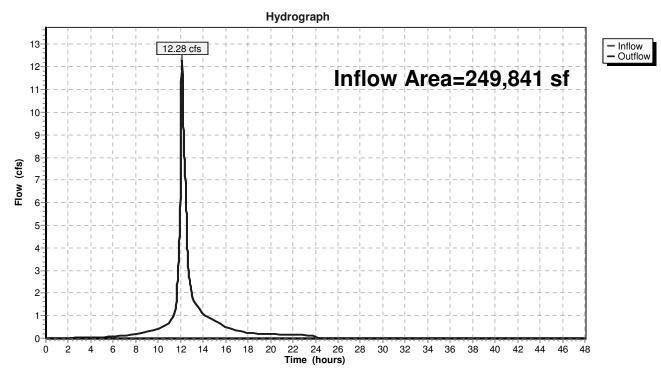
Proposed Conditions

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Inflow Are	a =	249,841 sf, 73.57% Impervious, Inflow Depth = 2.49" for 10-yr event	9,841 sf, 73	
Inflow	=	12.28 cfs @ 12.12 hrs, Volume= 51,756 cf	3 cfs @ 12.1	
Outflow	=	12.28 cfs @ 12.12 hrs, Volume= 51,756 cf, Atten= 0%, Lag= 0.0 min	3 cfs @ 12.1	min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach 17R: DP2-Wells Avenue

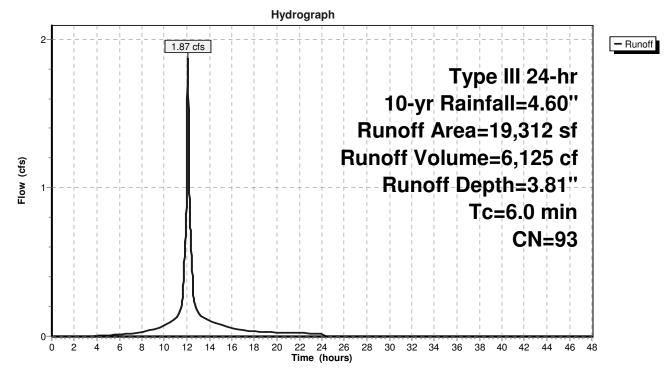
Summary for Subcatchment 20S: SUB3B

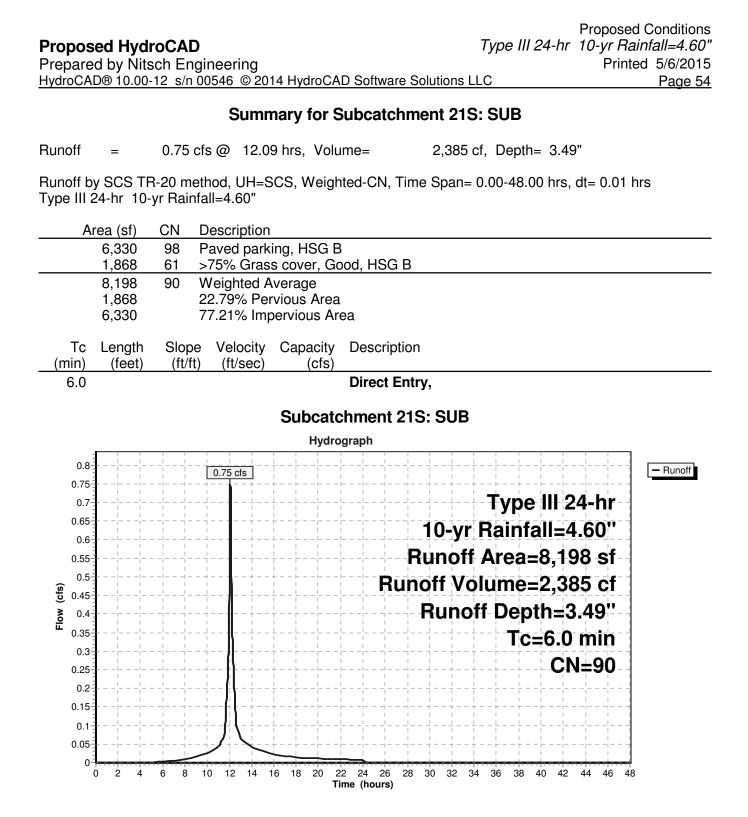
Runoff	=	1.87 cfs @	12.08 hrs,	Volume=	6,125 cf, Depth= 3.81"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=4.60"

Area	(sf)	CN I	Description				
14	089	98 I	Pavement				
2	823	61 :	>75% Gras	s cover, Go	od, HSG B		
2	400	98 I	Pavement				
19	312	93 \	Weighted Average				
2	823	-	14.62% Pei	vious Area			
16	489	8	35.38% Imp	pervious Are	a		
	•				Description		
iin)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		
	14, 2, 19, 19, 16, Tc Le	iin) (feet)	14,089 98 F 2,823 61 2 2,400 98 F 19,312 93 V 2,823 7 16,489 8 Tc Length Slope in) (feet) (ft/ft)	14,089 98 Pavement 2,823 61 >75% Gras 2,400 98 Pavement 19,312 93 Weighted A 2,823 14.62% Per 16,489 85.38% Imp Tc Length Slope Velocity in) (feet) (ft/ft) (ft/sec)	14,08998Pavement2,82361>75% Grass cover, God2,40098Pavement19,31293Weighted Average2,82314.62% Pervious Area16,48985.38% Impervious AreaTcLengthSlopeVelocityCapacity(ft/ft)(ft/sec)(cfs)	14,08998Pavement2,82361>75% Grass cover, Good, HSG B2,40098Pavement19,31293Weighted Average2,82314.62% Pervious Area16,48985.38% Impervious AreaTcLengthSlopeVelocityCapacityDescriptionin)(ft/ft)(ft/sec)(cfs)	

Subcatchment 20S: SUB3B

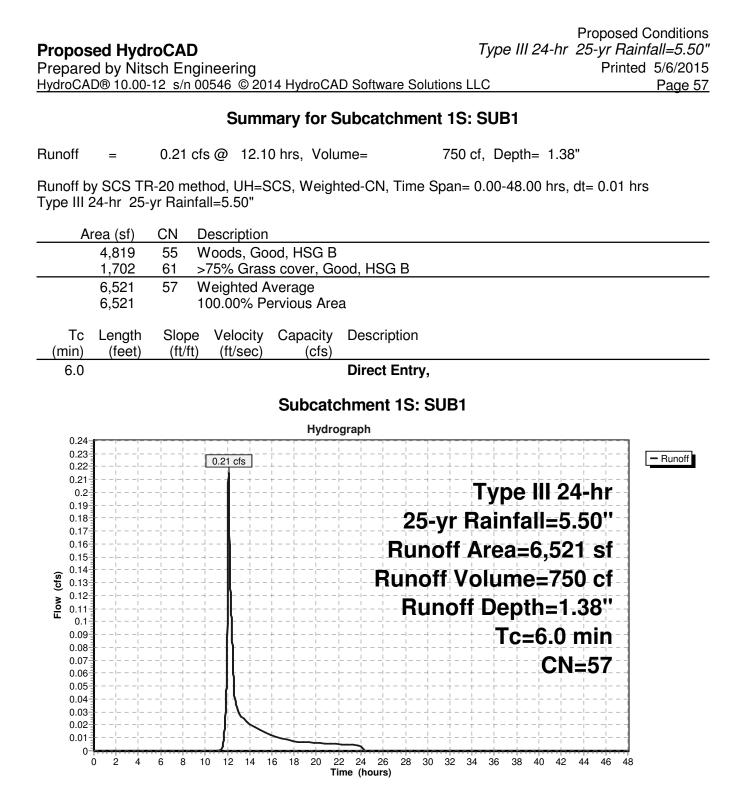


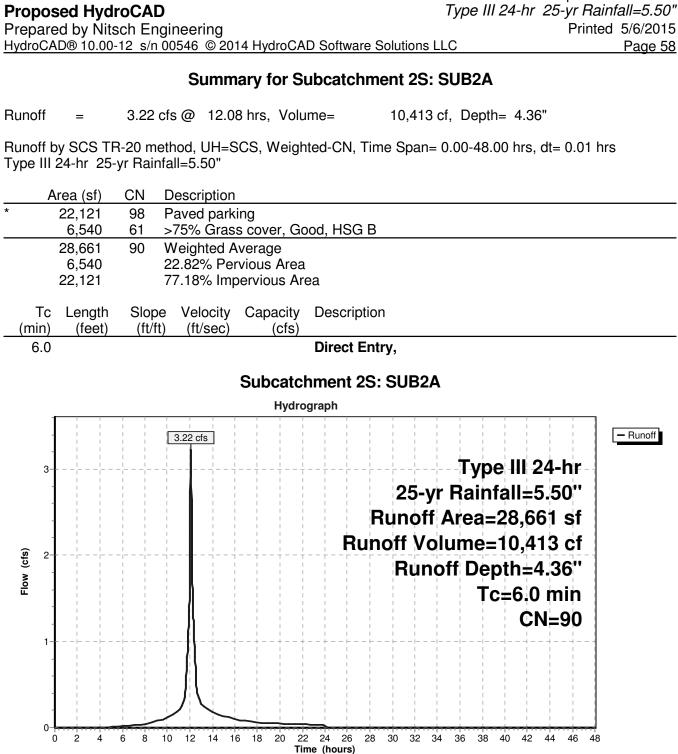


Proposed HydroCAD Prepared by Nitsch Engineering HydroCAD® 10.00-12 s/n 00546 © 2014 Hy	Proposed Conditions <i>Type III 24-hr 25-yr Rainfall=5.50"</i> Printed 5/6/2015 droCAD Software Solutions LLC Page 55
Runoff by SCS 1	00-48.00 hrs, dt=0.01 hrs, 4801 points FR-20 method, UH=SCS, Weighted-CN Trans method - Pond routing by Stor-Ind method
Subcatchment 1S: SUB1	Runoff Area=6,521 sf 0.00% Impervious Runoff Depth=1.38" Tc=6.0 min CN=57 Runoff=0.21 cfs 750 cf
Subcatchment 2S: SUB2A	Runoff Area=28,661 sf 77.18% Impervious Runoff Depth=4.36" Tc=6.0 min CN=90 Runoff=3.22 cfs 10,413 cf
Subcatchment 3S: SUB2B	Runoff Area=18,034 sf 89.00% Impervious Runoff Depth=4.80" Tc=6.0 min CN=94 Runoff=2.15 cfs 7,216 cf
Subcatchment 4S: SUB2C	Runoff Area=25,380 sf 63.98% Impervious Runoff Depth=3.73" Tc=6.0 min CN=84 Runoff=2.52 cfs 7,891 cf
	Peak Elev=101.33' Storage=13,356 cf Inflow=11.41 cfs 37,142 cf 19,384 cf Primary=4.05 cfs 17,759 cf Outflow=4.23 cfs 37,142 cf
Subcatchment 5S: SUB2D	Runoff Area=12,647 sf 81.62% Impervious Runoff Depth=4.47" Tc=6.0 min CN=91 Runoff=1.45 cfs 4,710 cf
Subcatchment 6S: SUB2E	Runoff Area=10,382 sf 54.97% Impervious Runoff Depth=3.43" Tc=6.0 min CN=81 Runoff=0.96 cfs 2,968 cf
Subcatchment 7S: SUB2F	Runoff Area=17,153 sf 86.38% Impervious Runoff Depth=4.69" Tc=6.0 min CN=93 Runoff=2.02 cfs 6,703 cf
Subcatchment 8S: SUB3A	Runoff Area=14,967 sf 30.34% Impervious Runoff Depth=2.59" Tc=8.7 min CN=72 Runoff=0.95 cfs 3,230 cf
Subcatchment 9S: SUB4	Runoff Area=37,456 sf 75.33% Impervious Runoff Depth=4.25" Tc=6.0 min CN=89 Runoff=4.14 cfs 13,273 cf
Subcatchment 10S: SUB5	Runoff Area=93,190 sf 8.24% Impervious Runoff Depth=1.76" Tc=13.8 min CN=62 Runoff=3.22 cfs 13,637 cf
Reach 12R: DP1-Northwest Property	Inflow=0.21 cfs 750 cf Outflow=0.21 cfs 750 cf
Subcatchment 13S: SUB3B	Runoff Area=9,052 sf 98.80% Impervious Runoff Depth=5.26" Tc=6.0 min CN=98 Runoff=1.12 cfs 3,970 cf
Pond 14P: Infiltration Trench Discarded=0.01	Peak Elev=101.54' Storage=403 cf Inflow=1.12 cfs 3,970 cf cfs 1,119 cf Primary=1.08 cfs 2,851 cf Outflow=1.09 cfs 3,970 cf
Subcatchment 14S: SUB2G	Runoff Area=15,595 sf 5.98% Impervious Runoff Depth=1.68" Tc=6.0 min CN=61 Runoff=0.66 cfs 2,182 cf
Subcatchment 15S: SUB	Runoff Area=2,500 sf 100.00% Impervious Runoff Depth=5.26" Tc=6.0 min CN=98 Runoff=0.31 cfs 1,096 cf

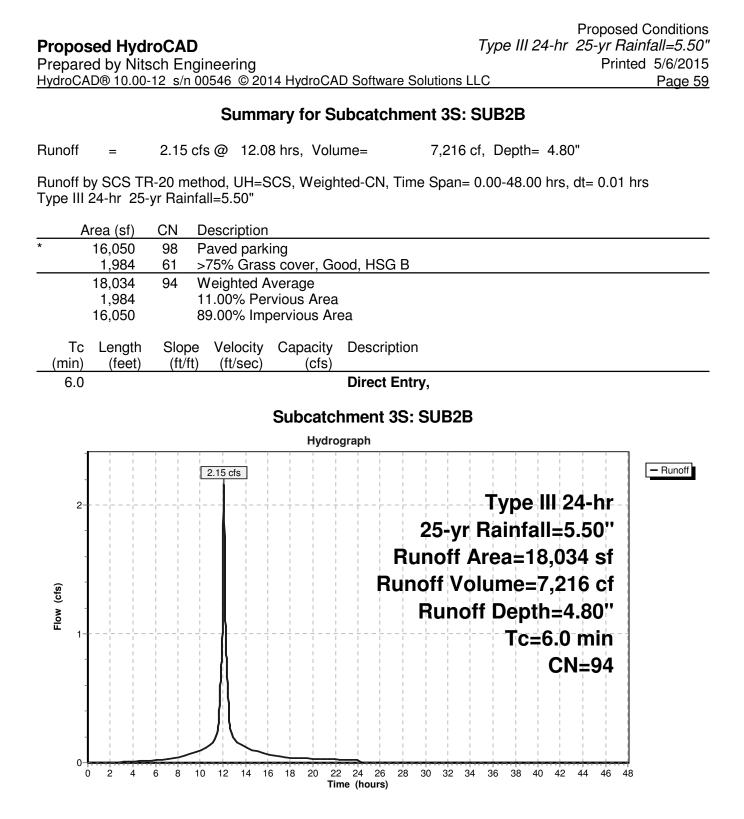
Reach 16R: DP3-Eastern Property	Inflow=3.96 cfs 16,488 cf Outflow=3.96 cfs 16,488 cf
Subcatchment 16S: SUB	Runoff Area=39,556 sf 100.00% Impervious Runoff Depth=5.26" Tc=6.0 min CN=98 Runoff=4.89 cfs 17,347 cf
Reach 17R: DP2-Wells Avenue	Inflow=18.35 cfs 68,171 cf Outflow=18.35 cfs 68,171 cf
Subcatchment 20S: SUB3B	Runoff Area=19,312 sf 85.38% Impervious Runoff Depth=4.69" Tc=6.0 min CN=93 Runoff=2.28 cfs 7,547 cf
Subcatchment 21S: SUB	Runoff Area=8,198 sf 77.21% Impervious Runoff Depth=4.36" Tc=6.0 min CN=90 Runoff=0.92 cfs 2,979 cf

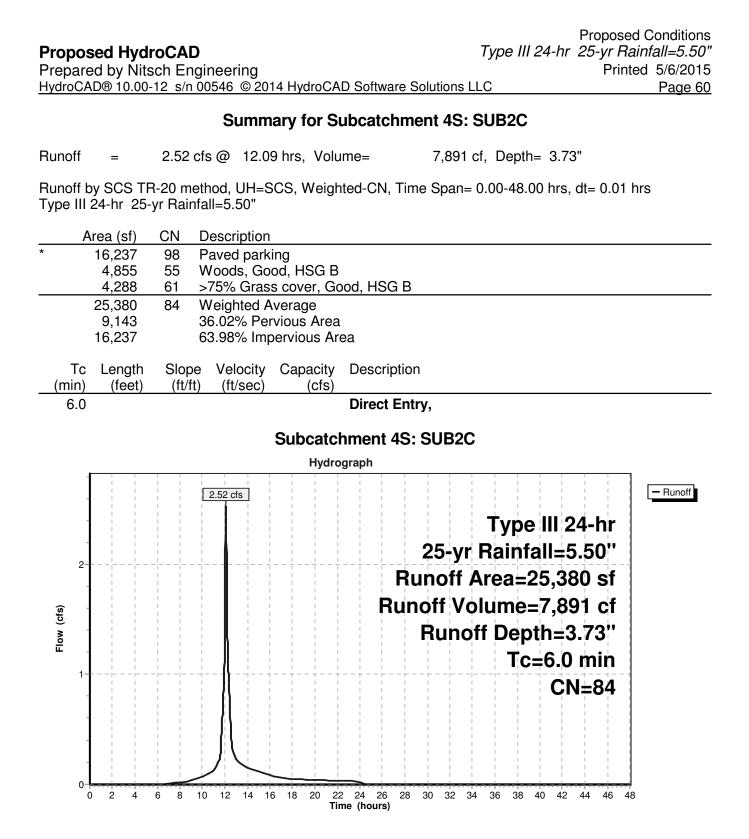
Total Runoff Area = 358,604 sf Runoff Volume = 105,912 cf Average Runoff Depth = 3.54" 44.11% Pervious = 158,164 sf 55.89% Impervious = 200,440 sf





Proposed Conditions





Summary for Pond 5P: Subsurface Infiltration System

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Inflow Area =	102,085 sf, 78.10% Impervious,	Inflow Depth = 4.37" for 25-yr event
Inflow =	11.41 cfs @ 12.08 hrs, Volume=	37,142 cf
Outflow =	4.23 cfs @ 12.33 hrs, Volume=	37,142 cf, Atten= 63%, Lag= 14.9 min
Discarded =	0.19 cfs @ 8.45 hrs, Volume=	19,384 cf
Primary =	4.05 cfs @ 12.33 hrs, Volume=	17,759 cf

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 101.33' @ 12.33 hrs Surf.Area= 7,912 sf Storage= 13,356 cf

Plug-Flow detention time= 249.3 min calculated for 37,135 cf (100% of inflow) Center-of-Mass det. time= 249.4 min (1,034.2 - 784.7)

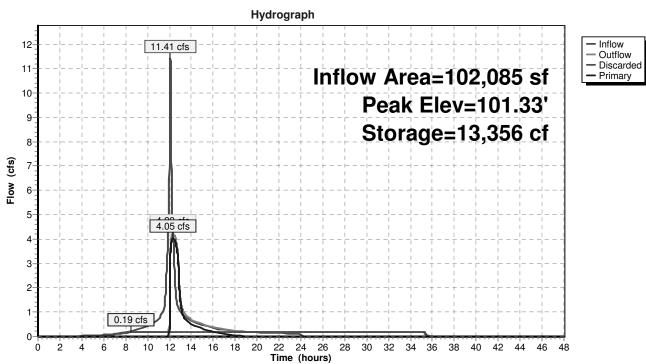
Volume	Invert	Avail.Storage	Storage Description
#1A	98.80'	6,077 cf	68.00'W x 116.36'L x 3.50'H Field A
			27,694 cf Overall - 10,330 cf Embedded = 17,363 cf x 35.0% Voids
#2A	99.30'	10,330 cf	ADS_StormTech SC-740 x 224 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			Row Length Adjustment= +0.44' x 6.45 sf x 14 rows
		16,407 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	98.80'	1.020 in/hr Exfiltration over Horizontal area
#2	Primary	99.30'	12.0" Round Culvert out of OCS
			L= 95.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 99.30' / 98.40' S= 0.0095 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	100.40'	5.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Discarded OutFlow Max=0.19 cfs @ 8.45 hrs HW=98.84' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.19 cfs)

Primary OutFlow Max=4.05 cfs @ 12.33 hrs HW=101.33' (Free Discharge) -2=Culvert out of OCS (Barrel Controls 4.05 cfs @ 5.15 fps) **1**-3=Sharp-Crested Rectangular Weir (Passes 4.05 cfs of 14.13 cfs potential flow)



Pond 5P: Subsurface Infiltration System

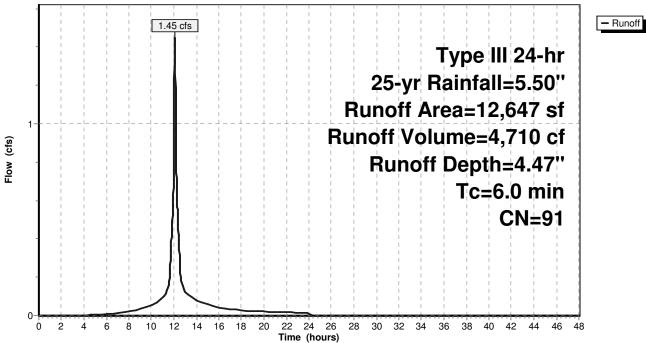
Proposed Conditions

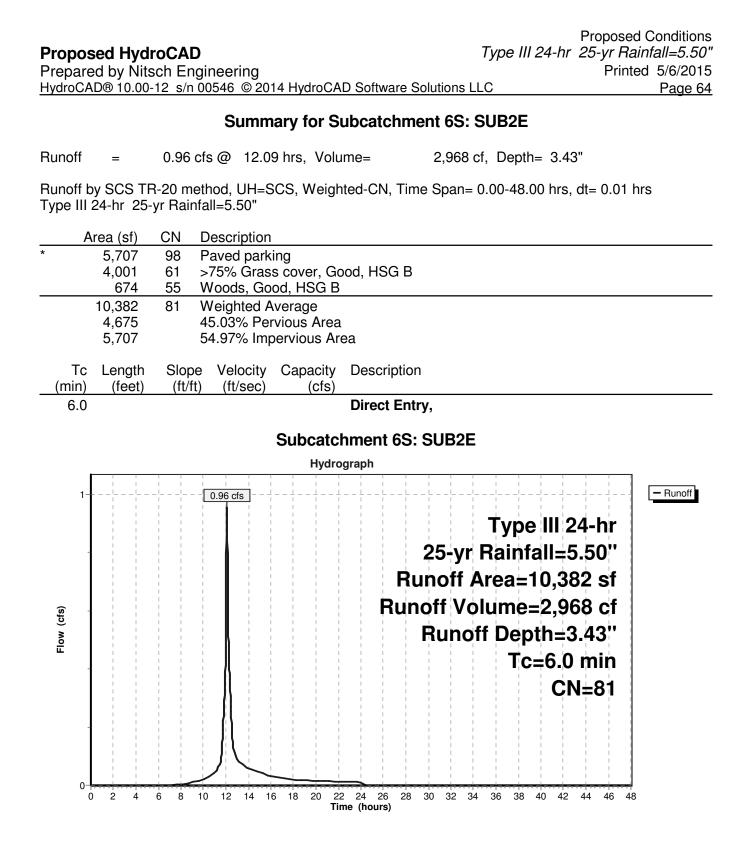
Summary for Subcatchment 5S: SUB2D

Runoff = 1.45 cfs @ 12.08 hrs, Volume= 4,710 cf, Depth= 4.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.50"

	А	rea (sf)	CN	Description						
*		10,322	98	Paved parking						
		2,325	61	>75% Grass cover, Good, HSG B						
		12,647	91 Weighted Average							
		2,325		18.38% Pervious Area						
		10,322		81.62% Impervious Area						
	Тс	Length	Length Slope Velocity Capacity Description							
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry,				
	Subcatchment 5S: SUB2D									
Hydrograph										





Summary for Subcatchment 7S: SUB2F

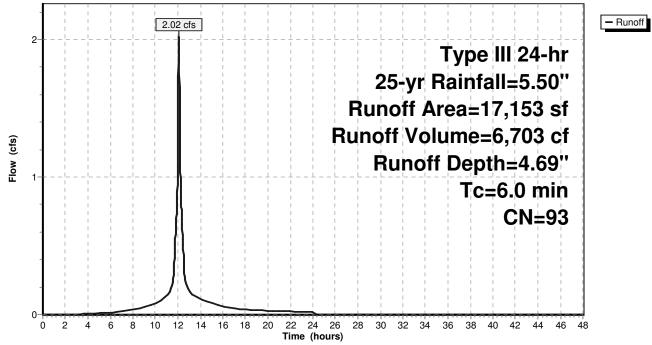
Runoff = 2.02 cfs @ 12.08 hrs, Volume= 6,703 cf, Depth= 4.6	ct, Deptn= 4.69"
---	------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.50"

_	A	rea (sf)	CN	Description				
*		14,816	98	Paved parking				
_		2,337	61	>75% Grass cover, Good, HSG B				
		17,153	93	Weighted Average				
		2,337		13.62% Pervious Area				
		14,816		86.38% Imp	pervious Ar	rea		
	Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description		
	6.0					Direct Entry,		

Subcatchment 7S: SUB2F





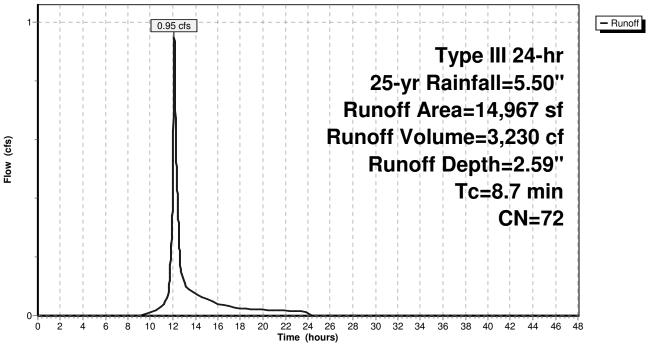
Runoff	=	0.95 cfs @	12.13 hrs.	Volume=	3,230 cf, Depth= 2	2.59"
i tunion	_	0.00 010 @	12.101.00,	• • • • • • • • •		00

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.50"

	A	vrea (sf)	CN	Description					
*		4,541	98	Impervious					
		10,426	61	>75% Grass cover, Good, HSG B					
		14,967	72	Weighted Average					
		10,426		69.66% Pervious Area					
		4,541		30.34% Impervious Area					
	_				- .				
	Tc	Length	Slope		Capacity	Description			
_	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
	8.7					Direct Entry, NO CHANGE FROM EXISTING			
						-			

Subcatchment 8S: SUB3A

Hydrograph



Proposed Conditions Type III 24-hr 25-yr Rainfall=5.50" Proposed HydroCAD Printed 5/6/2015 Prepared by Nitsch Engineering HydroCAD® 10.00-12 s/n 00546 © 2014 HydroCAD Software Solutions LLC Page 67 Summary for Subcatchment 9S: SUB4 Runoff 4.14 cfs @ 12.09 hrs, Volume= 13,273 cf, Depth= 4.25" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.50" Area (sf) CN Description * 5,435 98 Paved parking * 22,782 98 Roofs 9,239 61 >75% Grass cover, Good, HSG B 37,456 89 Weighted Average 24.67% Pervious Area 9,239 28,217 75.33% Impervious Area Tc Length Velocity Capacity Slope Description (feet) (ft/ft) (ft/sec) (cfs) (min) **Direct Entry**, 6.0 Subcatchment 9S: SUB4 Hydrograph - Runoff 4.14 cfs 4 Type III 24-hr 25-yr Rainfall=5.50" Runoff Area=37,456 sf 3 Runoff Volume=13,273 cf Flow (cfs) Runoff Depth=4.25"

2

1

0

0

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4 6 8

10 12 14 16 18 20

22 24 26

Time (hours)

28 30 32 34

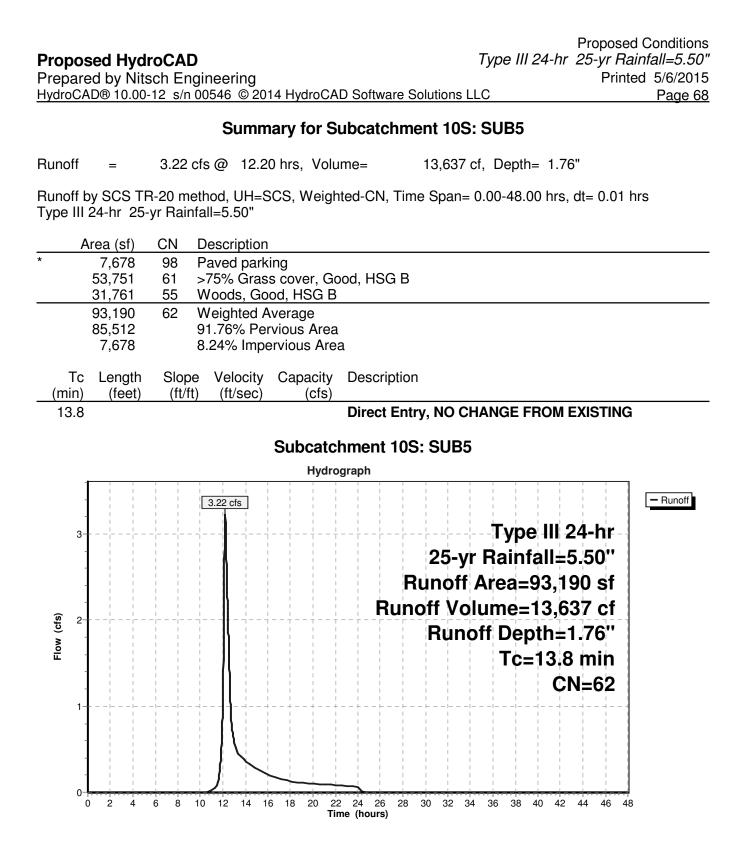
36

38 40

Tc=6.0 min

CN=89

42 44 46 48



Summary for Reach 12R: DP1-Northwest Property

Proposed Conditions

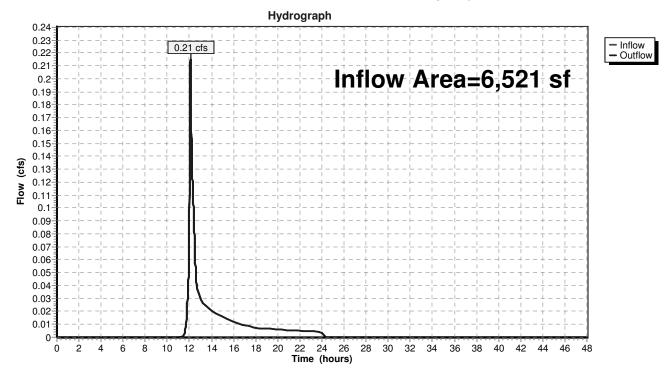
Printed 5/6/2015

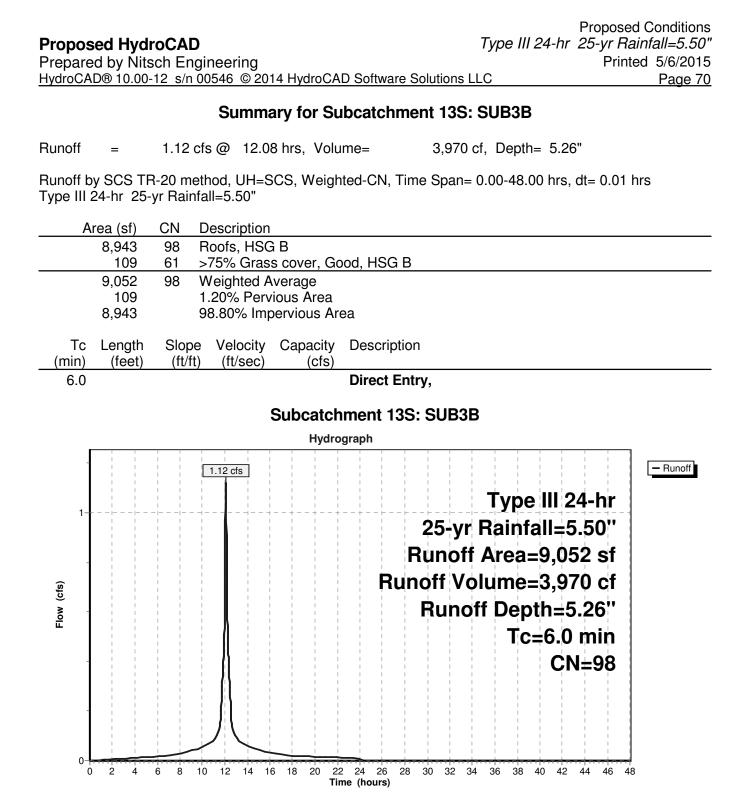
Page 69

Inflow Are	a =	6,521 sf,	0.00% Impervious,	Inflow Depth = 1.38"	for 25-yr event
Inflow	=	0.21 cfs @ 1	12.10 hrs, Volume=	750 cf	
Outflow	=	0.21 cfs @ 1	12.10 hrs, Volume=	750 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach 12R: DP1-Northwest Property





Summary for Pond 14P: Infiltration Trench

Proposed Conditions

Printed 5/6/2015

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Inflow Area =	9,052 sf, 98.80% Impervious,	Inflow Depth = 5.26" for 25-yr event
Inflow =	1.12 cfs @ 12.08 hrs, Volume=	3,970 cf
Outflow =	1.09 cfs @ 12.10 hrs, Volume=	3,970 cf, Atten= 3%, Lag= 1.2 min
Discarded =	0.01 cfs @ 4.17 hrs, Volume=	1,119 cf
Primary =	1.08 cfs @ 12.10 hrs, Volume=	2,851 cf

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 101.54' @ 12.10 hrs Surf.Area= 458 sf Storage= 403 cf

Plug-Flow detention time= 84.8 min calculated for 3,970 cf (100% of inflow) Center-of-Mass det. time= 84.7 min (831.2 - 746.5)

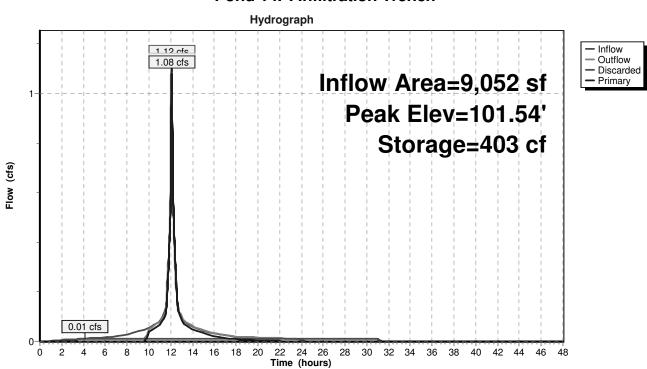
Volume	Inver	t Avail.Sto	rage	Storage D	escription	
#1	99.50	' 60	605 cf		smatic) Listed	l below (Recalc)
						Embedded = $1,513 \text{ cf } \times 40.0\%$ Voids
#2	101.00	' (90 cf		and Pipe Stora	age Inside #1
				L= 114.5'		
		69	95 cf	Total Avai	lable Storage	
Elovatio		urf Aroo	Inc	Store	Cum Store	
Elevatio		Surf.Area		Store	Cum.Store	
(fee	t)	(sq-ft)	(cubi	c-feet)	(cubic-feet)	
99.5	60	458		0	0	
103.0	0	458		1,603	1,603	
Device	Routing	Invert	Outl	et Devices		
#1	Primary	101.00'	12.0	" Round C	ulvert	
			L= 5	2.0' CPP.	square edge h	neadwall, Ke= 0.500
						100.20' S= 0.0154 '/' Cc= 0.900
						poth interior, Flow Area= 0.79 sf
#2	Discarded	99.50'	1.02	0 in/hr Exfi	Itration over S	Surface area
Discoul	D iscourse $\mathbf{D}_{\mathbf{r}}$ D iscourse $\mathbf{N}_{\mathbf{r}} = 0.04 \cdot (r_{\mathbf{r}} \odot 4.47 \cdot r_{\mathbf{r}} + 10.4 \cdot 0.05 \cdot 10.4 \cdot (r_{\mathbf{r}} \odot \mathbf{D}_{\mathbf{r}}^{2} \circ (r_{\mathbf{r}} \odot \mathbf{D}_{\mathbf{r}}^{2} \circ (r_{\mathbf{r}} \circ \mathbf{D}_{\mathbf{r}}^{2})$					

Discarded OutFlow Max=0.01 cfs @ 4.17 hrs HW=99.54' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=1.07 cfs @ 12.10 hrs HW=101.54' (Free Discharge)

Proposed HydroCAD

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Pond 14P: Infiltration Trench

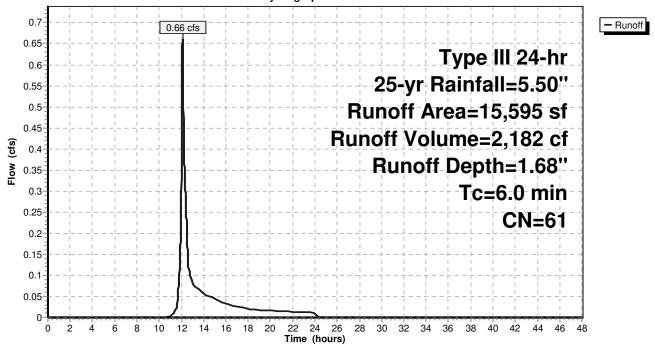
Proposed Conditions *Type III 24-hr 25-yr Rainfall=5.50"* Printed 5/6/2015 LC Page 72

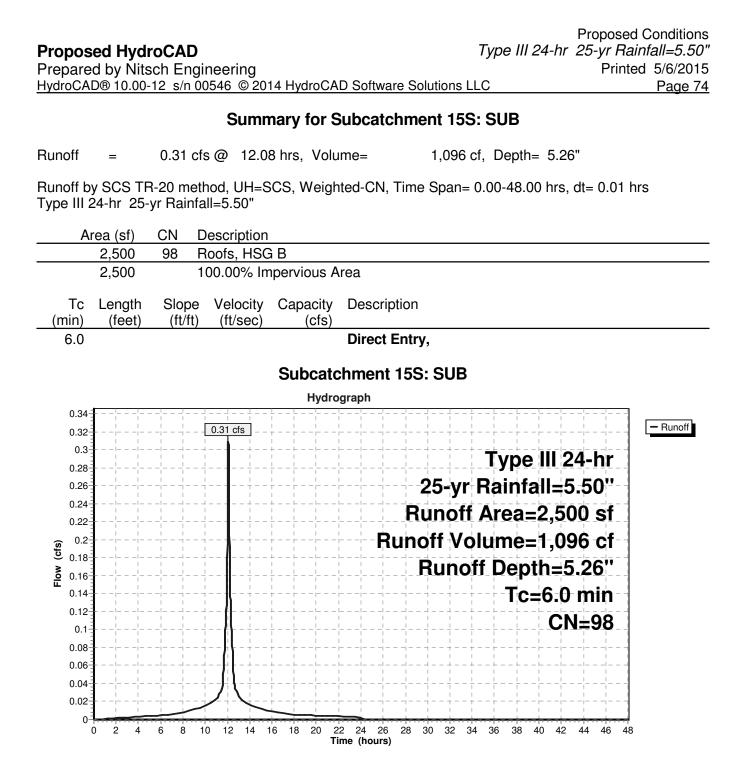
Summary for Subcatchment 14S: SUB2G

Runoff = 0.66 cfs @ 12.10 hrs, Volume= 2,182 cf, Depth= 1.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.50"

	A	rea (sf)	CN	Description				
*		933	98	Paved park	ing			
		10,125	61	>75% Gras	s cover, Go	bod, HSG B		
		4,537	55	Woods, Go	od, HSG B			
		15,595	61	Weighted A	verage			
		14,662		94.02% Pe	rvious Area	L		
		933		5.98% Impe	ervious Area	a		
	_							
	Tc	Length	Slop		Capacity	Description		
((min)	(feet)	(ft/ft	t) (ft/sec)	(cfs)			
	6.0					Direct Entry,		
	Subcatchment 14S: SUB2G							
	Hydrograph							

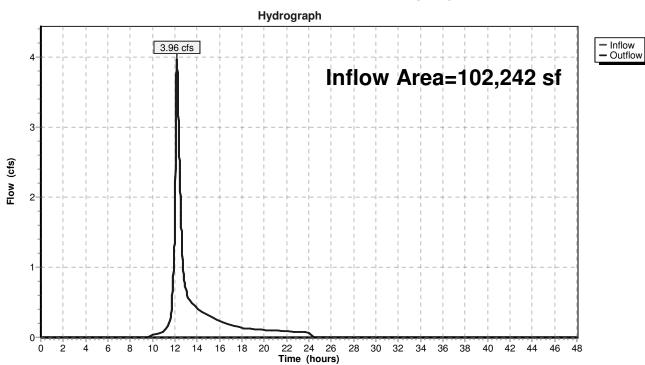




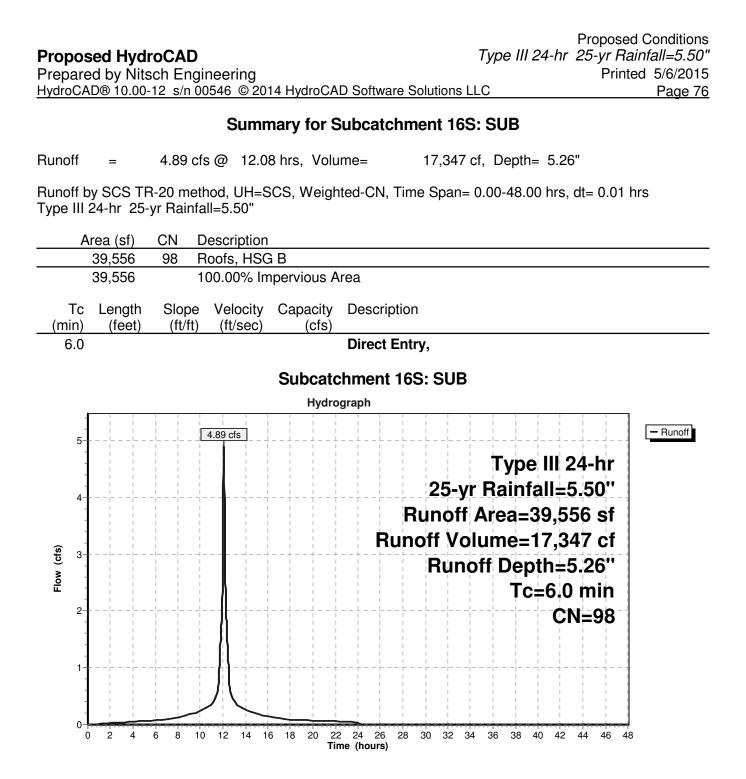
Summary for Reach 16R: DP3-Eastern Property

Inflow Are	a =	102,242 sf, 16.26% Impervious, Inflow Depth = 1.94	1" for 25-yr event
Inflow	=	3.96 cfs @ 12.18 hrs, Volume= 16,488 cf	
Outflow	=	3.96 cfs @ 12.18 hrs, Volume= 16,488 cf, A	ten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach 16R: DP3-Eastern Property



Summary for Reach 17R: DP2-Wells Avenue

Proposed Conditions

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Inflow Are	a =	249,841 sf, 73.57% Impervious, Inflow Depth = 3.27" for 25-yr event	
Inflow	=	18.35 cfs @ 12.09 hrs, Volume= 68,171 cf	
Outflow	=	18.35 cfs @ 12.09 hrs, Volume= 68,171 cf, Atten= 0%, Lag= 0.0	min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Hydrograph 20 Inflow Outflow 18.35 cfs 19 18 Inflow Area=249,841 sf 17 16 15-14 13 12 (sj) 11 Flow 10-9-8-7-6 5-4-3-2 1 0-10 22 24 26 Time (hours) 8 12 14 16 18 20 0 2 4 6 28 30 32 34 36 38 40 42 44 46 48

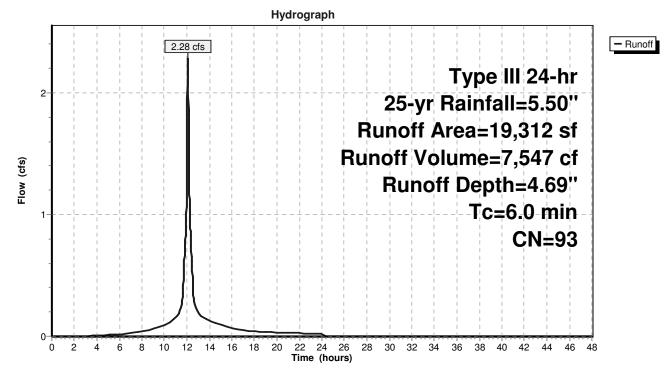
Reach 17R: DP2-Wells Avenue

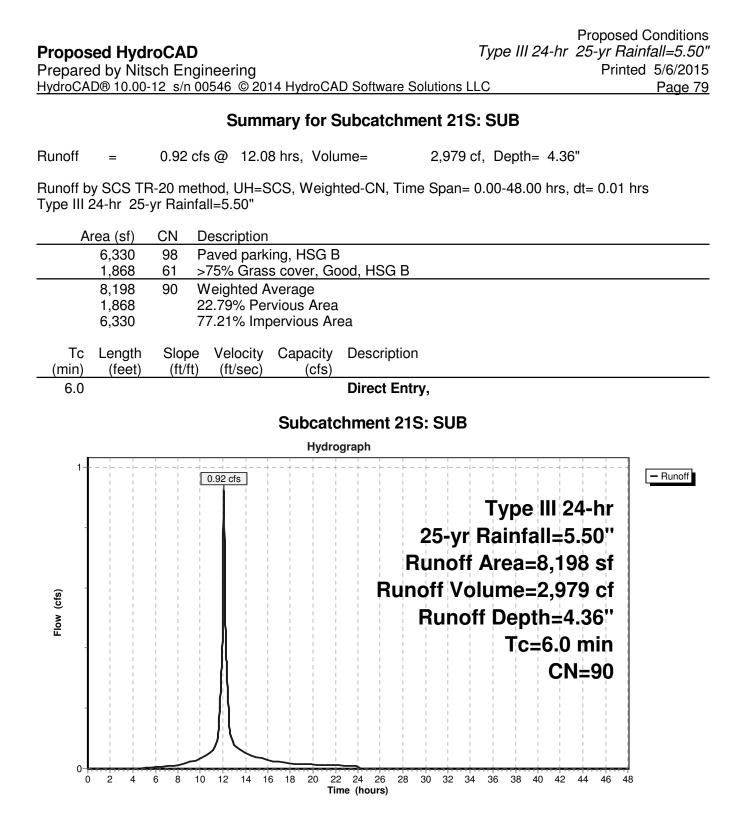
Runoff = 2.28 cfs @ 12.08 hrs, Volume= 7,547 cf, Depth= 4.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25-yr Rainfall=5.50"

	6.0					Direct Entry,		
	Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)			
		19,312 2,823 16,489	93	Weighted A 14.62% Per 85.38% Imp	rvious Area			
*		2,400	98	Pavement		·		
		2,823	61		>75% Grass cover, Good, HSG B			
*		14,089	98	Pavement				
	A	rea (sf)	CN	Description				

Subcatchment 20S: SUB3B

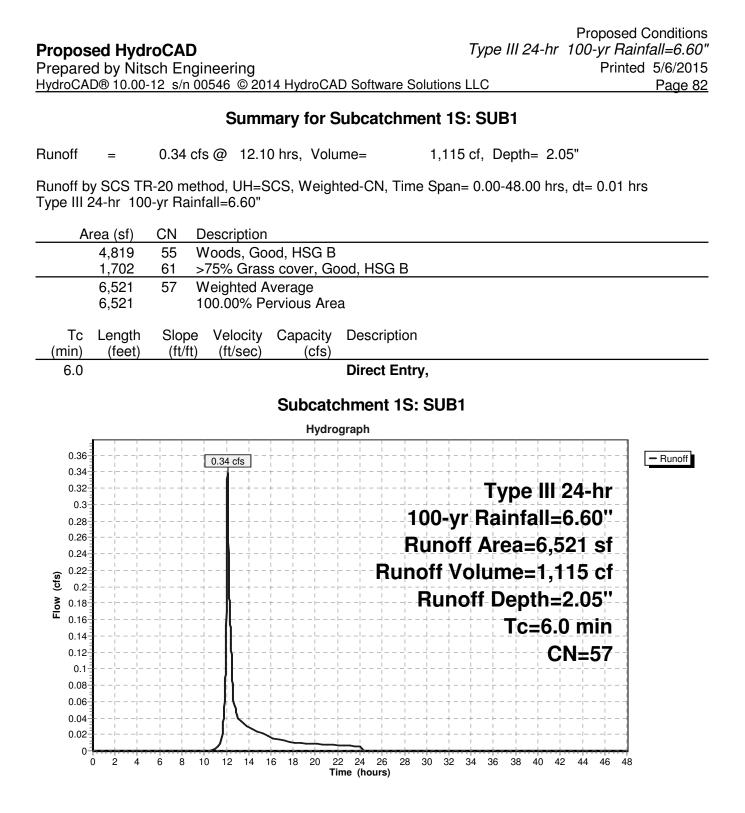




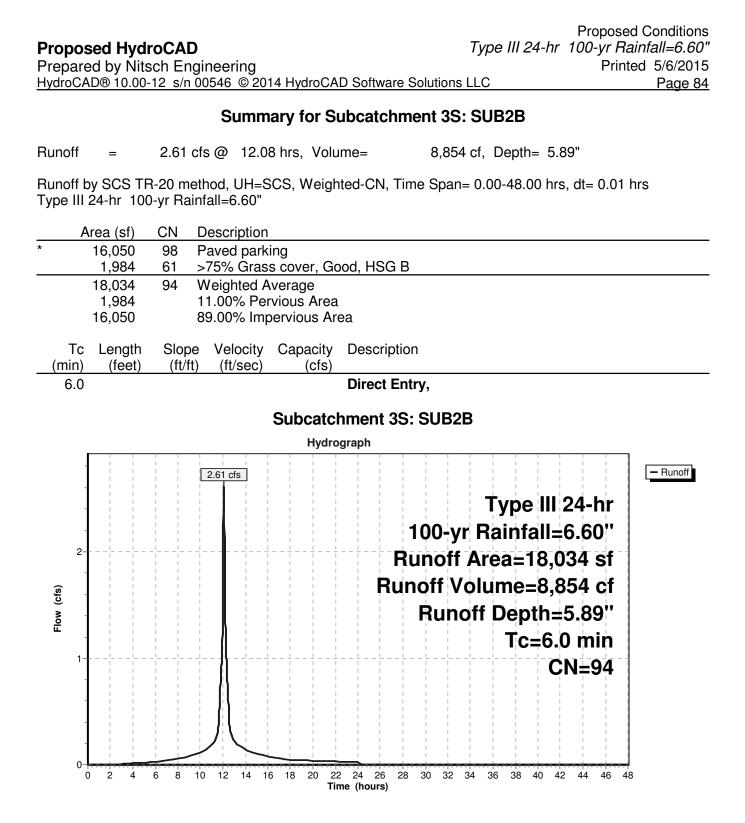
Proposed HydroCAD Prepared by Nitsch Engineering HydroCAD® 10.00-12 s/n 00546 © 2014 Hyd	Proposed Conditions <i>Type III 24-hr 100-yr Rainfall=6.60"</i> Printed 5/6/2015 roCAD Software Solutions LLC Page 80
Runoff by SCS T	0-48.00 hrs, dt=0.01 hrs, 4801 points R-20 method, UH=SCS, Weighted-CN rans method - Pond routing by Stor-Ind method
Subcatchment 1S: SUB1	Runoff Area=6,521 sf 0.00% Impervious Runoff Depth=2.05" Tc=6.0 min CN=57 Runoff=0.34 cfs 1,115 cf
Subcatchment 2S: SUB2A	Runoff Area=28,661 sf 77.18% Impervious Runoff Depth=5.43" Tc=6.0 min CN=90 Runoff=3.97 cfs 12,973 cf
Subcatchment 3S: SUB2B	Runoff Area=18,034 sf 89.00% Impervious Runoff Depth=5.89" Tc=6.0 min CN=94 Runoff=2.61 cfs 8,854 cf
Subcatchment 4S: SUB2C	Runoff Area=25,380 sf 63.98% Impervious Runoff Depth=4.76" Tc=6.0 min CN=84 Runoff=3.19 cfs 10,069 cf
	Peak Elev=102.29' Storage=16,377 cf Inflow=14.04 cfs 46,226 cf 20,364 cf Primary=4.95 cfs 25,862 cf Outflow=5.14 cfs 46,226 cf
Subcatchment 5S: SUB2D	Runoff Area=12,647 sf 81.62% Impervious Runoff Depth=5.55" Tc=6.0 min CN=91 Runoff=1.77 cfs 5,845 cf
Subcatchment 6S: SUB2E	Runoff Area=10,382 sf 54.97% Impervious Runoff Depth=4.43" Tc=6.0 min CN=81 Runoff=1.23 cfs 3,836 cf
Subcatchment 7S: SUB2F	Runoff Area=17,153 sf 86.38% Impervious Runoff Depth=5.78" Tc=6.0 min CN=93 Runoff=2.46 cfs 8,256 cf
Subcatchment 8S: SUB3A	Runoff Area=14,967 sf 30.34% Impervious Runoff Depth=3.49" Tc=8.7 min CN=72 Runoff=1.28 cfs 4,354 cf
Subcatchment 9S: SUB4	Runoff Area=37,456 sf 75.33% Impervious Runoff Depth=5.32" Tc=6.0 min CN=89 Runoff=5.11 cfs 16,600 cf
Subcatchment 10S: SUB5	Runoff Area=93,190 sf 8.24% Impervious Runoff Depth=2.51" Tc=13.8 min CN=62 Runoff=4.76 cfs 19,498 cf
Reach 12R: DP1-Northwest Property	Inflow=0.34 cfs 1,115 cf Outflow=0.34 cfs 1,115 cf
Subcatchment 13S: SUB3B	Runoff Area=9,052 sf 98.80% Impervious Runoff Depth=6.36" Tc=6.0 min CN=98 Runoff=1.35 cfs 4,799 cf
Pond 14P: Infiltration Trench Discarded=0.01 of	Peak Elev=101.60' Storage=419 cf Inflow=1.35 cfs 4,799 cf cfs 1,143 cf Primary=1.30 cfs 3,656 cf Outflow=1.31 cfs 4,799 cf
Subcatchment 14S: SUB2G	Runoff Area=15,595 sf 5.98% Impervious Runoff Depth=2.42" Tc=6.0 min CN=61 Runoff=0.98 cfs 3,141 cf
Subcatchment 15S: SUB	Runoff Area=2,500 sf 100.00% Impervious Runoff Depth=6.36" Tc=6.0 min CN=98 Runoff=0.37 cfs 1,325 cf

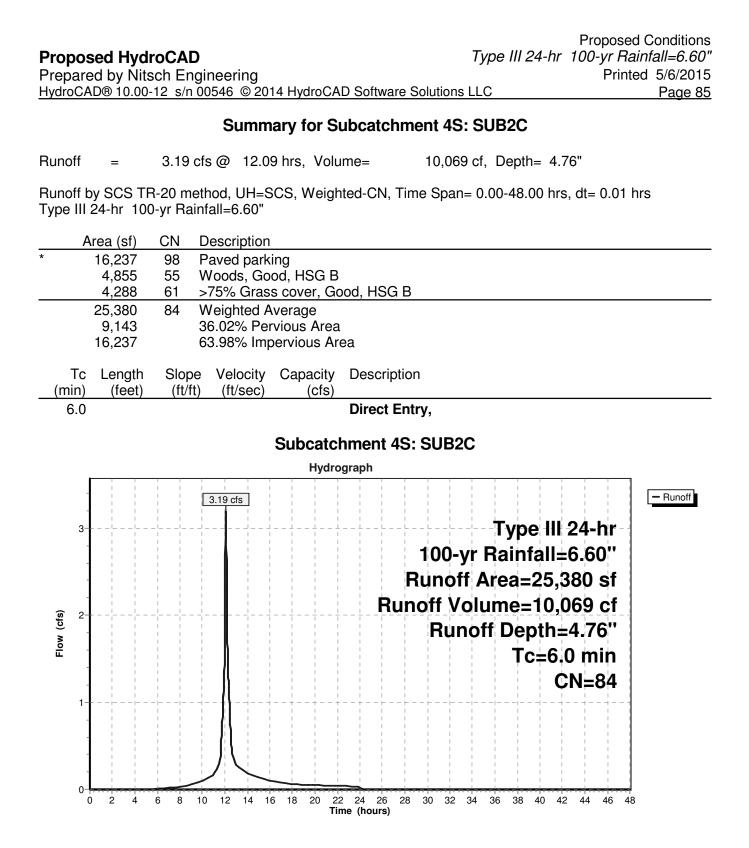
Reach 16R: DP3-Eastern Property	Inflow=5.66 cfs 23,154 cf Outflow=5.66 cfs 23,154 cf
Subcatchment 16S: SUB	Runoff Area=39,556 sf 100.00% Impervious Runoff Depth=6.36" Tc=6.0 min CN=98 Runoff=5.88 cfs 20,969 cf
Reach 17R: DP2-Wells Avenue	Inflow=22.61 cfs 88,862 cf Outflow=22.61 cfs 88,862 cf
Subcatchment 20S: SUB3B	Runoff Area=19,312 sf 85.38% Impervious Runoff Depth=5.78" Tc=6.0 min CN=93 Runoff=2.77 cfs 9,295 cf
Subcatchment 21S: SUB	Runoff Area=8,198 sf 77.21% Impervious Runoff Depth=5.43" Tc=6.0 min CN=90 Runoff=1.13 cfs 3,711 cf

Total Runoff Area = 358,604 sf Runoff Volume = 134,638 cf Average Runoff Depth = 4.51" 44.11% Pervious = 158,164 sf 55.89% Impervious = 200,440 sf



Proposed Hydro Prepared by Nitso HydroCAD® 10.00-1	
	Summary for Subcatchment 2S: SUB2A
Runoff =	3.97 cfs @ 12.08 hrs, Volume= 12,973 cf, Depth= 5.43"
Runoff by SCS TR- Type III 24-hr 100-	20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs yr Rainfall=6.60"
	CN Description
* 22,121 6,540	 98 Paved parking 61 >75% Grass cover, Good, HSG B
28,661 6,540 22,121	90 Weighted Average 22.82% Pervious Area 77.18% Impervious Area
Tc Length (min) (feet)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)
6.0	Direct Entry,
	Subcatchment 2S: SUB2A
-	Hydrograph
4	3.97 cfs
3	Type III 24-hr 100-yr Rainfall=6.60'' Runoff Area=28,661 sf Runoff Volume=12,973 cf Runoff Depth=5.43''
B 2	Tc=6.0 min
	CN=90
0 2 4 6	8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)





Summary for Pond 5P: Subsurface Infiltration System

Inflow Area =	102,085 sf, 78.10% Impervious,	Inflow Depth = 5.43" for 100-yr event
Inflow =	14.04 cfs @ 12.08 hrs, Volume=	46,226 cf
Outflow =	5.14 cfs @ 12.34 hrs, Volume=	46,226 cf, Atten= 63%, Lag= 15.1 min
Discarded =	0.19 cfs @ 7.74 hrs, Volume=	20,364 cf
Primary =	4.95 cfs @ 12.34 hrs, Volume=	25,862 cf

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 102.29' @ 12.34 hrs Surf.Area= 7,912 sf Storage= 16,377 cf

Plug-Flow detention time= 217.4 min calculated for 46,216 cf (100% of inflow) Center-of-Mass det. time= 217.5 min (996.8 - 779.3)

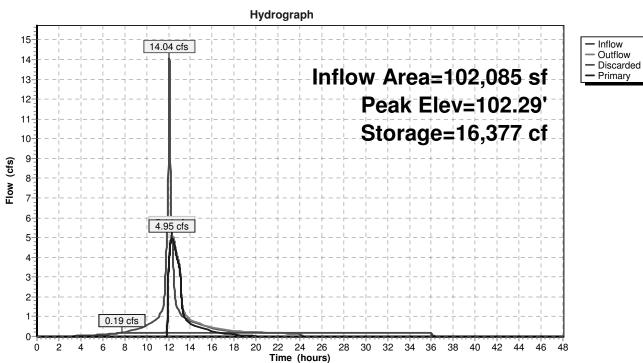
Volume	Invert	Avail.Storage	Storage Description
#1A	98.80'	6,077 cf	68.00'W x 116.36'L x 3.50'H Field A
			27,694 cf Overall - 10,330 cf Embedded = 17,363 cf x 35.0% Voids
#2A	99.30'	10,330 cf	ADS_StormTech SC-740 x 224 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			Row Length Adjustment= +0.44' x 6.45 sf x 14 rows
		16,407 cf	Total Available Storage

Storage Group A created with Chamber Wizard

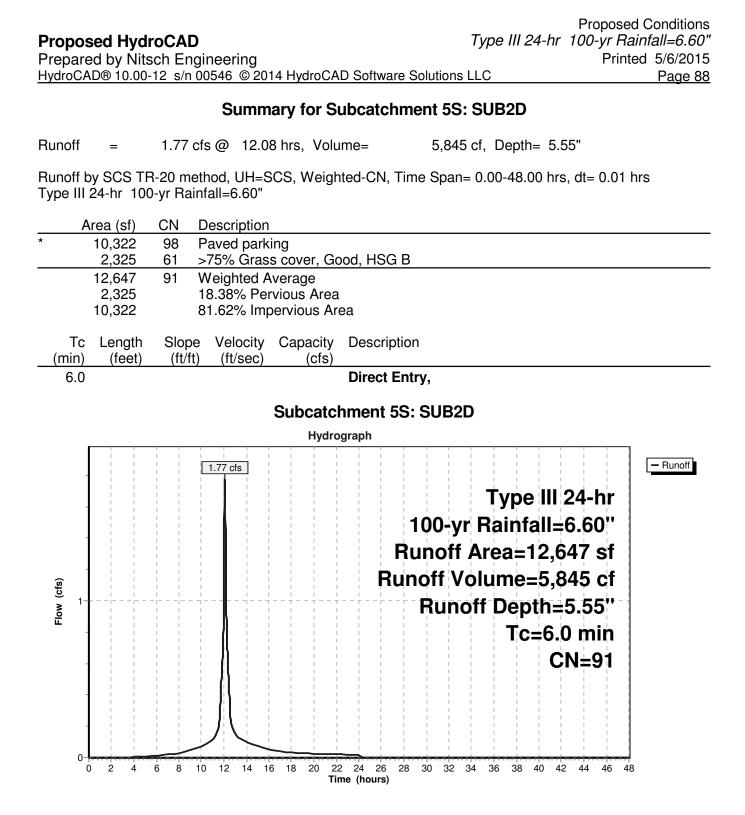
Device	Routing	Invert	Outlet Devices
#1	Discarded	98.80'	1.020 in/hr Exfiltration over Horizontal area
#2	Primary	99.30'	12.0" Round Culvert out of OCS
			L= 95.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 99.30' / 98.40' S= 0.0095 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	100.40'	5.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

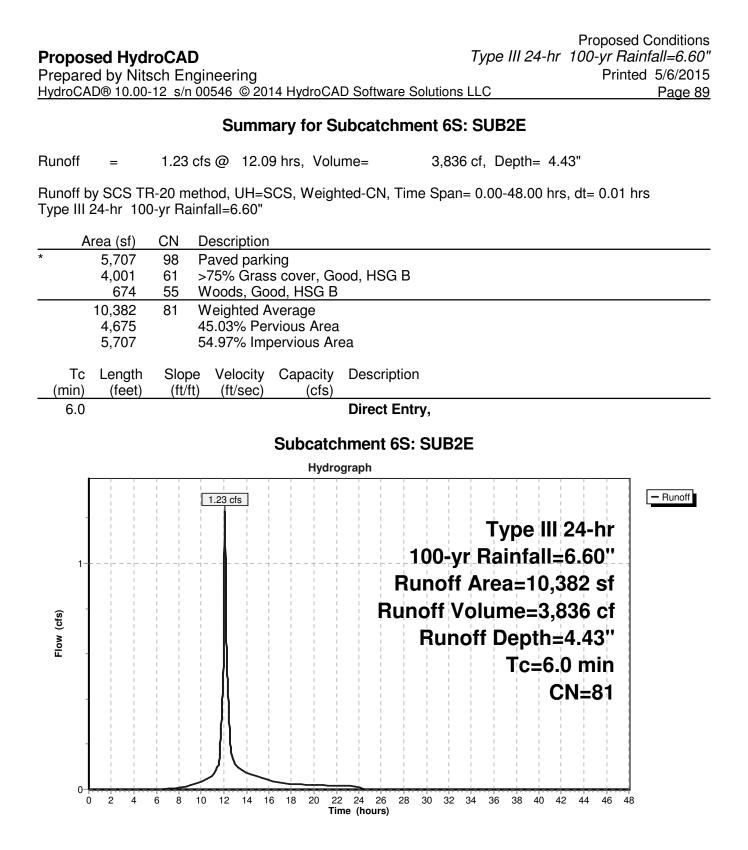
Discarded OutFlow Max=0.19 cfs @ 7.74 hrs HW=98.84' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.19 cfs)

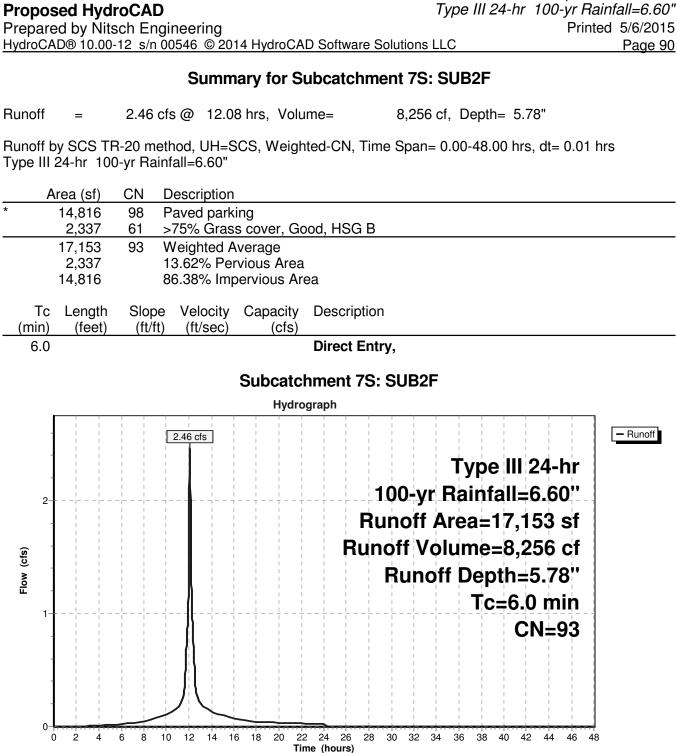
Primary OutFlow Max=4.95 cfs @ 12.34 hrs HW=102.29' (Free Discharge) 2=Culvert out of OCS (Barrel Controls 4.95 cfs @ 6.30 fps) 3=Sharp-Crested Rectangular Weir (Passes 4.95 cfs of 39.24 cfs potential flow)



Pond 5P: Subsurface Infiltration System







Proposed HydroCAD		Type III 24-hr 100-yr Rainfall=6.60"
Prepared by Nitsch Engine HydroCAD® 10.00-12 s/n 0054	ering 46 © 2014 HydroCAD Software S	olutions LLC Printed 5/6/2015
	Summary for Subcatchme	
Runoff = 1.28 cfs @	@ 12.13 hrs, Volume=	4,354 cf, Depth= 3.49"
Runoff by SCS TR-20 method Type III 24-hr 100-yr Rainfall		e Span= 0.00-48.00 hrs, dt= 0.01 hrs
	scription	
	ervious i% Grass cover, Good, HSG B	
14,967 72 We	ighted Average	
	66% Pervious Area 34% Impervious Area	
	/elocity Capacity Description (ft/sec) (cfs)	
8.7		y, NO CHANGE FROM EXISTING
	Subcatchment 8S:	SUB3A
	Hydrograph	
	^{18 cfs}	Type III 24-hr 00-yr Rainfall=6.60''
Eigenversion of the second sec	Run	inoff Area=14,967 sf off Volume=4,354 cf Runoff Depth=3.49'' Tc=8.7 min CN=72

22 24 26 Time (hours)

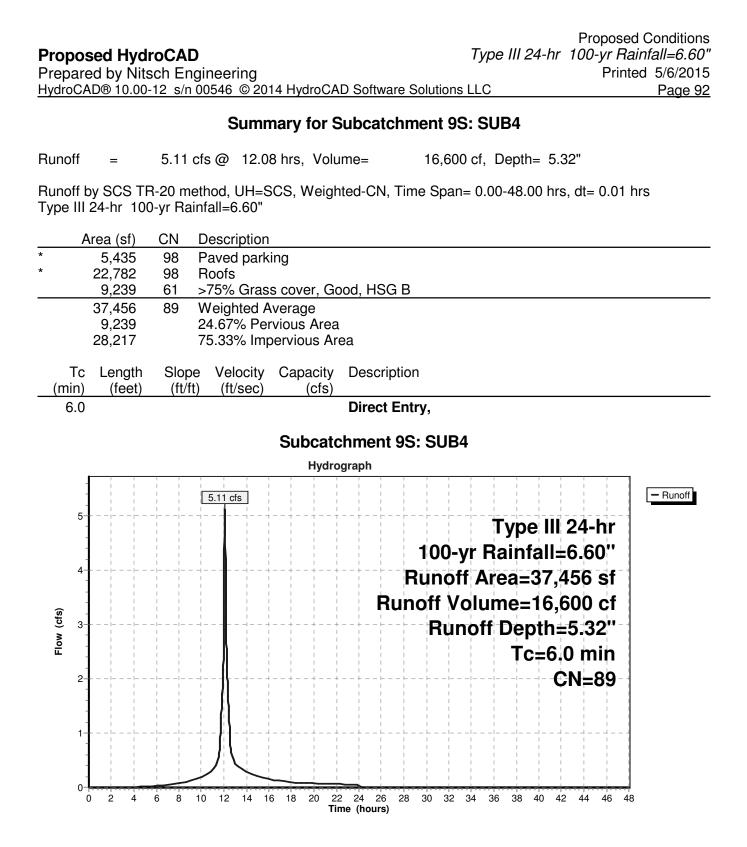
28 30 32 34 36 38 40 42 44 46 48

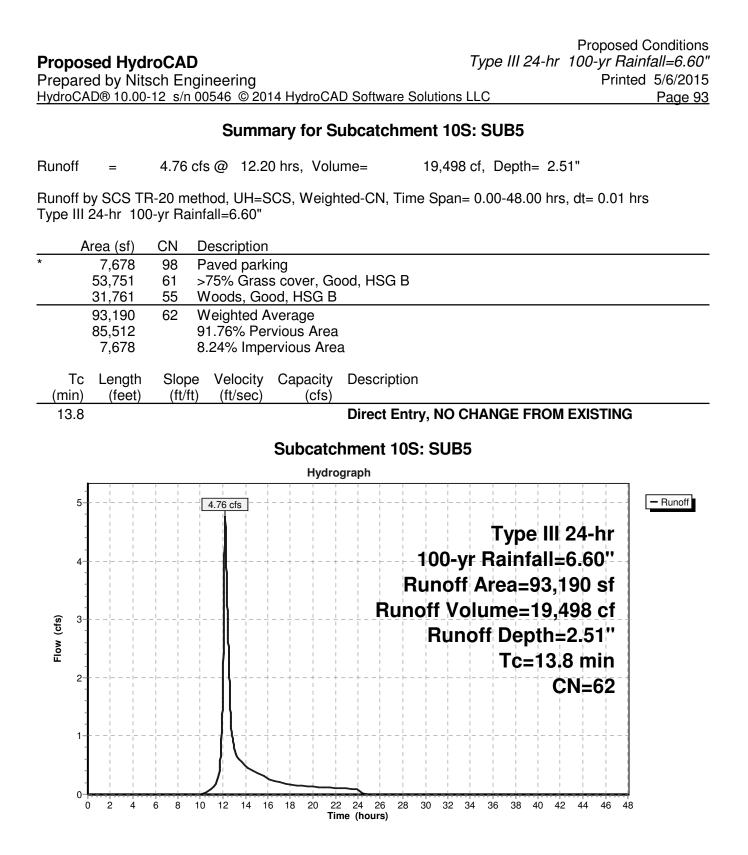
0+ 0

2

4 6 8 10 12 14 16 18 20

Proposed Conditions





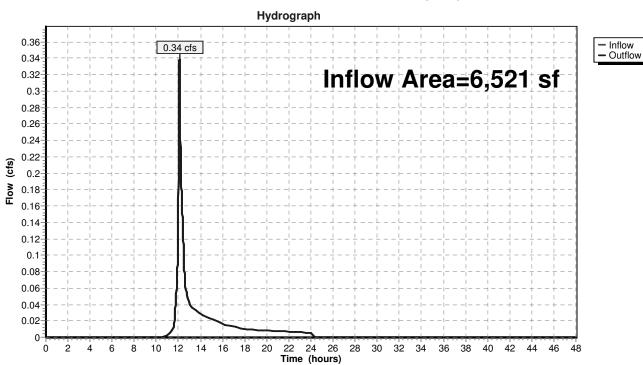
Proposed Conditions

Printed 5/6/2015

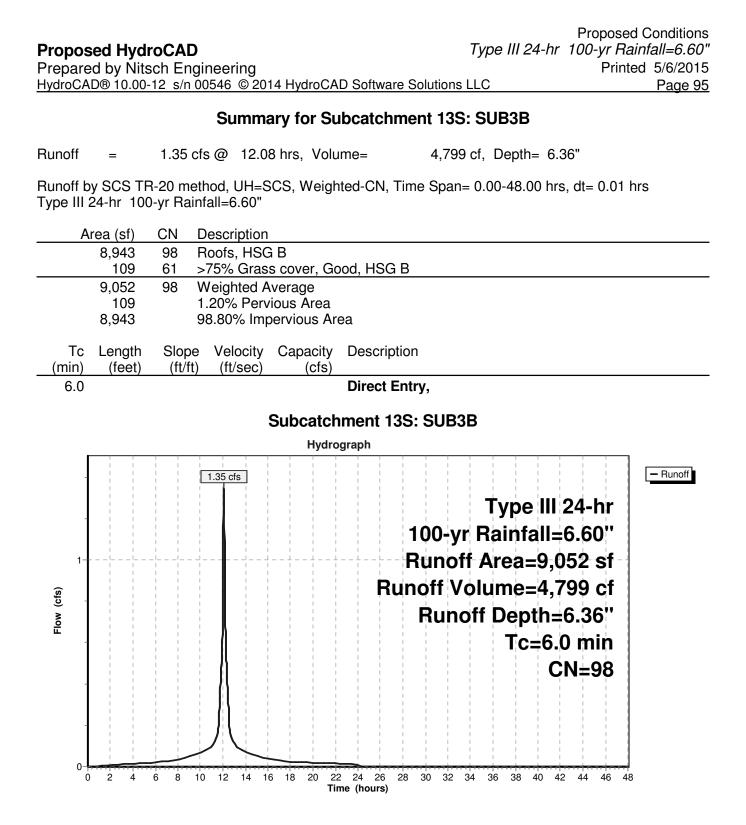
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Inflow Area =	6,521 sf,	0.00% Impervious,	Inflow Depth = 2.05" for 100-yr event
Inflow =	-	12.10 hrs, Volume=	1,115 cf
Outflow =	0.34 cfs @ 1	12.10 hrs, Volume=	1,115 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach 12R: DP1-Northwest Property



Summary for Pond 14P: Infiltration Trench

Inflow Area =	9,052 sf, 98.80% Impervious,	Inflow Depth = 6.36" for 100-yr event
Inflow =	1.35 cfs @ 12.08 hrs, Volume=	4,799 cf
Outflow =	1.31 cfs @ 12.10 hrs, Volume=	4,799 cf, Atten= 3%, Lag= 1.1 min
Discarded =	0.01 cfs @ 3.22 hrs, Volume=	1,143 cf
Primary =	1.30 cfs @ 12.10 hrs, Volume=	3,656 cf

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 101.60' @ 12.10 hrs Surf.Area= 458 sf Storage= 419 cf

Plug-Flow detention time= 74.1 min calculated for 4,798 cf (100% of inflow) Center-of-Mass det. time= 74.2 min (818.0 - 743.8)

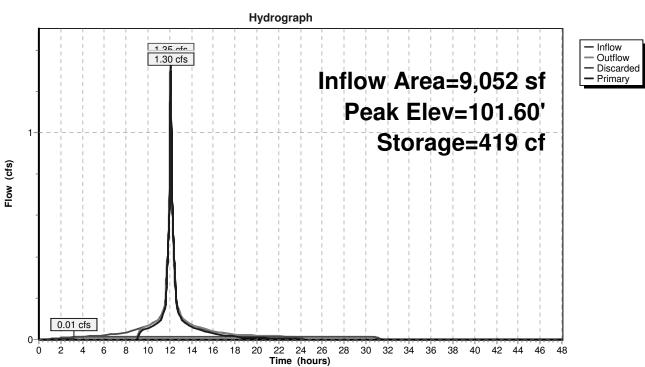
Volume	Inver	t Avail.Sto	rage	Storage	Description	
#1	99.50	' 60)5 cf	Stone (F	Prismatic) Listed	below (Recalc)
#2	101.00	' (90 cf		ound Pipe Stora	mbedded = 1,513 cf x 40.0% Voids ige Inside #1
		69	95 cf	Total Av	ailable Storage	
Elevatio (fee		Gurf.Area (sq-ft)		.Store c-feet)	Cum.Store (cubic-feet)	
99.5	50	458		0	0	
103.0	0	458		1,603	1,603	
Device	Routing	Invert	Outl	et Device	S	
#1	Primary	101.00'		" Round		
						eadwall, Ke= 0.500 100.20' S= 0.0154 '/' Cc= 0.900
			n= 0	.013 Cor	rrugated PE, smo	both interior, Flow Area= 0.79 sf
#2	Discarded	99.50'	1.02	0 in/hr Ex	xfiltration over S	Surface area
D ' I		N 0.04 (~ ~			

Discarded OutFlow Max=0.01 cfs @ 3.22 hrs HW=99.54' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=1.30 cfs @ 12.10 hrs HW=101.60' (Free Discharge) **1=Culvert** (Inlet Controls 1.30 cfs @ 2.64 fps)

Proposed HydroCAD

Prepared by Nitsch Engineering HydroCAD® 10.00-12 s/n 00546 © 2014 HydroCAD Software Solutions LLC



Pond 14P: Infiltration Trench

Proposed Conditions *Type III 24-hr 100-yr Rainfall=6.60"* Printed 5/6/2015 LLC Page 97

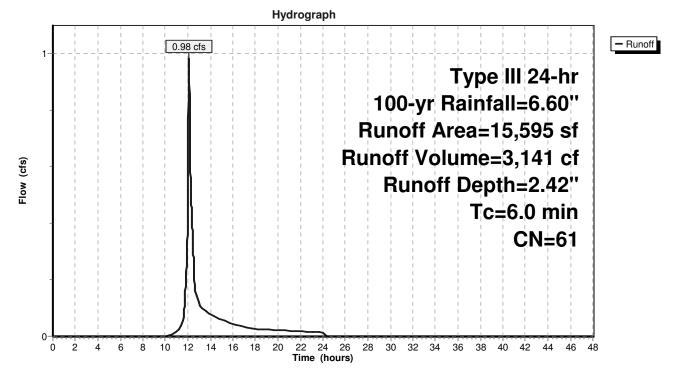
Summary for Subcatchment 14S: SUB2G

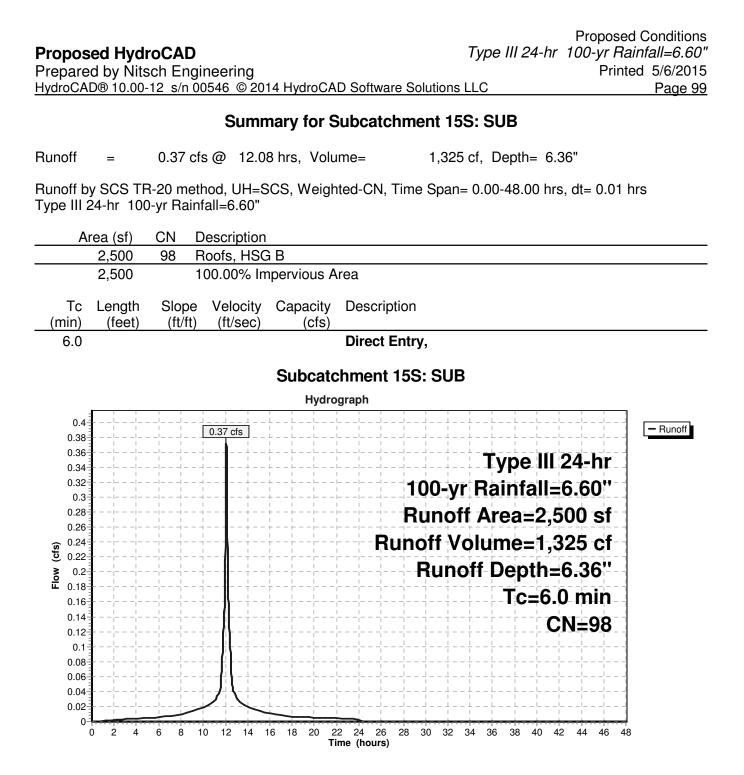
Runoff = 0.98 cfs @ 12.09 hrs, Volume= 3,141 cf, Depth= 2.42"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfall=6.60"

A	Area (sf)	CN	Description						
*	933	98	Paved park	ing					
	10,125	61	>75% Gras	s cover, Go	ood, HSG B				
	4,537	55	Woods, Go	od, HSG B					
	15,595	61	Weighted A	verage					
	14,662		94.02% Pervious Area						
	933		5.98% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft	•	Capacity (cfs)	Description				
6.0					Direct Entry,				

Subcatchment 14S: SUB2G





Summary for Reach 16R: DP3-Eastern Property

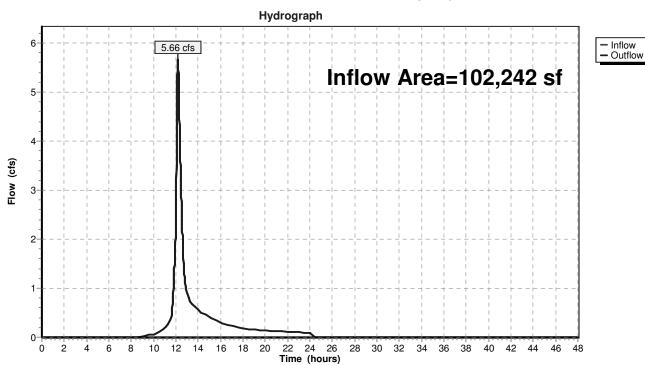
Proposed Conditions

Printed 5/6/2015

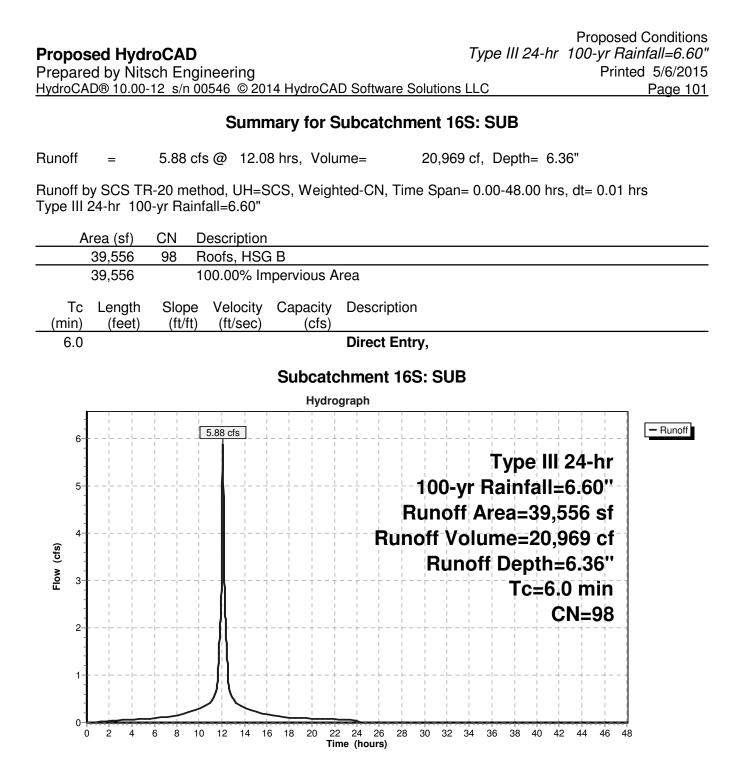
Page 100

Inflow Are	a =	102,242 sf, 16.26% Impervious, Inflow	v Depth = 2.72" for 100-yr event
Inflow	=	5.66 cfs @ 12.18 hrs, Volume=	23,154 cf
Outflow	=	5.66 cfs @ 12.18 hrs, Volume=	23,154 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs



Reach 16R: DP3-Eastern Property



Summary for Reach 17R: DP2-Wells Avenue

Proposed Conditions

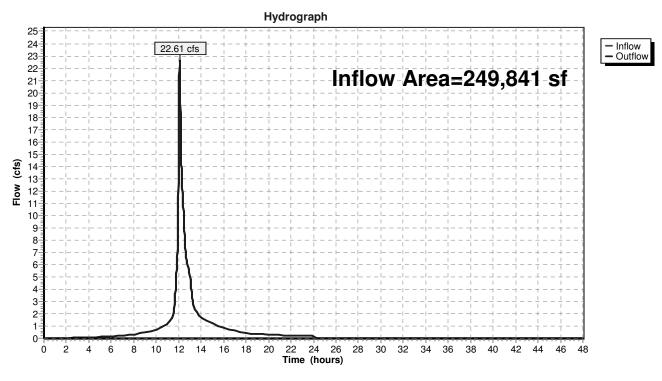
Printed 5/6/2015

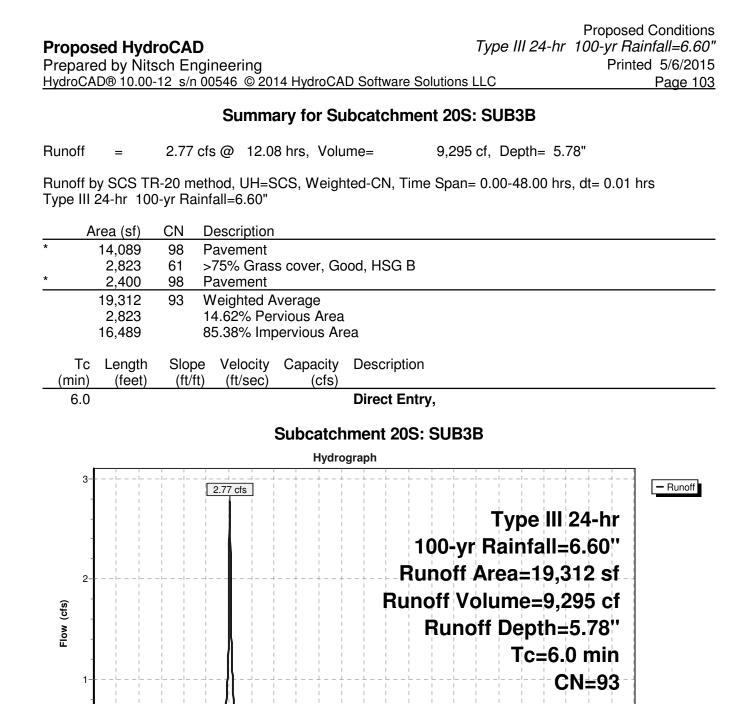
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Inflow Are	a =	249,841 sf, 73.57% Impervious, Inflow Depth = 4.27" for 100-yr event	
Inflow	=	22.61 cfs @ 12.09 hrs, Volume= 88,862 cf	
Outflow	=	22.61 cfs @ 12.09 hrs, Volume= 88,862 cf, Atten= 0%, Lag= 0.0 mir	iin

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach 17R: DP2-Wells Avenue





22 24 26

Time (hours)

28 30 32 34 36

38 40

42 44

46 48

0

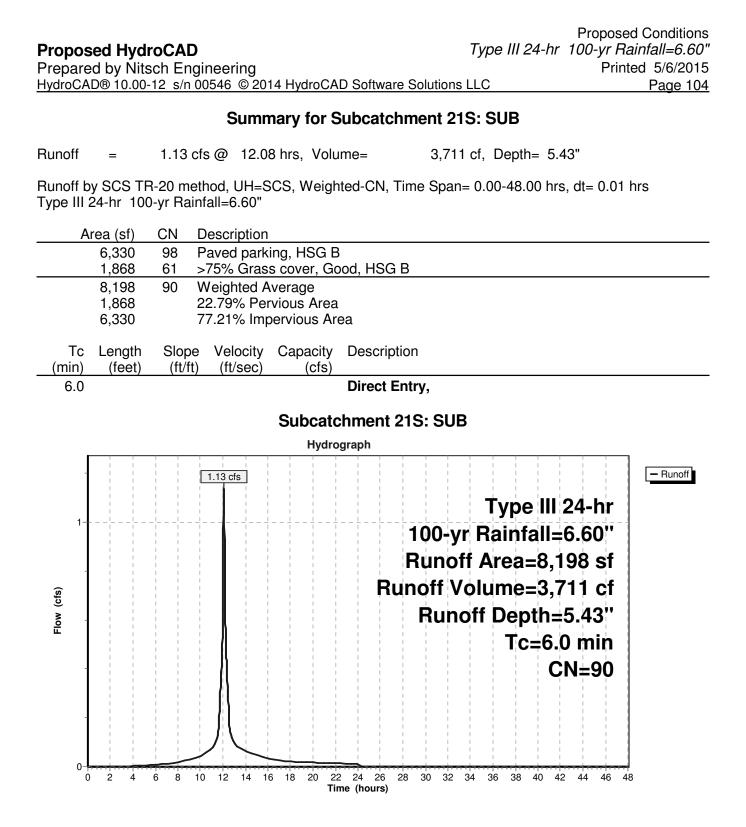
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10 12 14 16 18 20



APPENDIX D

Long-Term Pollution Prevention and Stormwater Operation and Maintenance Plan



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LONG-TERM POLLUTION PREVENTION PLAN AND STORMWATER OPERATION AND MAINTENANCE PLAN

2 Wells Avenue, Newton, MA

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1.0 INTRODUCTION

The purpose of this document is to specify the pollution prevention measures and stormwater management system operation and maintenance for the 2 Wells Avenue site. The Responsible Party indicated below shall implement the management practices outlined in this document and proactively conduct operations at the project site in an environmentally responsible manner. Compliance with this Manual does not in any way dismiss the responsible party, owner, property manager, or occupants from compliance with other applicable federal, state or local laws.

Responsible Party: TWO WELLS AVENUE, LLC 145 Rosemary Street Needham, Massachusetts 02494

This Document has been prepared in compliance with City of Newton requirements and Standards 4 and 9 of the 2008 Massachusetts Department of Environmental Protection (MassDEP) Stormwater Management Standards, which state:

Standard 4:

The Long Term Pollution Prevention Plan shall include the proper procedures for the following:

- Good housekeeping
- Storing materials and waste products inside or under cover
- Vehicle washing
- Routine inspections of stormwater best management practices
- Spill prevention and response
- Maintenance of lawns, gardens, and other landscaped areas
- Pet waste management
- Operation and management of septic systems
- Proper management of deicing chemicals and snow

Standard 9:

The Long-Term Operation and Maintenance Plan shall at a minimum include:

- Stormwater management system(s) owner(s)
- The party or parties responsible for operation and maintenance, including how future property owners shall be notified of the presence of the stormwater management system and the requirement for operation and maintenance
- The routine and non-routine maintenance tasks to be undertaken after construction is complete and a schedule for implementing those tasks
- A plan that is drawn to scale and shows the location of all stormwater BMPs in each treatment train along with the discharge point
- A description of public safety features
- An estimated operations and maintenance budget

2.0 LONG-TERM POLLUTION PREVENTION PLAN

The Responsible Party shall implement the following good housekeeping procedures at the project site to reduce the possibility of accidental releases and to reduce safety hazards.

2.1 Storage of Hazardous Materials

To prevent leaks and spills, keep hazardous materials and waste products under cover or inside. Use drip pans or spill containment systems to prevent chemicals from entering the drainage system. Inspect storage areas for materials and waste products at least once per year to determine amount and type of the material on site, and if the material requires disposal.

Securely store liquid petroleum products and other liquid chemicals in federally- and state-approved containers. Restrict access to maintenance personnel and administrators.

2.2 Storage of Waste Products

Collect and store all waste materials in securely lidded dumpster(s) or other secure containers as applicable to the material. Keep dumpster lids closed and the areas around them clean. Do not fill the dumpsters with liquid waste or hose them out. Sweep areas around the dumpster regularly and put the debris in the garbage, instead of sweeping or hosing it into the parking lot. Legally dispose of collected waste on a regular basis.

Segregate liquid wastes, including motor oil, antifreeze, solvents, and lubricants, from solid waste and recycle through hazardous waste disposal companies, whenever possible. Separate oil filters, batteries, tires, and metal filings from grinding and polishing metal parts from common trash items and recycle. These items are not trash and are illegal to dump. Contact a hazardous waste hauler for proper disposal to a hazardous waste collection center.

2.3 Spill Prevention and Response

Implement spill response procedures for releases of significant materials such as fuels, oils, or chemical materials onto the ground or other area that could reasonably be expected to discharge to surface or groundwater.

- For minor spills, keep fifty (50) gallon spill control kits and Speedy Dry at all shop and work areas.
- Immediately contact applicable Federal, State, and local agencies for reportable quantities as required by law.
- Immediately perform applicable containment and cleanup procedures following a spill release.
- Promptly remove and dispose of all material collected during the response in accordance with Federal, State and local requirements. A licensed emergency response contractor may be required to assist in cleanup of releases depending on the amount of the release, and the ability of the Contractor to perform the required response.
- Reportable quantities of chemicals, fuels, or oils are established under the Clean Water Act and enforced through Massachusetts Department of Environmental Protection (DEP).

2.4 Minimize Soil Erosion

Soil erosion facilitates mechanical transport of nutrients, pathogens, and organic matter to surface water bodies. Repair all areas where erosion is occurring throughout the project site. Stabilize bare soil with riprap, seed, mulch, or vegetation.

2.5 Maintenance of Lawns, Gardens, and other Landscaped Areas

Dumping of lawn wastes, brush or leaves or other materials or debris is not permitted in any

Resource Area. Grass clippings, pruned branches and any other landscaped waste should be disposed of or composted in an appropriate location.

2.6 Management of Deicing Chemicals and Snow

The qualified contractor selected for snow plowing and deicing shall be made fully aware of the requirements of this section.

No road salt (sodium chloride) shall be stored on-site. The use of magnesium chloride de-icing product with a 0.5 to 1.0 percent sodium chloride mix for snow and ice treatment is permitted. The product shall be stored in a locked room inside the building and shall be used at exterior stairs and walkways. The snow plow contractor shall adhere to these magnesium chloride use and storage requirements.

During typical snow plowing operations, snow shall be pushed to the designated snow removal areas noted on the Snow Storage Plan (Figure 2). Snow shall not be stockpiled in wetland resource areas or the 100-foot Buffer Zone, catch basins, or grass channels. In severe conditions where snow cannot be stockpiled on site, the snow shall be removed from the site and properly disposed of in accordance with DEP Guideline BRP601-01.

Before winter begins, the property owner and the contractor shall review snow plowing, deicing, and stockpiling procedures. Areas designated for stockpiling should be cleaned of any debris. Street and parking lot sweeping should be followed in accordance with the Operation and Maintenance Plan.

2.7 Coordination with other Permits and Requirements

Certain conditions of other approvals affecting the long term management of the property shall be considered part of this Long Term Pollution Prevention Plan. The Owner shall become familiar with those documents and comply with the guidelines set forth in those documents.

3.0 STORMWATER MANAGEMENT SYSTEM OPERATION AND MAINTENANCE PLAN

3.1 Introduction

This Operation and Maintenance Plan (O&M Plan) for the 2 Wells Avenue site is required by the City of Newton Engineering Division and under Standard 9 of the 2008 MassDEP Stormwater Handbook to provide best management practices for implementing maintenance activities for the stormwater management system in a manner that minimizes impacts to wetland resource areas.

The Owner shall implement this O&M Plan and proactively conduct operations at the site in an environmentally responsible manner. Compliance with this O&M Plan does not in any way dismiss the Owner from compliance with other applicable Federal, State or local laws.

All stormwater best management practices (BMPs) shall be operated and maintained in accordance with the design plans and the Operation and Maintenance Plan approved by the issuing authority. The Owner shall:

- a. Maintain an operation and maintenance log for the last three years, including inspections, repairs, replacement and disposal (for disposal the log shall indicate the type of material and the disposal location). This is a rolling log in which the responsible party records all operation and maintenance activities for the past three years; and
- b. Make this log available to the City of Newton upon request.

3.2 Stormwater Operation and Maintenance Requirements

Inspect and maintain the stormwater management system as directed below. Refer to the Stormwater Management System Location Map (Figure 1) for the location of each component of the system. Repairs to any component of the system shall be made as soon as possible to prevent any potential pollutants (including silt) from entering the resource areas.

Deep Sump and Hooded Catch Basins

Inspect catch basins four times per year, including after the foliage season. Other inspection and maintenance requirements include:

- Remove organic material, sediment and hydrocarbons four times per year or whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the basin.
- Always clean out catch basins after street sweeping. If any evidence of hydrocarbons is found during inspection, the material immediately remove using absorbent pads or other suitable measures and dispose of legally. Remove other accumulated debris as necessary.
- Transport and disposal of accumulated sediment off-site shall be in accordance with applicable local, state and federal guidelines and regulations.

<u>Area Drains</u>

Inspect area drains at least once per month and remove debris from the grate. Other inspection and maintenance requirements include:

- Clean out accumulated sediments at least once per year and more frequently as necessary.
- Transport and disposal of accumulated sediment off-site shall be in accordance with applicable local, state and federal guidelines and regulations.

Water Quality Units (Proprietary Separators)

Maintain water quality units according the recommendations set forth by the manufacturer. General inspection and maintenance procedures for proprietary devices are provided below:

- Inspect units following completion of construction, prior to being put into service.
- Inspect units at least twice per year following installation and no less than once per year thereafter.
- Inspect units immediately after any oil, fuel or chemical spill.
- All inspections shall include checking the oil level and sediment depth in the unit. Removal of sediments/oils shall occur per manufacturer recommendations.
- A licensed waste management company shall remove captured petroleum waste products from any oil, chemical or fuel spills and dispose.
- OSHA confined space entry protocols shall be followed if entry into the unit is required.

Tree Box Filter

The tree box filter shall be inspected twice per year during the first year after construction. In subsequent years, the swales shall be inspected annually and after rain events greater than 3 inches in 24 hours. Inspection and maintenance procedures for tree box filters are provided below:

- During and after storm events, the length of time standing water remains in the tree box filters shall be recorded:
 - o If the time is greater than 72 hours, thoroughly inspect the basin for signs of clogging.
 - A corrective action plan shall be developed by a qualified professional to restore infiltrative function. Immediate action shall be taken to implement these corrective measures.
- Inspect and remove trash from surface of filter.
- Inspect surface of filter for erosion and repair as necessary. Remulch void areas.
- Remove and replace all dead and diseased trees that cannot be treated.

Biofiltration Swales

Biofiltration swales shall be inspected twice per year during the first year after construction. In subsequent years, the swales shall be inspected annually and after rain events greater than 3 inches in 24 hours. Inspection and maintenance procedures for drainage channels are provided below:

- Inspect the riprap on the channel bottom and side slopes for signs of erosion and formation of rills and gullies. Replace riprap as necessary.
- Remove accumulated trash and debris.
- Remove sediment as needed. Use hand methods (i.e. a person with a shovel) when cleaning to minimize disturbance to vegetation and underlying soils.
- Snow shall be cleared from swales within 24-hours of a snow storm event.

Subsurface Infiltration System

• Inspect subsurface infiltration system twice per year. Inspect the inlets and observation ports to determine if there is accumulated sediment within the system. Remove all debris and accumulated sediment that may clog the system.

3.3 Street Sweeping

Perform street sweeping at least twice per year, whenever there is significant debris present on roads and parking lots. Street sweeping shall occur in the spring and fall. Sweepings must be handled and disposed of properly according to local and state regulations.

3.4 Repair of the Stormwater Management System

The stormwater management system shall be maintained. The repair of any component of the

system shall be made as soon as possible to prevent any potential pollutants including silt from entering the resource areas or the existing closed drainage system.

3.5 Reporting

The Owner shall maintain a record of drainage system inspections and maintenance (per this Plan) and submit maintenance records to the City of Newton upon request.

INSPECTIO		LIST (TREE	BOX FILTER)
Location:		- 1	Inspector:
Date: Time:			Site Conditions:
Date Since Last Rain Event:			
Inspection Items		ctory (S) or factory (U)	Comments/Corrective Action
Initial Inspection After Plar	nting		
Plants are stable, roots not exposed	S	U	
Surface is at design level	S	U	
Overflow bypass/inlet is functional	S	U	
Debris Cleanup (2 times a year minimu	m, Sprin	g & Fall)	
Litter, leaves, and dead vegetation removed from the system	S	U	
Prune perennial vegetation	S	U	
Standing Water (1 time a year, After lar	ge storm	events)	
No evidence of standing water after 72 hours	S	U	
Short Circuiting & Erosion (1 time a year events)	, After la	rge storm	
No evidence of animal burrows or other holes	S	U	
No evidence of erosion	S	U	
Drought Conditions (As nee	eded)		
Water plants as needed	S	U	
Dead or dying plants	S	U	
Overflow Bypass/Inlet Inspection (1 time storm events)	a year, A	After large	
No evidence of blockage or accumulated leaves	S	U	
Good condition, no need for repair	S	U	
Vegetation Health (once every	3 years)		
Dead or decaying plants removed from the system	S	U	
Corrective Action Neede	ed		Due Date
1.			
2.			
3.			
4.			

INSPECTION CHECKLIS	T (DEEP S	SUMP AND H	OODED CATCH BASINS)
Location:	-		Inspector:
Date: Time:			Site Conditions:
Date Since Last Rain Event:			
Inspection Items		ctory (S) or factory (U)	Comments/Corrective Action
Inspections (2 times a year minimum			
Visual evidence of trash, debris or dumping	S	Ú	
Dead animals or vegetation that could generate odors or gases and could cause complaints	S	U	
Evidence of oil, gasoline, contaminants, or other pollutants	S	U	
Condition of basin. Is there a safety, function, or design problem (need for repair)	S	U	
Vegetation blocking more than 10% of the basin opening (lawn areas)	S	U	
Trash and debris blocking more than 20% of grate surface inlet capacity	S	U	
Missing grate, missing or broken grate members	S	U	
Grout fillet is separated or cracked wider than ½ inch and longer than 1 foot at the joint of outlet pipe; evidence of soil entering through cracks	S	U	
Trash or debris in the basin exceeds 50% of the sump depth from the bottom of basin to invert of the outlet pipe; less than 6 inches clearance from the debris surface to the invert of the outlet pipe	S	U	
Sediment in the basin exceeds 50% of the sump depth from the bottom of basin to invert of the outlet pipe; less than 6 inches clearance from the debris surface to the invert of the outlet pipe	S	U	
Trash or debris blocking outlet pipe	S	U	
Debris Cleanup (2 times a year minimu			4
Remove and legally dispose sediment, trash, and debris	S	U	-
Remove and legally dispose contaminants or pollutants	S	U	
Repair catch basin (as necessary)	S	U	
Replace catch basin castings (as necessary)			
Controlling Run-On (2-4 times	a year)		1
Adjacent vegetated areas show no signs of erosion and run-on to catch basin	S	U	
Corrective Action Neede	ed		Due Date
1.			
2.			
3.			
4.			

INSPECTION CHECKL	IST (SUB	SURFACE IN	FILTRATION SYSTEM)
Location:	, ,		Inspector:
Date: Time:			Site Conditions:
Date Since Last Rain Event:			
Inspection Items	Satisfac	ctory (S) or	Comments/Corrective Action
		factory (U)	
Inspections (1 time a year, After large	e storm ev	ents)	
Visual evidence of trash, debris or dumping	S	U	
Evidence of oil, gasoline, contaminants, or other pollutants in manhole sumps	S	U	
Condition of manholes. Is there a safety, function, or design problem (need for repair)	S	U	
Grout fillet is separated or cracked wider than ½ inch and longer than 1 foot at the joint of inlet/outlet pipes; evidence of soil entering through cracks	S	U	
Sediment observed in piping or manhole sumps	S	U	
Trash or debris blocking inlet/outlet pipe	S	U	
Condition of manhole frame and cover	S	U	
Manhole rungs are determined to be unsafe (missing rungs, misaligned, cracked)			
Maintenance (1 time a year, After large	e storm ev	/ents)	
Remove and legally dispose sediment, trash, and debris	S	U	
Remove and legally dispose contaminants or pollutants	S	U	
Repair manholes (as necessary)	S	U	
Replace manhole castings (as necessary)			
Corrective Action Neede	ed		Due Date
1.			
2.			
3.			
4.			

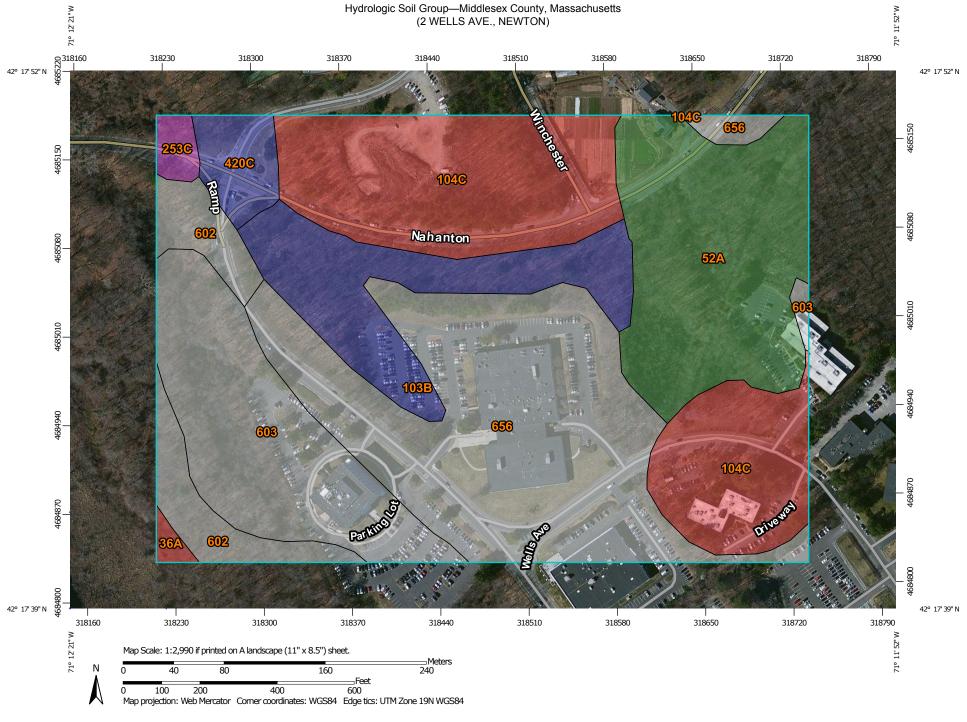
	HECKLIS	ST (WATER Q	UALITY UNITS)
Location:			Inspector:
Date: Time:			Site Conditions:
Date Since Last Rain Event:			
Inspection Items		actory (S) or	Comments/Corrective Action
		sfactory (U)	
Inspections (2 times a year minimum	, Spring a	& Fall)	
Visual evidence of trash, debris or dumping	S	U	
Evidence of oil, gasoline, contaminants, or other pollutants	S	U	
Condition of structure. Is there a safety, function, or design problem (need for repair)	S	U	
Condition of frame and cover	S	U	
Sediment in the basin exceeds manufacturer recommended levels	S	U	
Trash or debris blocking inlet/outlet pipe	S	U	
Debris Cleanup (2 times a year minimu	m, Spring	g & Fall)	
Remove and legally dispose sediment, trash, and debris	S	U	
Remove and legally dispose contaminants or pollutants	S	U	
Repair structure (as necessary)	S	U	
Replace structure castings (as necessary)			
Corrective Action Neede	ed		Due Date
1.			
2.			
3.			
4.			

INSPECTI	ON CHEC	KLIST (ARE	A DRAINS)
Location:		- (Inspector:
Date: Time:			Site Conditions:
Date Since Last Rain Event:			
Inspection Items	Satisfa	ctory (S) or	Comments/Corrective Action
		factory (U)	
Inspections (2 times a year minimum			
Visual evidence of trash, debris or dumping	S	U	
Dead animals or vegetation that could generate odors or gases and could cause complaints	S	U	
Evidence of oil, gasoline, contaminants, or other pollutants	S	U	
Condition of basin. Is there a safety, function, or design problem (need for repair)	S	U	-
Vegetation blocking more than 10% of the basin opening (lawn areas)	S	U	
Trash and debris blocking more than 20% of grate surface inlet capacity	S	U	
Missing grate, missing or broken grate members	S	U	
Trash or debris in the basin exceeds 50% of the sump depth from the bottom of basin to invert of the outlet pipe; less than 6 inches clearance from the debris surface to the invert of the outlet pipe	S	U	
Sediment in the basin exceeds 50% of the sump depth from the bottom of basin to invert of the outlet pipe; less than 6 inches clearance from the debris surface to the invert of the outlet pipe	S	U	
Trash or debris blocking outlet pipe	S	U	
Debris Cleanup (2 times a year minimu	m, Spring	& Fall)	
Remove and legally dispose sediment, trash, and debris	S	U	
Remove and legally dispose contaminants or pollutants	S	U	
Repair area drain (as necessary)	S	U	
Replace area drain castings (as necessary)			
Controlling Run-On (2-4 times	a year)		
Adjacent vegetated areas show no signs of erosion and run-on to area drain	S	U	
Corrective Action Neede	ed		Due Date
1.			
2.			
3.			
4.			

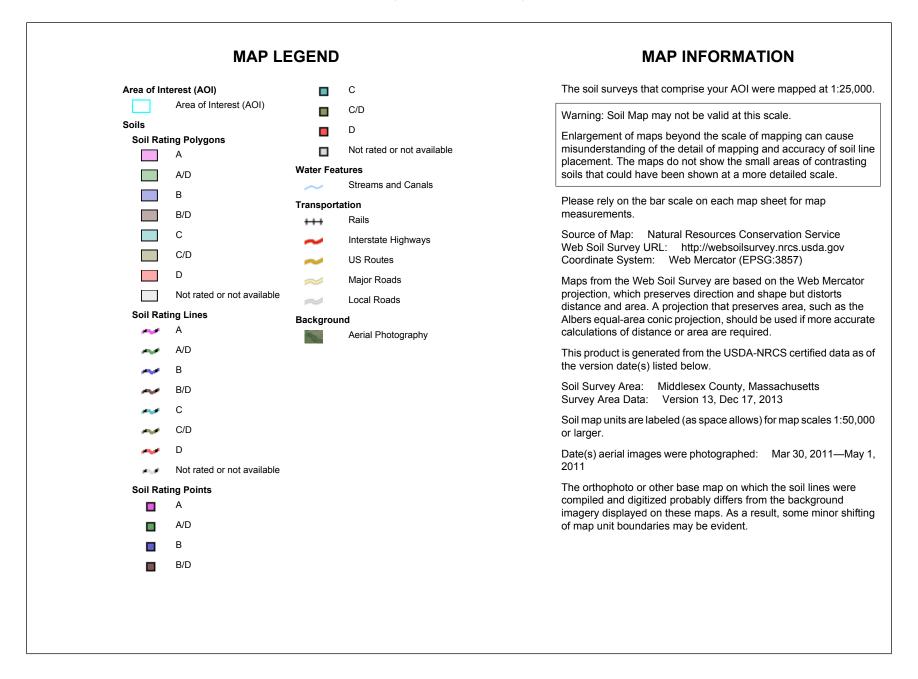
APPENDIX E

Soil Investigations

NRCS Soil Maps and Descriptions Soil Test Pit Logs and Percolation Test Results



USDA Natural Resources Conservation Service Web Soil Survey National Cooperative Soil Survey



Hydrologic Soil Group

Hydrolo	gic Soil Group— Summary	ı by Map Unit — Middle	esex County, Massachusetts	(MA017)
Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
36A	Saco mucky silt loam, 0 to 1 percent slopes	D	0.2	0.4%
52A	Freetown muck, 0 to 1 percent slopes	A/D	7.5	16.6%
103B	Charlton-Hollis-Rock outcrop complex, 3 to 8 percent slopes	В	4.6	10.1%
104C	Hollis-Rock outcrop- Charlton complex, 3 to 15 percent slopes	D	10.3	22.8%
253C	Hinckley loamy sand, 8 to 15 percent slopes	A	0.4	0.9%
420C	Canton fine sandy loam, 8 to 15 percent slopes	В	1.2	2.6%
602	Urban land		2.5	5.4%
603	Urban land, wet substratum		6.4	14.1%
656	Udorthents-Urban land complex		12.3	27.2%
Totals for Area of Inter	rest	1	45.3	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher



A.	Facility Information					
	Two Wells Avenue LLC C/O BAL Management LL	C				
	Owner Name					
	2 Wells Avenue					
	Street Address Newton		MA		Map/Lot # 02459	
	City		State		Zip Code	
B	Site Information					
1.	(Check one) X New Construction	Upgrade		Repair		
~			L.	JSGA		103B/656
2.	Soil Survey Available? X Yes	🗌 No		ource		Soil Map Unit
	Charlton-Hollis-Rock / Udorthents-Urban land Con	nplex		low Depth the groun	dwater	
	Soil Name		Soil Limitations	s 006/USGS	1" =250k'	
3.	Surficial Geological Report Available? 🔀 Yes	🗌 No	If year	ear Published/Source	Publication Scale	Map Unit
	Glacio - Fluvial Deposits			wash and Drift that in	ncludes outwash, e	•
	Geologic/Parent Material		Landform			
4.	Flood Rate Insurance Map					
	Above the 500-year flood boundary? 🛛 Yes	🗌 No	Within the 10	00-year flood boundai	ry? 🗌 Yes	🔀 No
	Within the 500-year flood boundary? 🔲 Yes	🗙 No	Within a velo	ocity zone?	🗌 Yes	🔀 No
_			N/A		DEP 12k I	Map (Charles River
5.	Wetland Area: Wetlands Conservancy P	rogram Map 07/14	Map Unit		Name Easterly V	Vetland Area)
6.	Current Water Resource Conditions (USGS):	Month/Year	Range: 🗌	Above Normal	Normal 🗌 Belov	w Normal
7.	Other references reviewed:					



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserved disposal area)

	Deep Observat	ion Hole Number:	#1	8/26/2014 Date	7:45 am Time	Sunny (75 degree Weather	ees)
1.	Location Ground Elevatio	on at Surface of Hole:	tbd	Location (ident	ify on plan):	Test Hole #1 (See Attac	hed Plan)
2.	Land Use	Parking Lot (e.g., woodland, agricultural f	ield, vacant lot, etc.)	Outwash	None App Surface Stor		3% Slope (%) lot flat area
3.	Distances from:	Vegetation Open Water Body Property Line	1000'* feet 150' feet	Landform Drainage W	/ay fe N	A Other	500'
4.	Parent Material:	Sand, Gravel, Silt	Fill Material	Un	suitable Materials ver(s)	Present: X Yes	No Bedrock
5.	Groundwater Ol Estimated Depth	bserved: Ves h to High Groundwater:	X No 70" down inches	tbo		eping from Pit Depth	Standing Water in Hole

* -Charles River



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Test Hole #1

Deep Observation Hole Number:

Depth (in.)	Soil Horizon/	oil Horizon/ Soil Matrix: Color	Redoximorphic Features (mottles)		Soil Texture		Coarse Fragments % by Volume		Soil Consistence	Other	
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	Other
3"-0"	PAVE	NA				NA					
0-14"	FILL	10YR5/6				Sandy Loam	2%	0%	Massive	Friable	
14"-28"	Buried A	10YR5/2				Sandy Loam	3%	1%	Granular	Friable	
28"-38"	Buried B	10YR5/8				Sandy Loam	4%	2%	Massive	Friable	
38"-62"	C1	10YR5/4				Loamy Sand*1	30%	10%	Massive	Friable	
62"-114"	C2	10YR5/4	70"	Con- 10YR5/8	5%	Loamy Sand*2	30%	4%	Massive	Friable	
114"-118"	C3	10YR6/3	Ļ	Ļ		Silty Loamy Sand*3	4%	1%	Massive	Friable	

Additional Notes:

*1 Many round stones, very coarse, close to a Sand.

*2 - Fewer Stones Close to a Sand

*3 - Very Fine, Still a lot of Grit, Close to Sandy Loam



C. On-Site Review (continued)

	Deep Observat	ion Hole Number: —		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:		Location (identify on	plan): ——		
2.	Land Use	(e.g., woodland, agricultural field	I, vacant lot, etc.)		Surface Stones		Slope (%)
		Vegetation		Landform		Position on Landscape	e (attach sheet)
3.	Distances from:	Open Water Body	feet	 Drainage Way 	feet	Possible Wet Are	ea feet
		Property Line	feet	 Drinking Water \ 	Vell feet	Other	feet
4.	Parent Material:			Unsuital	ole Materials Prese	nt: 🗌 Yes	🗌 No
	If Yes:	Disturbed Soil	Il Material	Impervious Layer(s)	U Weather	red/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	🗌 No	If yes:	Depth Weeping fro	m Pit Depth S	Standing Water in Hole
	Estimated Deptl	h to High Groundwater:	inches	elevation			



C. On-Site Review (continued)

Deep Observation Hole Number:

Depth (in.)		Horizon/ Soil Matrix: Color-	Redoximorphic Features (mottles)		Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other	
	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	other

Additional Notes:



D. Determination of High Groundwater Elevation

1. Method Used:

	Depth observed standing water in observed	ation hole <u>A.</u>		<u> </u>	
		alion noie	inches	inc	hes
	Depth weeping from side of observation	hole	A. 115"*	<u> </u>	
			inches	inc	hes
	X Depth to soil redoximorphic features (mottles)	ottloo)	A. 70"	В.	
		Jules)	inches		hes
	Groundwater adjustment (USGS method		Α.	<u>B.</u>	
			inches	inc	hes
2.					
	Index Well Number	Reading Date		Index Well Level	
	Adjustment Factor	Adjusted Groundwate	r Level		

E. Depth of Pervious Material

- 1. Depth of Naturally Occurring Pervious Material N/A
 - a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
 - 🗌 Yes 🗌 No
 - b. If yes, at what depth was it observed? Upp

Upper boundary: inches

Lower boundary:

inches

* Very slight, bucket teeth were moist.



F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator	Date
Justin Richardson / SE13688	April 23, 2013
Typed or Printed Name of Soil Evaluator / License #	Date of Soil Evaluator Exam

Name of Board of Health Witness

Board of Health

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with <u>Percolation Test Form 12</u>.



Field Diagrams

Use this sheet for field diagrams:

SEE ATTACHED SKETCH



Α.	Facility Information						
	Two Wells Avenue LLC C/O BAL Management LLC	C					
	Owner Name						
	2 Wells Avenue						
	Street Address Newton		MA		Map/Lot # 02459		
	City		State		Zip Code		
B.	Site Information						
1.	(Check one) X New Construction	Upgrade		Repair			
2		□ No	If yooy	USGA		103B/656	
2.	Soil Survey Available? Xes		If yes:	Source		Soil Map Unit	
	Charlton-Hollis-Rock / Udorthents-Urban land Com Soil Name	plex	Soil Limita	Shallow Depth the ground	water		
		□ No	Son Limita	2006/USGS	1" =250k'		
3.	Surficial Geological Report Available? 🔟 Yes	If yes:	Year Published/Source	Publication Scale	Map Unit		
	Glacio - Fluvial Deposits	Glacial Outwash and Drift that includes outwash, eskers, kames					
	Geologic/Parent Material		Landform				
4.	Flood Rate Insurance Map						
	Above the 500-year flood boundary? X Yes	🗌 No	Within th	ne 100-year flood boundary	/? 🗌 Yes	🔀 No	
	Within the 500-year flood boundary?	🔀 No	Within a	velocity zone?	🗌 Yes	📉 No	
	· · -		N/A	·		Map (Charles River	
5.	Wetland Area: Wetlands Conservancy Pr	•	Map Unit		Name Easterly V		
6	Current Water Resource Conditions (USGS):	07/14	Range:	Above Normal X	-	v Normal	
6.		Month/Year	range.			v inoffial	
7.	Other references reviewed:						



Commonwealth of Massachusetts

City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserved disposal area*)

	Deep Observat	ion Hole Number:	#2	8/26/2014 Date	8:10 am Time	Sunny (75 Weather	degrees)
1.	Location	n at Surface of Hole:	tbd	Location (iden	tify on plan):	Test Hole #2 (See	Attached Plan)
2.	l and lise	Grassed edge of Wood (e.g., woodland, agricultural fi Grassed on the Edge	eld, vacant lot, etc.)	Outwash	,		10% Slope (%) running north/south
3.	Distances from:	Vegetation Open Water Body Property Line	1000'* feet 75' feet	Landform Drainage V Drinking V	Vay	Position on Lar NA feet Possible V NA feet Other	vet Area ${500'\over feet} {NA\over feet}$
4.	Parent Material:	Sand, Gravel, Silt		U	nsuitable Material	ls Present:	Yes 🔀 No
5.	Groundwater Ot		Fill Material [X No NA inches	tb	NA	Weathered/Fractured R	Rock 🛛 Bedrock NA Depth Standing Water in Hole

* -Charles River



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Test Hole #2

Deep Observation Hole Number:

Depth (in.)	Soil Horizon	orizon/ Soil Matrix: Color-	Redoximorphic Features (mottles)		Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other	
Deptil (III.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	Other
0"-3"	A	10YR3/2				NA	2%	0%	Granular	Friable	
3"-3""	BEDROCK	NA				Rock					

Additional Notes:



C. On-Site Review (continued)

	Deep Observat	ion Hole Number: —		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:		Location (identify on	plan): —		
2.	Land Use	(e.g., woodland, agricultural field	I, vacant lot, etc.)		Surface Stones		Slope (%)
		Vegetation		Landform		Position on Landscape	e (attach sheet)
3.	Distances from:	Open Water Body	feet	 Drainage Way 	feet	Possible Wet Are	ea feet
		Property Line	feet	 Drinking Water \ 	Vell feet	Other	feet
4.	Parent Material:			Unsuital	ole Materials Prese	nt: 🗌 Yes	🗌 No
	If Yes:	Disturbed Soil	II Material	Impervious Layer(s)	U Weather	red/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	🗌 No	If yes:	Depth Weeping fro	m Pit Depth S	Standing Water in Hole
	Estimated Deptl	h to High Groundwater:	inches	elevation			



C. On-Site Review (continued)

Deep Observation Hole Number:

Depth (in.)	Soil Horizon/ Layer	il Horizon/ Soil Matrix: Color- Layer Moist (Munsell)	Redoximorphic Features (mottles)		Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other	
Depth (m.)			Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	

Additional Notes:



D. Determination of High Groundwater Elevation

1. Method Used:

	Depth observed standing water in observed	votion hole	Α.	E	3.
		alion noie	inches	ir	nches
	Depth weeping from side of observation	holo	Α.	<u>E</u>	3.
			inches	ir	nches
	Depth to soil redoximorphic features (main	ottloc)	<u>A</u> .	E	3.
			inches	ir	nches
	Groundwater adjustment (USGS methodology)		Α.	<u>E</u>	3.
			inches	ir	nches
2.					
	Index Well Number	Reading Date		Index Well Leve	
	Adjustment Factor	Adjusted Groundwate	r Level		

E. Depth of Pervious Material

- 1. Depth of Naturally Occurring Pervious Material NA
 - a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
 - 🗌 Yes 🗌 No
 - b. If yes, at what depth was it observed? Upper boundary:

undary: inches

Lower boundary:

inches



F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator	Date	
Justin Richardson / SE13688	April 23, 2013	
Typed or Printed Name of Soil Evaluator / License #	Date of Soil Evaluator Exam	

Name of Board of Health Witness

Board of Health

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with <u>Percolation Test Form 12</u>.



Field Diagrams

Use this sheet for field diagrams:

SEE ATTACHED SKETCH



Α.	Facility Information						
	Two Wells Avenue LLC C/O BAL Management LLC	2					
	Owner Name						
	2 Wells Avenue						
	Street Address Newton		MA		Map/Lot # 02459		
	City		State		Zip Code		
B.	Site Information						
1.	(Check one) X New Construction	Upgrade		🗌 Repair			
2	Soil Survey Available?	□ No	If yooy	USGA		103B/656	
2.	, _	—	If yes:	Source		Soil Map Unit	
	Charlton-Hollis-Rock / Udorthents-Urban land Com Soil Name	plex	Soil Limita	Shallow Depth the ground	water		
		□ No	Soli Limita	2006/USGS	1" =250k'		
3.	Surficial Geological Report Available? 🔀 Yes	If yes:	Year Published/Source	Publication Scale	Map Unit		
	Glacio - Fluvial Deposits	Glacial Outwash and Drift that includes outwash, eskers, kames					
	Geologic/Parent Material		Landform				
4.	Flood Rate Insurance Map						
	Above the 500-year flood boundary? Xes	🗌 No	Within th	ne 100-year flood boundary	/? 🗌 Yes	🔀 No	
	Within the 500-year flood boundary?	🔀 No	Within a	velocity zone?	Yes	📉 No	
	· · _		N/A	·		Map (Charles River	
5.	Wetland Area: Wetlands Conservancy Pro	•	Map Unit		Name Easterly W		
e	Current Water Resource Conditions (USGS):	07/14	Dongo	Above Normal X		v Normal	
6.		Month/Year	Range:			v inoffilal	
7.	Other references reviewed:						



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserved disposal area*)

	Deep Observat	ion Hole Number:	#3	8/26/2014 Date	8:20 am Time	Sunny (75 degre Weather	es)
1.	Location Ground Elevation	on at Surface of Hole:	tbd	Location (identify	on plan):	Test Hole #3 (See Attack	hed Plan)
2.	Land Use	Grassed edge of Wood (e.g., woodland, agricultural to Grassed on the Edge	ield, vacant lot, etc.)	Outwash	• •	crop at top of hill nes top of ridge runnir	10% Slope (%) ng north/south
3.	Distances from:	Vegetation	1000'*	Landform Drainage Way Drinking Wate	N	et Possible wet Ar A Other	500'
4.	Parent Material:			_	uitable Materials		🔀 No
5.	Groundwater Ol	Disturbed Soil	Fill Material [No NA inches	Impervious Layer	S: NA Depth Wee	Veathered/Fractured Rock	X Bedrock

* -Charles River



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Test Hole #3

Deep Observation Hole Number:

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Redox	imorphic Fe (mottles)	eatures	Soil Texture	Coarse F % by \	Fragments Volume	Soil	Soil Consistence	Other
Deptil (III.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	Other
0"-10"	A	10YR3/2				Sandy Loam	2%	0%	Massive	Friable	
10"-20"	В	10YR5/8				Sandy Loam	3%	1%	Granular	Friable	
20"-20"	Bedrock	NA				Rock					



C. On-Site Review (continued)

	Deep Observat	ion Hole Number: —		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:		Location (identify on	plan): —		
2.	Land Use	(e.g., woodland, agricultural field	I, vacant lot, etc.)		Surface Stones		Slope (%)
		Vegetation		Landform		Position on Landscape	e (attach sheet)
3.	Distances from:	Open Water Body	feet	 Drainage Way 	feet	Possible Wet Are	ea feet
		Property Line	feet	 Drinking Water \ 	Vell feet	Other	feet
4.	Parent Material:			Unsuital	ole Materials Prese	nt: 🗌 Yes	🗌 No
	If Yes:	Disturbed Soil	Il Material	Impervious Layer(s)	U Weather	red/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	🗌 No	If yes:	Depth Weeping fro	m Pit Depth S	Standing Water in Hole
	Estimated Deptl	h to High Groundwater:	inches	elevation			



C. On-Site Review (continued)

Deep Observation Hole Number:

Depth (in.)		Soil Matrix: Color-		(imorphic Fe (mottles)	atures	Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA) G	Gravel	Cobbles & Stones	Structure	(Moist)	Other



D. Determination of High Groundwater Elevation

1. Method Used:

	Depth observed standing water in observed	ution halo	Α.		B.	
		auon noie	inches		inches	
	Depth weeping from side of observation	holo	Α.		В.	
		inches		inches		
	Depth to soil redoximorphic features (me	ottloc)	<u>A</u> .		В.	
		Jules	inches		inches	
	Groundwater adjustment (USGS method		Α.		В.	
		ology)	inches		inches	
2.						
	Index Well Number	Reading Date		Index Well Le	vel	
	Adjustment Factor	Adjusted Groundwate	er Level			

E. Depth of Pervious Material

- 1. Depth of Naturally Occurring Pervious Material NA
 - a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
 - 🗌 Yes 🗌 No
 - b. If yes, at what depth was it observed? Upper boundary:

undary: inches

Lower boundary:

inches



F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator	Date
Justin Richardson / SE13688	April 23, 2013
Typed or Printed Name of Soil Evaluator / License #	Date of Soil Evaluator Exam

Name of Board of Health Witness

Board of Health

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with <u>Percolation Test Form 12</u>.



Field Diagrams

Use this sheet for field diagrams:

SEE ATTACHED SKETCH



Α.	Facility Information				
	Two Wells Avenue LLC C/O BAL Management LL	C			
	Owner Name				
	2 Wells Avenue			NA	
	Street Address Newton		МА	Map/Lot # 02459	
	City		State	Zip Code	
B.	Site Information				
1.	(Check one) X New Construction	Upgrade	🗌 Repair		
2.	Soil Survey Available?	□ No	If yes: USGA		103B/656
۷.	,	_	Rocky Shallow Depth the ground	dwator	Soil Map Unit
	Charlton-Hollis-Rock / Udorthents-Urban land Con Soil Name	nplex	Soil Limitations	uwalei	
_	_	—	2006/USGS	1" =250k'	
3.	Surficial Geological Report Available? X Yes Glacio - Fluvial Deposits	🗌 No	If yes: Year Published/Source Glacial Outwash and Drift that in	Publication Scale	Map Unit skers, kames
	Geologic/Parent Material		Landform		
4.	Flood Rate Insurance Map				
	Above the 500-year flood boundary? X Yes	🗌 No	Within the 100-year flood boundar	y? 🗌 Yes	🗙 No
	Within the 500-year flood boundary?	X No	Within a velocity zone?	Yes	📉 No
5.	Wetland Area: Wetlands Conservancy P	rogram Man	N/A		Map (Charles River
0.		07/14	Map Unit	Name Easterly V	Vetland Area)
6.	Current Water Resource Conditions (USGS):	Month/Year	Range: 🗌 Above Normal 🛛	Normal 🗌 Belov	w Normal
7.	Other references reviewed:				



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserved disposal area*)

	Deep Observat	ion Hole Number:	#4	8/26/2014 Date	8:30 a Time	<u>m</u>	Sunny (75 degree Weather	ees)
1.	Location		tbd			Test I	Hole #4 (See Attac	hed Plan)
	Ground Elevation	on at Surface of Hole:		Location (ident	ify on plan):			
-		On the edge of wooded	area and open	area	ledg	e outcrop at	top of hill	3%
2.	Land Use	(e.g., woodland, agricultural fi Some large trees and		Outwash	Surfa	ce Stones	toe of ridge, low p	Slope (%) point edge of parking lot
3.	Distances from:	Vegetation Open Water Body Property Line	<u>150'</u>	Landform Drainage W Drinking W	•	NA feet NA	Position on Landscape Possible Wet Ar Other	rea $\frac{500'}{feet}$ NA
4.	Parent Material:	Sand Gravel Silt	feet	Ū	suitable Mat	^{feet} erials Preser	_	feet
	If Yes:	Disturbed Soil	Fill Material [Impervious Lay	yer(s)	U Weathere	ed/Fractured Rock	Bedrock
5.	Groundwater Ol Estimated Dept	bserved: X Yes	□ No 90" (*1) inches	tbo		5" th Weeping fror	n Pit Depth	Standing Water in Hole

* -Charles River

*1 - Seasonal High Ground water appeared to be in the fill layer. It was difficult to determine because mottles were not completely developed.



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Test Hole #4

Deep Observation Hole Number:

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Redox	imorphic Fe (mottles)	atures	Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	
3"-0"	0	NA				NA					
0-84"	FILL	10YR6/6				Sandy Loam	2%	0%	Massive	Friable	
84"-102"	Buried A	10YR2/2	90"	Con- 10YR5/8	5%	Sandy Loam	3%	1%	Granular	Friable	
102"-114"	Buried B	10YR3/4	Ļ	Ļ	Ļ	Sandy Loam	4%	2%	Massive	Friable	
114"-126"	С	10YR6/3	Ļ	•	Ļ	Silty Loamy Sand*	10%	1%	Massive	Friable	

Additional Notes:

* - Very Fine, Still a lot of Grit, Close to Sandy Loam



C. On-Site Review (continued)

	Deep Observat	ion Hole Number: —		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:		Location (identify on	plan): —		
2.	Land Use	(e.g., woodland, agricultural field	I, vacant lot, etc.)		Surface Stones		Slope (%)
		Vegetation		Landform		Position on Landscape	e (attach sheet)
3.	Distances from:	Open Water Body	feet	 Drainage Way 	feet	Possible Wet Are	ea feet
		Property Line	feet	 Drinking Water \ 	Vell feet	Other	feet
4.	Parent Material:			Unsuital	ole Materials Prese	nt: 🗌 Yes	🗌 No
	If Yes:	Disturbed Soil	Il Material	Impervious Layer(s)	U Weather	red/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	🗌 No	If yes:	Depth Weeping fro	m Pit Depth S	Standing Water in Hole
	Estimated Deptl	h to High Groundwater:	inches	elevation			



C. On-Site Review (continued)

Deep Observation Hole Number:

Depth (in.)		Soil Matrix: Color-		(imorphic Fe (mottles)	atures	Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA) G	Gravel	Cobbles & Stones	Structure	(Moist)	Other



D. Determination of High Groundwater Elevation

1. Method Used:

Adjustment Factor	Adjusted Groundwa	ter Level	
Index Well Number	Reading Date		Index Well Level
Groundwater adjustment (USG	S methodology)	A. inches	B. inches
Depth to soil redoximorphic fea	tures (mottles)	inches	inches
Depth to goil redevimerable for	turon (mottlon)	A. 100"	В.
Depth weeping from side of obs	servation noie	inches	inches
No. Double supervision from a side of the	a mustice de alla	A. 115"	В.
Depth observed standing water	in observation hole	inches	inches
		A. ¹²⁰ "	В.

E. Depth of Pervious Material

- 1. Depth of Naturally Occurring Pervious Material N/A
 - a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
 - 🗌 Yes 🗌 No
 - b. If yes, at what depth was it observed? Upper boundary:

undary: inches

Lower boundary:



F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator	Date
Justin Richardson / SE13688	April 23, 2013
Typed or Printed Name of Soil Evaluator / License #	Date of Soil Evaluator Exam

Name of Board of Health Witness

Board of Health

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with <u>Percolation Test Form 12</u>.



Field Diagrams

Use this sheet for field diagrams:

SEE ATTACHED SKETCH



Α.	Facility Information					
	Two Wells Avenue LLC C/O BAL Management LLC	C				
	Owner Name					
	2 Wells Avenue					
	Street Address Newton		MA	Map/Lot # 02459 Zip Code		
	City		State			
B.	Site Information					
1.	(Check one) X New Construction	Upgrade	🗌 Repair			
2.	Soil Survey Available? Xes	□ No	If yes: USGA		103B/656	
	•		Rocky Shallow Depth the ground	water	Soil Map Unit	
	Charlton-Hollis-Rock / Udorthents-Urban land Com Soil Name	piex	Soil Limitations	Water		
~			2006/USGS	1" =250k'		
3.	Surficial Geological Report Available? X Yes Glacio - Fluvial Deposits	🗌 No	If yes: Year Published/Source Glacial Outwash and Drift that inc	Publication Scale	Map Unit skers, kames	
	Geologic/Parent Material		Landform			
4.	Flood Rate Insurance Map					
	Above the 500-year flood boundary? Xes	🗌 No	Within the 100-year flood boundary	/? 🗌 Yes	🗙 No	
	Within the 500-year flood boundary?	🔀 No	Within a velocity zone?	Yes	📉 No	
5.	Wetland Area: Wetlands Conservancy Pr	ogram Man	N/A		Aap (Charles River	
5.	Welland Alea. Wellands Conservancy Fr	07/14	Map Unit	Name Easterly W	/etland Area)	
6.	Current Water Resource Conditions (USGS):	Month/Year	Range: 🗌 Above Normal 🛛 I	Normal 🗌 Belov	v Normal	
7.	Other references reviewed:					



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserved disposal area*)

	Deep Observat	ion Hole Number:	#5	8/26/2014 Date	1(D:10 am ne	Sunny (85 degr Weather	ees)
1.	Location Ground Elevatio	on at Surface of Hole:	tbd	Location (ide	entify on pl	an):	t Hole #5 (See Atta	ched Plan)
2.	Land Use	Parking Lot (e.g., woodland, agricultural f	ield, vacant lot, etc.)	Outwash		ledge outcrop a Surface Stones	at top of hill Middle of parking	3% Slope (%) lot flat area
3.	Distances from:	Vegetation Open Water Body Property Line	1000'* feet 100' feet	Landform Drainage Drinking	e Way Water We	II NA feet NA feet	Position on Landscap - Possible Wet A - Other	500'
4.	Parent Material:		Fill Material] Impervious		Materials Pres	ent: X Yes ered/Fractured Rock	No Bedrock
5.	Groundwater Of Estimated Depth	bserved: Ves h to High Groundwater:	X No 84" down inches	<u>-</u>	If yes: tbd elevation	NA Depth Weeping f	rom Pit Depth	Standing Water in Hole

* -Charles River



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Test Hole #5

Deep Observation Hole Number:

Depth (in.)	Soil Horizon	izon/ Soil Matrix: Color-	Redoximorphic Features . (mottles)		Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other	
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	Other
3"-0"	PAVE	NA				NA					
0-32"	FILL	10YR4/4				Sandy Loam	2%	0%	Massive	Friable	
32"-77"	C1	10YR6/3				Loamy Sand*1	30%	10%	Massive	Friable	
77"-120"	C2	10YR6/3	84"	Con- 10YR5/8	5%	Silty Loamy Sand*2	20%	2%	Massive	Friable	
120"-136"	C3	10YR5/3	Ļ	Ļ	Ļ	Sandy Loam	10%	5%	Massive	Friable	

Additional Notes:

*1 - Many round stones, very coarse, close to a Sand.

*2 - Very Fine, Still a lot of Grit, Close to Sandy Loam



C. On-Site Review (continued)

	Deep Observat	ion Hole Number: —		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:		Location (identify on	plan): ——		
2.	Land Use	(e.g., woodland, agricultural field	I, vacant lot, etc.)		Surface Stones		Slope (%)
		Vegetation		Landform		Position on Landscape	e (attach sheet)
3.	Distances from:	Open Water Body	feet	 Drainage Way 	feet	Possible Wet Are	ea feet
		Property Line	feet	 Drinking Water \ 	Vell feet	Other	feet
4.	Parent Material:			Unsuital	ole Materials Prese	nt: 🗌 Yes	🗌 No
	If Yes:	Disturbed Soil	Il Material	Impervious Layer(s)	U Weather	red/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	🗌 No	If yes:	Depth Weeping fro	m Pit Depth S	Standing Water in Hole
	Estimated Deptl	h to High Groundwater:	inches	elevation			



C. On-Site Review (continued)

Deep Observation Hole Number:

Depth (in.)		Soil Matrix: Color-	Redoximorphic Features (mottles)		Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other	
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	Other



D. Determination of High Groundwater Elevation

1. Method Used:

	Depth observed standing water in observed	votion hole	Α.		В.	
	Depth observed standing water in observed	alion noie	inches		inches	
	Depth weeping from side of observation	hole	Α.		В.	
			inches		inches	
	Depth to soil redoximorphic features (mottles)		A. 84"		В.	
			inches		inches	
	Groundwater adjustment (USGS method	lology)	Α.		В.	
			inches		inches	
2.						
	Index Well Number	Reading Date		Index Well L	evel	
	Adjustment Factor	Adjusted Groundwate	r Level			

E. Depth of Pervious Material

- 1. Depth of Naturally Occurring Pervious Material N/A
 - a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
 - 🗌 Yes 🗌 No
 - b. If yes, at what depth was it observed? Upper boundary:

undary: inches

Lower boundary:

inches



F. Certification

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Signature of Soil Evaluator Justin Richardson / SE13688

Typed or Printed Name of Soil Evaluator / License #

Date April 23, 2013

Date of Soil Evaluator Exam

Name of Board of Health Witness

Board of Health

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with <u>Percolation Test Form 12</u>.



Field Diagrams

Use this sheet for field diagrams:

SEE ATTACHED SKETCH



Α.	Facility Information							
	Two Wells Avenue LLC C/O BAL Management LLC	С						
	Owner Name							
	2 Wells Avenue							
	Street Address Newton		MA		Map/Lot # 02459 Zip Code			
	City		State					
B.	Site Information							
1.	(Check one) X New Construction	Upgrade		🗌 Repair				
2.	Soil Survey Available?	□ No	If yes:	USGA		103B/656		
۷.	,		•	Source Shallow Depth the ground	dwatar	Soil Map Unit		
	Charlton-Hollis-Rock / Udorthents-Urban land Com Soil Name	nplex	Soil Limita		Jwalei	sand and		
		_		2006/USGS	1" =250k'	gravel with		
3.	Surficial Geological Report Available? X Yes	🗌 No	If yes: Year Published/Source		Publication Scale Map Unit			
	Glacio - Fluvial Deposits		Glacial Outwash and Drift that includes outwash, eskers, kames					
	Geologic/Parent Material		Landform					
4.	Flood Rate Insurance Map							
	Above the 500-year flood boundary? X Yes	🗌 No	Within th	ne 100-year flood boundar	y? 🗌 Yes	🔀 No		
	Within the 500-year flood boundary?	🔀 No	Within a	velocity zone?	Yes	📉 No		
_	Matlend Area		N/A			Map (Charles River		
5.	Wetland Area: Wetlands Conservancy Pr	07/14	Map Unit		Name Easterly W	/etland Area)		
6.	Current Water Resource Conditions (USGS):	Month/Year	Range:	Above Normal	Normal 🗌 Belov	v Normal		
7.	Other references reviewed:							



Commonwealth of Massachusetts

City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserved disposal area*)

	Deep Observat	ion Hole Number:	#6	8/26/2014 Date	12:30 pm Time	Sunny (90 degro Weather	ees)
1.	Location Ground Elevatio	on at Surface of Hole:	tbd	Location (identify of	on plan):	Test Hole #6 (See Attac	ched Plan)
2.	Land Use	woodland (e.g., woodland, agricultural Large trees with low b		Outwash	Surface Ston	utcrop showing es bottom of slope	5% Slope (%)
3.	Distances from:	Vegetation Open Water Body Property Line	7 feet 30' feet	Landform Drainage Way Drinking Wate	NA	t Possible Wet A	500'
4.	Parent Material:	Sand, Gravel, Silt	Fill Material [Unsui	table Materials ∣ s) □ W	Present: X Yes	☐ No ⊠ Bedrock
5.	Groundwater Of Estimated Depth	oserved: Ves To High Groundwater:	No NA inches	If yes: NA elevation	Depth wee	ping from Pit Depth	Standing Water in Hole

* -Charles River



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Test Hole #6

Deep Observation Hole Number:

Depth (in.)	Soil Horizon/	n/ Soil Matrix: Color-			atures	Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	Cullor
3"-0"	0	NA				NA					
0-3"	Α	10YR3/2				Sandy Loam	2%	0%	Granular	Friable	
3"-54"	В	10YR6/6	25"	Con- 10YR5/8	5%	Sandy Loam	5%	2%	Massive	Friable	
56"-60"	С	10YR7/3	Ļ	Ļ	\rightarrow	Silty Sandy Loam	5%	1%	Massive	Friable	
60"-60"	Bedrock					Rock					



C. On-Site Review (continued)

	Deep Observat	ion Hole Number: —		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:		Location (identify on	plan): —		
2.	Land Use	(e.g., woodland, agricultural field	I, vacant lot, etc.)		Surface Stones		Slope (%)
		Vegetation		Landform		Position on Landscape	e (attach sheet)
3.	Distances from:	Open Water Body	feet	 Drainage Way 	feet	Possible Wet Are	ea feet
		Property Line	feet	 Drinking Water \ 	Vell feet	Other	feet
4.	Parent Material:			Unsuital	ole Materials Prese	nt: 🗌 Yes	🗌 No
	If Yes:	Disturbed Soil	Il Material	Impervious Layer(s)	U Weather	red/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	🗌 No	If yes:	Depth Weeping fro	m Pit Depth S	Standing Water in Hole
	Estimated Deptl	h to High Groundwater:	inches	elevation			



C. On-Site Review (continued)

Deep Observation Hole Number:

Depth (in.)		Soil Matrix: Color-	Redoximorphic Features (mottles)		Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other	
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	Other



D. Determination of High Groundwater Elevation

1. Method Used:

	Depth observed standing water in above	votion hole	Α.		В.	
	Depth observed standing water in observed sta	valion noie	inches		inches	
	Depth weeping from side of observation	hole	Α.		В.	
		noie	inches		inches	
	Depth to soil redoximorphic features (mottles)		A. 25"		В.	
		iomes)	inches		inches	
	Croundwater adjustment (USCS metho	dology)	Α.		В.	
	Groundwater adjustment (USGS methodology)		inches		inches	
2.						
	Index Well Number	Reading Date		Index Well L	evel	
	Adjustment Factor	Adjusted Groundwater	r Level			

E. Depth of Pervious Material

- 1. Depth of Naturally Occurring Pervious Material N/A
 - a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
 - 🗌 Yes 🗌 No
 - b. If yes, at what depth was it observed? Upper boundary:

undary: inches

Lower boundary:



F. Certification

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Signature of Soil Evaluator Justin Richardson / SE13688

Typed or Printed Name of Soil Evaluator / License #

Date April 23, 2013

Date of Soil Evaluator Exam

Name of Board of Health Witness

Board of Health

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with <u>Percolation Test Form 12</u>.



Field Diagrams

Use this sheet for field diagrams:

SEE ATTACHED SKETCH



Α.	Facility Information								
	Two Wells Avenue LLC C/O BAL Management LLC	с							
	Owner Name								
	2 Wells Avenue								
	Street Address Newton		MA		Map/Lot # 02459 Zip Code				
	City		State						
					1				
B.	Site Information								
1.	(Check one) X New Construction	Upgrade		Repair					
2	Soil Survey Available?	🗌 No	If yoo	USGA		103B/656			
2.	· —		If yes: Source Soil Map Unit						
	Charlton-Hollis-Rock / Udorthents-Urban land Com Soil Name	nplex	Rocky Shallow Depth the groundwater Soil Limitations						
			SOII LIMILA	2006/USGS	1" =250k'				
3.	Surficial Geological Report Available? 🛛 Yes 🗌 No			Year Published/Source	Publication Scale	Map Unit			
	Glacio - Fluvial Deposits		Glacial (Outwash and Drift that in	cludes outwash, e	skers, kames			
	Geologic/Parent Material		Landform						
4.	Flood Rate Insurance Map								
	Above the 500-year flood boundary? X Yes	🗌 No	Within th	ne 100-year flood boundar	y? 🗌 Yes	🔀 No			
	Within the 500-year flood boundary?	🔀 No	Within a	velocity zone?	🗌 Yes	📉 No			
	· · -		N/A	·		Map (Charles River			
5.	Wetland Area: Wetlands Conservancy Pr	•	Map Unit		Name Easterly V				
6.	Current Water Resource Conditions (USGS):	07/14							
0.		Month/Year	Range:	Above Normal		w notitidi			
7.	Other references reviewed:								



Commonwealth of Massachusetts

City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserved disposal area*)

	Deep Observat	ion Hole Number:	<u>#7</u>	8/26/2014 Date	1:20 pm Time	Sunny (90 degre Weather	ees)
1.	Location Ground Elevatio	on at Surface of Hole:	tbd	Location (ider	ntify on plan):	Test Hole #7 (See Attac	hed Plan)
2.	Land Use	Woodland (e.g., woodland, agricultural f larger trees with some	,	Outwash	,	nes toe of slope	10% Slope (%)
3.	Distances from:	Vegetation Open Water Body Property Line	, feet <u>150'</u> feet	Landform Drainage	Way fe	Position on Landscap Position on Landscap Possible Wet A Possible Wet A Other	500'
4.	Parent Material:	Sand, Gravel, Silt	Fill Material	U	Insuitable Materials	Present: Yes	🛛 No
5.	Groundwater Ol		X No NA inches	lf		NA	Standing Water in Hole

* -Charles River



Commonwealth of Massachusetts City/Town of

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Test Hole #7

Deep Observation Hole Number:

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Redox	imorphic Fe (mottles)	eatures	Soil Texture	Coarse F % by	Fragments Volume	Soil	Soil Consistence	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	
0"-8"	A	10YR3/2				Sandy Loam	2%	0%	Massive	Friable	
8"-24"	В	10YR5/8				Sandy Loam	3%	1%	Granular	Friable	
24"-24"	Bedrock	NA				Rock					



C. On-Site Review (continued)

	Deep Observat	ion Hole Number: —		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:		Location (identify on	plan): —		
2.	Land Use	(e.g., woodland, agricultural field	I, vacant lot, etc.)		Surface Stones		Slope (%)
		Vegetation		Landform		Position on Landscape	e (attach sheet)
3.	Distances from:	Open Water Body	feet	 Drainage Way 	feet	Possible Wet Are	ea feet
		Property Line	feet	 Drinking Water \ 	Vell feet	Other	feet
4.	Parent Material:			Unsuital	ole Materials Prese	nt: 🗌 Yes	🗌 No
	If Yes:	Disturbed Soil	II Material	Impervious Layer(s)	U Weather	red/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	🗌 No	If yes:	Depth Weeping fro	m Pit Depth S	Standing Water in Hole
	Estimated Deptl	h to High Groundwater:	inches	elevation			



C. On-Site Review (continued)

Deep Observation Hole Number:

Depth (in.)	Soil Horizon/ Layer	n/ Soil Matrix: Color- Moist (Munsell)	Redoximorphic Features (mottles)		Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other	
Depth (m.)			Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	Child



D. Determination of High Groundwater Elevation

1. Method Used:

	Depth observed standing water in observation hole		Α.		В.	
		alion noie	inches		inches	
	Depth weeping from side of observation	holo	Α.		В.	
			inches		inches	
	Dopth to soil redevimerphic features (m)	ottloc)	Α.		В.	
	Depth to soil redoximorphic features (mottles)		inches		inches	
	Groundwater adjustment (USGS methodology)		Α.		В.	
			inches		inches	
2.						
	Index Well Number	Reading Date		Index Well Le	evel	
	Adjustment Factor	Adjusted Groundwate	r Level			

E. Depth of Pervious Material

- 1. Depth of Naturally Occurring Pervious Material NA
 - a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
 - 🗌 Yes 🗌 No
 - b. If yes, at what depth was it observed? Upper boundary:

undary: inches

Lower boundary:

inches



F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator	Date
Justin Richardson / SE13688	April 23, 2013
Typed or Printed Name of Soil Evaluator / License #	Date of Soil Evaluator Exam

Name of Board of Health Witness

Board of Health

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with <u>Percolation Test Form 12</u>.



Field Diagrams

Use this sheet for field diagrams:

SEE ATTACHED SKETCH



Α.	Facility Information					
	Two Wells Avenue LLC C/O BAL Management LLC	с				
	Owner Name					
	2 Wells Avenue					
	Street Address Newton		MA		Map/Lot # 02459	
	City		State		Zip Code	
					·	
B.	Site Information					
1.	(Check one) X New Construction	Upgrade		🗌 Repair		
2	Soil Survey Available?	🗌 No	If yooy	USGA		103B/656
2.	· —		If yes:	Source	l star	Soil Map Unit
	Charlton-Hollis-Rock / Udorthents-Urban land Com Soil Name	nplex	Soil Limita	Shallow Depth the ground	dwater	
			SOII LIMILA	2006/USGS	1" =250k'	
3.	Surficial Geological Report Available? 🔀 Yes	🗌 No	If yes:	Year Published/Source	Publication Scale	Map Unit
	Glacio - Fluvial Deposits			Outwash and Drift that in	cludes outwash, e	skers, kames
	Geologic/Parent Material		Landform			
4.	Flood Rate Insurance Map					
	Above the 500-year flood boundary? X Yes	🗌 No	Within th	ne 100-year flood boundar	y? 🗌 Yes	🔀 No
	Within the 500-year flood boundary?	🔀 No	Within a	velocity zone?	🗌 Yes	κ Νο
			N/A			Map (Charles River
5.	Wetland Area: Wetlands Conservancy Pr	•	Map Unit		Name Easterly V	
6.	Current Water Resource Conditions (USGS):	07/14	Range:	Above Normal	-	w Normal
0.		Month/Year	itanye.			
7.	Other references reviewed:					



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserved disposal area)

	Deep Observat	ion Hole Number:	#8	8/26/2014 Date	2:20 pm Time	Sunny (90 degre Weather	es)
1.	Location Ground Elevation	on at Surface of Hole:	tbd	Location (identify o	n plan):	Test Hole #8 (See Attac	hed Plan)
2.	Land Use	Woodland (e.g., woodland, agricultural f larger trees with some	,	Top of Ridge	Bedrock O Surface Stone	utcrop Apparent es Top of Slope	10% Slope (%)
3.	Distances from:	Vegetation	1000'*	Landform Drainage Way Drinking Water	Well NA	Possible Wet Ar	500'
4.	Parent Material:	Sand, Gravel, Silt	Fill Material	_	able Materials F	Present: Yes eathered/Fractured Rock	X No
5.	Groundwater Ol		NA inches	Impervious Layer(s If yes: tbd elevation	NA Depth Weep	NA	Standing Water in Hole

* -Charles River



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Test Hole #8

Deep Observation Hole Number:

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Redox	imorphic Fe (mottles)	eatures	Soil Texture	Coarse F % by \	Fragments Volume	Soil	Soil Consistence	Other
Deptil (III.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	•
0"-9"	A	10YR3/2				Sandy Loam	2%	0%	Massive	Friable	
9"-27"	В	10YR5/8				Sandy Loam	3%	1%	Granular	Friable	
27"-27"	Bedrock	NA				Rock					

Additional Notes:



C. On-Site Review (continued)

	Deep Observat	ion Hole Number: —		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:		Location (identify on	plan): ——		
2.	Land Use	(e.g., woodland, agricultural field	I, vacant lot, etc.)		Surface Stones		Slope (%)
		Vegetation		Landform		Position on Landscape	e (attach sheet)
3.	Distances from:	Open Water Body	feet	 Drainage Way 	feet	Possible Wet Are	ea feet
		Property Line	feet	 Drinking Water \ 	Vell feet	Other	feet
4.	Parent Material:			Unsuital	ole Materials Prese	nt: 🗌 Yes	🗌 No
	If Yes:	Disturbed Soil	Il Material	Impervious Layer(s)	U Weather	red/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	🗌 No	If yes:	Depth Weeping fro	m Pit Depth S	Standing Water in Hole
	Estimated Deptl	h to High Groundwater:	inches	elevation			



C. On-Site Review (continued)

Deep Observation Hole Number:

Depth (in.)		Soil Matrix: Color-		(imorphic Fe (mottles)	atures	Soil Texture	0/ 1	ragments /olume	Soil	Soil Consistence	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	Other

Additional Notes:



D. Determination of High Groundwater Elevation

1. Method Used:

	Depth observed standing water in observed	ution halo	Α.		B.	
		auon noie	inches		inches	
	Depth weeping from side of observation	holo	Α.		В.	
			inches		inches	
	Depth to soil redoximorphic features (me	ottloc)	<u>A</u> .		В.	
		Jules	inches		inches	
	Groundwater adjustment (USGS method		Α.		В.	
		ology)	inches		inches	
2.						
	Index Well Number	Reading Date		Index Well Le	vel	
	Adjustment Factor	Adjusted Groundwate	er Level			

E. Depth of Pervious Material

- 1. Depth of Naturally Occurring Pervious Material NA
 - a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
 - 🗌 Yes 🗌 No
 - b. If yes, at what depth was it observed? Upper boundary:

undary: inches

Lower boundary:

inches



F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator Justin Richardson / SE13688

Typed or Printed Name of Soil Evaluator / License #

Date April 23, 2013

Date of Soil Evaluator Exam

Name of Board of Health Witness

Board of Health

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with <u>Percolation Test Form 12</u>.



Field Diagrams

Use this sheet for field diagrams:

SEE ATTACHED SKETCH



Α.	Facility Information				
	Two Wells Avenue LLC C/O BAL Management LLC				
	Owner Name				
	2 Wells Avenue				
	Street Address Newton		MA	Map/Lot # 02459	
	City		State	Zip Code	
B.	Site Information				
1.	(Check one) X New Construction	Upgrade	🗌 Repair		
2.	Soil Survey Available?	□ No	USGA		103B/656
Ζ.	·		If yes: Source		Soil Map Unit
	Charlton-Hollis-Rock / Udorthents-Urban land Com Soil Name	plex	Rocky Shallow Depth the ground Soil Limitations	water	
			2006/USGS	1" =250k'	
3.	Surficial Geological Report Available? 🔀 Yes	🗌 No	If yes: Year Published/Source	Publication Scale	Map Unit
	Glacio - Fluvial Deposits		Glacial Outwash and Drift that inc	ludes outwash, es	skers, kames
	Geologic/Parent Material		Landform		
4.	Flood Rate Insurance Map				
	Above the 500-year flood boundary? Xes	🗌 No	Within the 100-year flood boundary	? 🗌 Yes	🗙 No
	Within the 500-year flood boundary? 🔲 Yes	🗙 No	Within a velocity zone?	Yes	📉 No
-			N/A		lap (Charles River
5.	Wetland Area: Wetlands Conservancy Pro	07/14	Map Unit	Name Easterly W	etland Area)
6.	Current Water Resource Conditions (USGS):	Month/Year	Range: 🗌 Above Normal 🛛 N	Normal 🗌 Belov	v Normal
7.	Other references reviewed:				



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserved disposal area)

	Deep Observat	ion Hole Number:	#9	8/27/2014 Date	7:20 am Time	Sunny (75 degree Weather	ees)
1.	Location Ground Elevatio	on at Surface of Hole:	tbd	Location (iden	tify on plan):	Test Hole #9 (See Attac	hed Plan)
2.	Land Use	Woodland (e.g., woodland, agricultural f some large trees	ield, vacant lot, etc.)	Outwash	• • •	nes Middle of Slope	10% Slope (%)
3.	Distances from:	Vegetation Open Water Body Property Line	1000'* feet 20' feet	Landform - Drainage V - Drinking W	/vay fe	Position on Landscap A Possible Wet An A Other	500'
4.	Parent Material:		Fill Material	Un Impervious La	nsuitable Materials ayer(s) 🛛 🗌 V	Present: X Yes	☐ No □ Bedrock
5.	Groundwater Ol Estimated Dept	bserved: 🔀 Yes h to High Groundwater:	No 91" down inches	tb		eping from Pit Depth	Standing Water in Hole

* -Charles River



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Test Hole #9

Deep Observation Hole Number:

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Redox	imorphic Fe (mottles)	atures	Soil Texture	Coarse F % by \	ragments /olume	Soil	Soil Consistence	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	Other
0"-15"	AO	10YR4/2				NA					
15"-85"	FILL	10YR6/4				Sandy Loam*	2%	0%	Granular	Friable	
85"-115"	C1	10YR5/3	91"	Con- 10YR5/8	5%	Loamy Sand	3%	1%	Massive	Friable	
115"-121"	C2	10YR4/6	*1 🖕	•		Loamy Sand	4%	2%	Massive	Friable	

Additional Notes:

* -A lot of organics mixed in with the fill.

*1 -Soil on teeth of bucket was starting to get moist



C. On-Site Review (continued)

	Deep Observat	ion Hole Number: —		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:		Location (identify on	plan): —		
2.	Land Use	(e.g., woodland, agricultural field	I, vacant lot, etc.)		Surface Stones		Slope (%)
		Vegetation		Landform		Position on Landscape	e (attach sheet)
3.	Distances from:	Open Water Body	feet	 Drainage Way 	feet	Possible Wet Are	ea feet
		Property Line	feet	 Drinking Water \ 	Vell feet	Other	feet
4.	Parent Material:			Unsuital	ole Materials Prese	nt: 🗌 Yes	🗌 No
	If Yes:	Disturbed Soil	Il Material	Impervious Layer(s)	U Weather	red/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	🗌 No	If yes:	Depth Weeping fro	m Pit Depth S	Standing Water in Hole
	Estimated Deptl	h to High Groundwater:	inches	elevation			



C. On-Site Review (continued)

Deep Observation Hole Number:

Depth (in.)		Soil Matrix: Color-		(imorphic Fe (mottles)	atures	Soil Texture	0/ 1	ragments /olume	Soil	Soil Consistence	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	Other

Additional Notes:



D. Determination of High Groundwater Elevation

1. Method Used:

	Depth observed standing water in observed	votion halo	Α.		В.	
	Depth observed standing water in observed	valion noie	inches		inches	
	Depth weeping from side of observation	hole	Α.		В.	
		noic	inches		inches	
	Depth to soil redoximorphic features (m	ottloc)	A. 91"*		В.	
		otties)	inches		inches	
	Groundwater adjustment (USGS method	lology)	Α.		В.	
		lology)	inches		inches	
2.						
	Index Well Number	Reading Date		Index Well L	evel	
	Adjustment Factor	Adjusted Groundwater	Level			

E. Depth of Pervious Material

- 1. Depth of Naturally Occurring Pervious Material N/A
 - a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
 - 🗌 Yes 🗌 No
 - b. If yes, at what depth was it observed? Upper boundary:

indary: inches

Lower boundary:

inches



F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator	Date	
Justin Richardson / SE13688	April 23, 2013	
Typed or Printed Name of Soil Evaluator / License #	Date of Soil Evaluator Exam	

Name of Board of Health Witness

Board of Health

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with <u>Percolation Test Form 12</u>.



Field Diagrams

Use this sheet for field diagrams:

SEE ATTACHED SKETCH



Α.	Facility Information						
	Two Wells Avenue LLC C/O BAL Management LLC						
	Owner Name						
	2 Wells Avenue						
	Street Address Newton	MA			Map/Lot # 02459		
	City		State		Zip Code		
Β.	Site Information						
1.	(Check one) X New Construction	Upgrade		🗌 Repair			
2	Soil Survey Available?	□ No	If yooy	USGA		103B/656	
2.	, _	_	If yes:	Source		Soil Map Unit	
	Charlton-Hollis-Rock / Udorthents-Urban land Com Soil Name	plex	Soil Limita	Shallow Depth the ground	water		
			Son Limita	2006/USGS	1" =250k'		
3.	Surficial Geological Report Available? 🔀 Yes	🗌 No	If yes:	Year Published/Source	Publication Scale	Map Unit	
	Glacio - Fluvial Deposits			Outwash and Drift that inc	cludes outwash, e	skers, kames	
	Geologic/Parent Material		Landform				
4.	Flood Rate Insurance Map						
	Above the 500-year flood boundary? Xes	🗌 No	Within th	ne 100-year flood boundary	/? 🗌 Yes	🔀 No	
	Within the 500-year flood boundary?	🔀 No	Within a	velocity zone?	Yes	📉 No	
	· · -		N/A	·		Map (Charles River	
5.	Wetland Area: Wetlands Conservancy Pro	•	Map Unit		Name Easterly W		
6.	Current Water Resource Conditions (USGS):	07/14	Range:	Above Normal X		v Normal	
0.		Month/Year	Naliye.			v inoffial	
7.	Other references reviewed:						



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserved disposal area*)

	Deep Observat	ion Hole Number:	#10	8/26/2014 Date	8:10 am Time	Sunny (75 degre Weather	es)
1.	Location Ground Elevation	on at Surface of Hole:	tbd	Location (identify	on plan):	Test Hole #10 (See Atta	ched Plan)
2.	Land Use	Grassed Area (e.g., woodland, agricultural fi Grassed on the edge of	, ,	Outwash	Bedrock o Surface Stor	eutcrop at top of hill hes Flat area off next	10% Slope (%) to parking lot
3.	Distances from:	Vegetation Open Water Body Property Line	1000'* feet 10' feet	Landform Drainage Way Drinking Wate	N	et Possible wet Ar	450'
4.	Parent Material:	_	Fill Material	Unsu Impervious Layer	itable Materials	Present: Yes Veathered/Fractured Rock	X No
5.	Groundwater Ol		X No NA inches	If yes tbd elevation	NA Depth Wee	NA	Standing Water in Hole

* -Charles River



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Test Hole #10

Deep Observation Hole Number:

Depth (in.)		/ Soil Matrix: Color- Moist (Munsell)	Redoximorphic Features (mottles)		Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other	
Deptil (iii.)	Layer		Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	other
0"-12"	A	10YR3/2				NA	2%	0%	Granular	Friable	
12"-12"	BEDROCK	NA				Rock					

Additional Notes:



C. On-Site Review (continued)

	Deep Observat	ion Hole Number: —		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:		Location (identify on	plan): —		
2.	Land Use	(e.g., woodland, agricultural field	I, vacant lot, etc.)		Surface Stones		Slope (%)
		Vegetation		Landform		Position on Landscape	e (attach sheet)
3.	Distances from:	Open Water Body	feet	 Drainage Way 	feet	Possible Wet Are	ea feet
		Property Line	feet	 Drinking Water \ 	Vell feet	Other	feet
4.	Parent Material:			Unsuital	ole Materials Prese	nt: 🗌 Yes	🗌 No
	If Yes:	Disturbed Soil	Il Material	Impervious Layer(s)	U Weather	red/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	🗌 No	If yes:	Depth Weeping fro	m Pit Depth S	Standing Water in Hole
	Estimated Deptl	h to High Groundwater:	inches	elevation			



C. On-Site Review (continued)

Deep Observation Hole Number:

Depth (in.)		Soil Matrix: Color- Moist (Munsell)		(imorphic Fe (mottles)	atures	Soil Texture (USDA)	Coarse Fragments % by Volume		Soil	Soil Consistence	Other
Depth (m.)	Layer		Depth	Color	Percent		Gravel	Cobbles & Stones	Structure	(Moist)	0

Additional Notes:



D. Determination of High Groundwater Elevation

1. Method Used:

	Depth observed standing water in observed	ution halo	Α.		B.	
		auon noie	inches		inches	
	Depth weeping from side of observation	holo	Α.		В.	
			inches		inches	
	Depth to soil redoximorphic features (me	ottloc)	<u>A</u> .		В.	
		Jules	inches		inches	
	Groundwater adjustment (USGS method		Α.		В.	
		ology)	inches		inches	
2.						
	Index Well Number	Reading Date		Index Well Le	vel	
	Adjustment Factor	Adjusted Groundwate	er Level			

E. Depth of Pervious Material

- 1. Depth of Naturally Occurring Pervious Material NA
 - a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
 - 🗌 Yes 🗌 No
 - b. If yes, at what depth was it observed? Upper boundary:

undary: inches

Lower boundary:

inches



F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator	Date	
Justin Richardson / SE13688	April 23, 2013	
Typed or Printed Name of Soil Evaluator / License #	Date of Soil Evaluator Exam	

Name of Board of Health Witness

Board of Health

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with <u>Percolation Test Form 12</u>.



Field Diagrams

Use this sheet for field diagrams:

SEE ATTACHED SKETCH



Α.	Facility Information						
	Two Wells Avenue LLC C/O BAL Management LLC	;					
	Owner Name						
	2 Wells Avenue				Map/Lot #		
	Street Address		MA				
	Newton City		State		02459 Zip Code		
	City		State				
B.	Site Information						
1.	(Check one) X New Construction	Upgrade		🗌 Repair			
~		□ No	lf	USGA		103B/656	
2.	Soil Survey Available? Yes	_	If yes:	Source		Soil Map Unit	
	Charlton-Hollis-Rock / Udorthents-Urban land Comp	olex		Shallow Depth the ground	water		
	Soil Name		Soil Limita	ations 2006/USGS	1" =250k'	sand and gravel with	
3.	Surficial Geological Report Available? 🔀 Yes	🗌 No	If yes:	Year Published/Source	Publication Scale	Map Unit	
	Glacio - Fluvial Deposits		Glacial (Outwash and Drift that inc	cludes outwash, es	kers, kames	
	Geologic/Parent Material		Landform				
4.	Flood Rate Insurance Map						
	Above the 500-year flood boundary? 🔀 Yes	🗌 No	Within th	ne 100-year flood boundary	/? 🗌 Yes	🔀 No	
	Within the 500-year flood boundary?	🗙 No	Within a	velocity zone?	Yes	📉 No	
			N/A			<u> 1ap (Charles Rive</u>	
5.	Wetland Area: Wetlands Conservancy Pro	• •	Map Unit		Name Easterly W		
c	Current Water Descures Conditions (USCS)	07/14	Danga			v Normal	
6.	Current Water Resource Conditions (USGS):	Month/Year	Range:	Above Normal X	Normal 🗌 Below	normal	
7.	Other references reviewed:						



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserved disposal area*)

	Deep Observat	ion Hole Number:	#11	8/26/2014 Date	2:20 pm Time	Sunny (90 degre Weather	es)
1.	Location Ground Elevation	on at Surface of Hole:	tbd	Location (identify o	n plan): -	Test Hole #11 (See Atta	ched Plan)
2.	Land Use	Grassed Area (e.g., woodland, agricultural f Grass	ield, vacant lot, etc.)	Outwash	Bedrock ou Surface Stone	utcrop at top of hill s Grassed area new	10% Slope (%) tt to parking lot
3.	Distances from:	Vegetation Open Water Body Property Line	1000'* feet 150' feet	Landform Drainage Way Drinking Water	Well NA feet	Possible Wet Ar	500'
4.	Parent Material:		Fill Material	Unsuita	able Materials F	Present: Ves eathered/Fractured Rock	🔀 No
5.	Groundwater Ol Estimated Dept	bserved: Ves h to High Groundwater:	No NA inches	If yes: tbd elevation	NA Depth Weep	bing from Pit Depth 3	Standing Water in Hole

* -Charles River



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Test Hole #11

Deep Observation Hole Number:

Depth (in.)		/ Soil Matrix: Color- Moist (Munsell)	Redoximorphic Features (mottles)		Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other	
Deptil (III.)	Layer		Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	
0"-6"	A	10YR3/2				Sandy Loam	2%	0%	Granular	Friable	
6"-22"	В	10YR5/8				Sandy Loam	3%	1%	Massive	Friable	
22"-22"	Bedrock	NA				Rock					

Additional Notes:



C. On-Site Review (continued)

	Deep Observat	ion Hole Number: —		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:		Location (identify on	plan): ——		
2.	Land Use	(e.g., woodland, agricultural field	I, vacant lot, etc.)		Surface Stones		Slope (%)
		Vegetation		Landform		Position on Landscape	e (attach sheet)
3.	Distances from:	Open Water Body	feet	 Drainage Way 	feet	Possible Wet Are	ea feet
		Property Line	feet	 Drinking Water \ 	Vell feet	Other	feet
4.	Parent Material:			Unsuital	ole Materials Prese	nt: 🗌 Yes	🗌 No
	If Yes:	Disturbed Soil	II Material	Impervious Layer(s)	U Weather	red/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	🗌 No	If yes:	Depth Weeping fro	m Pit Depth S	Standing Water in Hole
	Estimated Deptl	h to High Groundwater:	inches	elevation			



C. On-Site Review (continued)

Deep Observation Hole Number:

Depth (in.)	Soil Horizon/ Layer	Soil Matrix: Color- Moist (Munsell)	Redoximorphic Features (mottles)		Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other	
Depth (m.)			Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	Culor

Additional Notes:



D. Determination of High Groundwater Elevation

1. Method Used:

	Depth observed standing water in observed	votion holo	Α.		В.	
		alion noie	inches		inches	
	Depth weeping from side of observation	holo	<u>A.</u>		В.	
			inches		inches	
	Depth to soil redoximorphic features (main	ottloc)	<u>A.</u>		В.	
		Jules)	inches		inches	
	Groundwater adjustment (USGS method		Α.		В.	
		lology)	inches		inches	
2.						
	Index Well Number	Reading Date Inc		Index Well Le	dex Well Level	
	Adjustment Factor	Adjusted Groundwater Level				

E. Depth of Pervious Material

- 1. Depth of Naturally Occurring Pervious Material NA
 - a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
 - 🗌 Yes 🗌 No
 - b. If yes, at what depth was it observed? Upper boundary:

undary: inches

Lower boundary:

inches



F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator	Date	
Justin Richardson / SE13688	April 23, 2013	
Typed or Printed Name of Soil Evaluator / License #	Date of Soil Evaluator Exam	

Name of Board of Health Witness

Board of Health

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with <u>Percolation Test Form 12</u>.



Field Diagrams

Use this sheet for field diagrams:

SEE ATTACHED SKETCH



Α.	Facility Information							
	Two Wells Avenue LLC C/O BAL Management LL	C						
	Owner Name							
	2 Wells Avenue			NA				
	Street Address Newton		МА	Map/Lot # 02459				
	City		State	Zip Code				
B.	Site Information							
1.	(Check one) X New Construction	Upgrade	🗌 Repair					
2.	Soil Survey Available? X Yes	□ No	If yes: USGA		103B/656			
	,	_	Rocky Shallow Depth the groundwater Soil Map Unit					
	Charlton-Hollis-Rock / Udorthents-Urban land Con Soil Name	npiex	Soil Limitations					
~		□ No	2006/USGS	1" =250k'				
3.	Surficial Geological Report Available? X Yes Glacio - Fluvial Deposits	If yes: Year Published/Source Glacial Outwash and Drift that in	Publication Scale Map Unit cludes outwash, eskers, kames					
	Geologic/Parent Material		Landform					
4.	Flood Rate Insurance Map							
	Above the 500-year flood boundary? 🛛 Yes	🗌 No	Within the 100-year flood boundar	y? 🗌 Yes	🗙 No			
	Within the 500-year flood boundary? Yes	X No	Within a velocity zone?	Yes	📉 No			
5.	Wetland Area: Wetlands Conservancy Pr	rogram Man	N/A		Map (Charles River			
5.		07/14	Map Unit	Name Easterly V	Vetland Area)			
6.	Current Water Resource Conditions (USGS):	Month/Year	Range: 🗌 Above Normal 🛛	Normal 🗌 Belov	w Normal			
7.	Other references reviewed:							



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserved disposal area*)

	Deep Observat	ion Hole Number:	#12	8/26/2014 Date	8:25 a	am	Sunny (75 de Weather	egrees)	
1.	1. Location Ground Elevation at Surface of Hole:		tbd	Location (identify on plan):		Test	Test Hole #12 (See Attached Plan)		
2.	Land Use	Parking Lot (e.g., woodland, agricultural f	ield, vacant lot, etc.)	Outwash		ne Apparent face Stones	Flat Edge of p	- 3% Slope (%) parking lot	
3.	Distances from:	Vegetation Open Water Body Property Line	1000'* feet 20' feet	Landform Drainage Drinking \	Way Water Well	NA feet NA feet	Position on Lands Possible We Other	et Area $300'$ feet NA feet	
4.	Parent Material:		Fill Material	L		aterials Preser	nt: X Ye	es 🗌 No ck 🔀 Bedrock	
5.	Groundwater Ol Estimated Deptl	oserved: Ves n to High Groundwater:	X No 36" down inches	t	f yes: $\frac{N}{De}$ bd	A epth Weeping fro		IA epth Standing Water in Hole	

* -Charles River



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Test Hole #12

Deep Observation Hole Number:

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Redox	imorphic Fe (mottles)	atures	Soil Texture		Fragments Volume	Soil	Soil Consistence	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	
4"-0"	PAVE	NA				NA					
0-15"	FILL	10YR2/2				Sandy Loam	15%	5%	Massive	Friable	
15"-74"	C1	10YR5/4	36"	Con- 10YR3/6	5%	Loamy Sand*	30%	20%	Massive	Friable	
74"-80"	Cd	10YR5/4	Ļ	Ļ		Sandy Loam	15%	4%	Massive	Friable	

Additional Notes:

*1 Many round stones, very coarse, close to a Sand.



C. On-Site Review (continued)

	Deep Observat	ion Hole Number: —		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:		Location (identify on	plan): ——		
2.	Land Use	(e.g., woodland, agricultural field	I, vacant lot, etc.)		Surface Stones		Slope (%)
		Vegetation		Landform		Position on Landscape	e (attach sheet)
3.	Distances from:	Open Water Body	feet	 Drainage Way 	feet	Possible Wet Are	ea feet
		Property Line	feet	 Drinking Water \ 	Vell feet	Other	feet
4.	Parent Material:			Unsuital	ole Materials Prese	nt: 🗌 Yes	🗌 No
	If Yes:	Disturbed Soil	Il Material	Impervious Layer(s)	U Weather	red/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	🗌 No	If yes:	Depth Weeping fro	m Pit Depth S	Standing Water in Hole
	Estimated Deptl	h to High Groundwater:	inches	elevation			



C. On-Site Review (continued)

Deep Observation Hole Number:

Depth (in.)		Soil Matrix: Color-		(imorphic Fe (mottles)	atures	Soil Texture	0/ 1	ragments /olume	Soil	Soil Consistence	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	

Additional Notes:



D. Determination of High Groundwater Elevation

1. Method Used:

	Depth observed standing water in observed	votion hole	Α.	B	B.
	Depth observed standing water in observ	valion noie	inches	ir	iches
	Depth weeping from side of observation	hole	Α.	B	
			inches	ir	iches
	Depth to soil redoximorphic features (main	ottles)	A. 36"	B	3.
		otties)	inches	ir	iches
	Groundwater adjustment (USGS method	lology)	Α.	<u>B</u>	3
		iology)	inches	ir	iches
2.					
	Index Well Number	Reading Date		Index Well Leve	
	Adjustment Factor	Adjusted Groundwater	Level		

E. Depth of Pervious Material

- 1. Depth of Naturally Occurring Pervious Material N/A
 - a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
 - 🗌 Yes 🗌 No
 - b. If yes, at what depth was it observed? Uppe

Upper boundary: inches

Lower boundary:

inches

* Very slight, bucket teeth were moist.



F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Circulture of Coll Fuckator		
Signature of Soil Evaluator	Date	
Justin Richardson / SE13688	April 23, 2013	
Typed or Printed Name of Soil Evaluator / License #	Date of Soil Evaluator Exam	

Name of Board of Health Witness

Board of Health

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with <u>Percolation Test Form 12</u>.



Field Diagrams

Use this sheet for field diagrams:

SEE ATTACHED SKETCH



Α.	Facility Information					
	Two Wells Avenue LLC C/O BAL Management LL	С				
	Owner Name					
	2 Wells Avenue					
	Street Address Newton		MA		Map/Lot # 02459	
	City		State		Zip Code	
B	Site Information					
1.	(Check one) X New Construction	Upgrade		🗌 Repair		
2.	Soil Survey Available?	□ No	If yes:	USGA		103B/656
۷.	·	—	•	Source	duratan	Soil Map Unit
	Charlton-Hollis-Rock / Udorthents-Urban land Con Soil Name	nplex	Soil Limita	Shallow Depth the ground	uwater	
		_		2006/USGS	1" =250k'	sand and gravel with
3.	Surficial Geological Report Available? X Yes Glacio - Fluvial Deposits	🗌 No	If yes:	Year Published/Source Outwash and Drift that in	Publication Scale	Map Unit
	Geologic/Parent Material		Landform			
4.	Flood Rate Insurance Map					
	Above the 500-year flood boundary? 🛛 Yes	🗌 No	Within th	ne 100-year flood boundar	ry? 🗌 Yes	🗙 No
	Within the 500-year flood boundary? 🔲 Yes	🔀 No		velocity zone?	🗌 Yes	📉 No
F	Wetland Area: Wetlands Conservancy Pr	arom Mon	N/A			Map (Charles River
5.	Wetland Area: Wetlands Conservancy Pr	07/14	Map Unit		Name Easterly W	/etland Area)
6.	Current Water Resource Conditions (USGS):	Month/Year	Range:	Above Normal	Normal 🗌 Belov	w Normal
7.	Other references reviewed:					



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserved disposal area)

	Deep Observat	ion Hole Number:	#13	8/27/2014 Date	9: 	50 am	Sunny (85 degree Weather	es)
1.	Location Ground Elevatio	on at Surface of Hole:	tbd	Location (id	dentify on pla	an):	Hole #13 (See Attac	hed Plan)
2.	Land Use	Parking Lot (e.g., woodland, agricultural to none	ield, vacant lot, etc.)	Outwash		None Apparent Surface Stones		3% Slope (%) rea adjacent to swale
3.	Distances from:	Vegetation Open Water Body Property Line	1000'* feet 30' feet	Landform - Drainag - Drinking	ge Way g Water We	II NA feet NA feet	Position on Landscape Possible Wet Are Other	130'
4.	Parent Material:	Sand, Gravel, Silt	Fill Material			Materials Prese	nt: X Yes	No Bedrock
5.	Groundwater O		No 50" down inches	_ Impervious	If yes: tbd elevation	NA Depth Weeping fro	NA	tanding Water in Hole

* -Charles River



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Test Hole #13

Deep Observation Hole Number:

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Redox	imorphic Fe (mottles)	atures	Soil Texture		Fragments Volume	Soil	Soil Consistence	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	
3"-0"	PAVE	NA				NA					
0-3"	FILL	10YR3/2				Sandy Loam	2%	0%	Massive	Friable	
3"-73"	C1	10YR5/4	50"	Con- 10YR5/8	5%	Loamy Sand*1	30%	10%	Massive	Friable	
73"-113"	C2	10YR6/3	Ļ	Ļ	Ļ		20%	2%	Massive	Friable	

Additional Notes:

*1 Many round stones, very course, close to a Sand.



C. On-Site Review (continued)

	Deep Observat	ion Hole Number: —		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:		Location (identify on	plan): —		
2.	Land Use	(e.g., woodland, agricultural field	I, vacant lot, etc.)		Surface Stones		Slope (%)
		Vegetation		Landform		Position on Landscape	e (attach sheet)
3.	Distances from:	Open Water Body	feet	 Drainage Way 	feet	Possible Wet Are	ea feet
		Property Line	feet	 Drinking Water \ 	Vell feet	Other	feet
4.	Parent Material:			Unsuital	ole Materials Prese	nt: 🗌 Yes	🗌 No
	If Yes:	Disturbed Soil	Il Material	Impervious Layer(s)	U Weather	red/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	🗌 No	If yes:	Depth Weeping fro	m Pit Depth S	Standing Water in Hole
	Estimated Deptl	h to High Groundwater:	inches	elevation			



C. On-Site Review (continued)

Deep Observation Hole Number:

Depth (in.)		Soil Matrix: Color-		(imorphic Fe (mottles)	atures	Soil Texture	0/ 1	ragments /olume	Soil	Soil Consistence	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	

Additional Notes:



D. Determination of High Groundwater Elevation

1. Method Used:

	Dopth observed standing water in observed	vation halo	Α.		В.	
	Depth observed standing water in observed	alion noie	inches		inches	
	Depth weeping from side of observation	hole	<u>A.</u>	_	<u>B.</u>	
			inches		inches	
	X Depth to soil redoximorphic features (m	ottloc)	A. 50"		В.	
	X Depth to soil redoximorphic features (m	ollies)	inches		inches	
	Groundwater adjustment (USGS method		Α.		В.	
		lology)	inches		inches	
2.						
	Index Well Number	Reading Date		Index Well Le	evel	
	Adjustment Factor	Adjusted Groundwater	Level			

E. Depth of Pervious Material

- 1. Depth of Naturally Occurring Pervious Material N/A
 - a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
 - 🗌 Yes 🗌 No
 - b. If yes, at what depth was it observed? Upper boundary:

undary: inches

Lower boundary:

inches



F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator	Date
Justin Richardson / SE13688	April 23, 2013
Typed or Printed Name of Soil Evaluator / License #	Date of Soil Evaluator Exam

Name of Board of Health Witness

Board of Health

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with <u>Percolation Test Form 12</u>.



Field Diagrams

Use this sheet for field diagrams:

SEE ATTACHED SKETCH



Α.	Facility Information					
	Two Wells Avenue LLC C/O BAL Management LL	С				
	Owner Name					
	2 Wells Avenue					
	Street Address Newton		MA		Map/Lot # 02459	
	City		State		Zip Code	
B	Site Information					
1.	(Check one) X New Construction	Upgrade		🗌 Repair		
2.	Soil Survey Available?	□ No	If yes:	USGA		103B/656
۷.	·	—	•	Source	duratan	Soil Map Unit
	Charlton-Hollis-Rock / Udorthents-Urban land Con Soil Name	nplex	Soil Limita	Shallow Depth the ground	uwater	
		_		2006/USGS	1" =250k'	sand and gravel with
3.	Surficial Geological Report Available? X Yes Glacio - Fluvial Deposits	🗌 No	If yes:	Year Published/Source Outwash and Drift that in	Publication Scale	Map Unit
	Geologic/Parent Material		Landform			
4.	Flood Rate Insurance Map					
	Above the 500-year flood boundary? 🛛 Yes	🗌 No	Within th	ne 100-year flood boundar	ry? 🗌 Yes	🗙 No
	Within the 500-year flood boundary? 🔲 Yes	🔀 No		velocity zone?	🗌 Yes	📉 No
F	Wetland Area: Wetlands Conservancy Pr	arom Mon	N/A			Map (Charles River
5.	Wetland Area: Wetlands Conservancy Pr	07/14	Map Unit		Name Easterly W	/etland Area)
6.	Current Water Resource Conditions (USGS):	Month/Year	Range:	Above Normal	Normal 🗌 Belov	w Normal
7.	Other references reviewed:					



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserved disposal area*)

	Deep Observat	ion Hole Number:	#14	8/27/2014 Date	10:10 am Time	Sunny (85 degre Weather	ees)
1.	Location Ground Elevation	on at Surface of Hole:	tbd	Location (identify	v on plan):	Test Hole #14 (See Atta	ched Plan)
2.	Land Use	Grassed area (e.g., woodland, agricultural f Grassed area	ield, vacant lot, etc.)	Outwash	None Appa Surface Store		3% Slope (%) kt to building.
3.	Distances from:	Vegetation Open Water Body Property Line	1000'* feet 75' feet	Landform Drainage Wa Drinking Wate	NA	t Possible Wet Ar	150'
4.	Parent Material:	Sand, Gravel, Silt	Fill Material	Unsu	uitable Materials I r(s)	Present: X Yes	No Bedrock
5.	Groundwater Ol Estimated Deptl	bserved: Yes h to High Groundwater:	X No 46" inches	If yes tbd elevat		ping from Pit Depth 3	Standing Water in Hole

* -Charles River



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Test Hole #14

Deep Observation Hole Number:

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Redox	timorphic Fe (mottles)	eatures	Soil Texture	Coarse F % by	Fragments Volume	Soil	Soil Consistence	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	Other
0-3"	А	10YR3/2				Sandy Loam	2%	0%	Granular	Friable	
3"-12"	В	10YR6/6				Sandy Loam	3%	1%	Massive	Friable	
12"-99"	CD	10YR6/2	46"	Con- 10YR5/6	5%	Sandy Loam*	4%	2%	Massive	Friable	

Additional Notes:

* - Very Tightly packed and stony



C. On-Site Review (continued)

	Deep Observat	ion Hole Number: —		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:		Location (identify on	plan): ——		
2.	Land Use	(e.g., woodland, agricultural field	I, vacant lot, etc.)		Surface Stones		Slope (%)
		Vegetation		Landform		Position on Landscape	e (attach sheet)
3.	Distances from:	Open Water Body	feet	 Drainage Way 	feet	Possible Wet Are	ea feet
		Property Line	feet	 Drinking Water \ 	Vell feet	Other	feet
4.	Parent Material:			Unsuital	ole Materials Prese	nt: 🗌 Yes	🗌 No
	If Yes:	Disturbed Soil	Il Material	Impervious Layer(s)	U Weather	red/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	🗌 No	If yes:	Depth Weeping fro	m Pit Depth S	Standing Water in Hole
	Estimated Deptl	h to High Groundwater:	inches	elevation			



C. On-Site Review (continued)

Deep Observation Hole Number:

Depth (in.)		Soil Matrix: Color-		(imorphic Fe (mottles)	atures	Soil Texture	0/ 1	ragments /olume	Soil	Soil Consistence	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	Other

Additional Notes:



D. Determination of High Groundwater Elevation

1. Method Used:

	Depth observed standing water in observed	votion hole	Α.		B.	
	Depth observed standing water in observ	vation note	inches		inches	
	Depth weeping from side of observation	hole	Α.		В.	
			inches		inches	
	Depth to soil redoximorphic features (main	ottles)	A. 46"		В.	
		otties)	inches		inches	
	Groundwater adjustment (USGS method	lology)	Α.		В.	
		lology)	inches		inches	
2.						
	Index Well Number	Reading Date		Index Well L	evel	
	Adjustment Factor	Adjusted Groundwater	Level			

E. Depth of Pervious Material

- 1. Depth of Naturally Occurring Pervious Material N/A
 - a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
 - 🗌 Yes 🗌 No
 - b. If yes, at what depth was it observed? Upper boundary:

undary: inches

Lower boundary:



F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator	Date
Justin Richardson / SE13688	April 23, 2013
Typed or Printed Name of Soil Evaluator / License #	Date of Soil Evaluator Exam

Name of Board of Health Witness

Board of Health

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with <u>Percolation Test Form 12</u>.



Field Diagrams

Use this sheet for field diagrams:

SEE ATTACHED SKETCH



A.	Facility Information				
	Two Wells Avenue LLC C/O BAL Management LL	C			
	Owner Name				
	2 Wells Avenue				
	Street Address Newton		МА	Map/Lot # 02459	
	City		State	Zip Code	
B	Site Information				
1.	(Check one) X New Construction	Upgrade	🗌 Repair		
0			USGA		103B/656
2.	Soil Survey Available? X Yes	🗌 No	If yes: Source		Soil Map Unit
	Charlton-Hollis-Rock / Udorthents-Urban land Con	nplex	Rocky Shallow Depth the ground	dwater	
	Soil Name		Soil Limitations 2006/USGS	1" =250k'	
3.	Surficial Geological Report Available? 🔀 Yes	🗌 No	If yes: Year Published/Source	Publication Scale	Map Unit
	Glacio - Fluvial Deposits		Glacial Outwash and Drift that in	cludes outwash, e	
	Geologic/Parent Material		Landform		
4.	Flood Rate Insurance Map				
	Above the 500-year flood boundary? 🛛 Yes	🗌 No	Within the 100-year flood boundar	y? 🗌 Yes	🗙 No
	Within the 500-year flood boundary? 🔲 Yes	🗙 No	Within a velocity zone?	🗌 Yes	📉 No
_			N/A	DEP 12k l	Map (Charles River
5.	Wetland Area: Wetlands Conservancy P	rogram Map 07/14	Map Unit	Name Easterly V	Vetland Area)
6.	Current Water Resource Conditions (USGS):	Month/Year	Range: 🗌 Above Normal 🛛	Normal 🗌 Belo	w Normal
7.	Other references reviewed:				



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserved disposal area*)

	Deep Observat	ion Hole Number:	#15	8/27/2014 Date	11:30 am Time	Sunny (80 degro Weather	ees)
1.	Location Ground Elevation	on at Surface of Hole:	tbd	Location (identify d	on plan): —	Test Hole #15 (See Atta	ached Plan)
2.	Land Use	Grassed area (e.g., woodland, agricultural f Grass	ield, vacant lot, etc.)	Outwash	Some along		10% Slope (%) adjacent to driveway
3.	Distances from:	Vegetation Open Water Body Property Line	1000'* feet 50' feet	Landform Drainage Way Drinking Water	NA	Position on Landscap Possible Wet A Other	e (attach sheet) 130'
4.	Parent Material:	Sand, Gravel, Silt	Fill Material	Unsui	table Materials P	resent: X Yes	No Bedrock
5.	Groundwater O		No 72" down inches	If yes: 	NA Depth Weepi	NA	Standing Water in Hole

* -Charles River



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Test Hole #15

Deep Observation Hole Number:

Depth (in.)	Soil Horizon/	Soil Matrix: Color-	Redox	imorphic Fe (mottles)	atures	Soil Texture		Fragments Volume	Soil	Soil Consistence	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	Other
0"-6"	AO	10YR3/1				Sandy Loam	5%	3%	Granular	Friable	
6"-15"	Afill	10YR3/6				Sandy Loam	5%	3%	Granular	Friable	
15"-41"	FILL	10YR7/4				Loamy Sand	2%	1%	Massive	Friable	
41"-79"	Organic Peat	10YR2/2	72"	Con- 10YR5/8	5%	Sandy Loam*	3%	1%	Massive	Friable	
79"-116"	Cd	10YR6/1	Ļ	Ļ	Ļ	Sandy Loam	5%	5%	Massive	Friable	

Additional Notes:

* -A lot of organics including tree stumps oder was apparent



C. On-Site Review (continued)

	Deep Observat	ion Hole Number: —		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:		Location (identify on	plan): ——		
2.	Land Use	(e.g., woodland, agricultural field	I, vacant lot, etc.)		Surface Stones		Slope (%)
		Vegetation		Landform		Position on Landscape	e (attach sheet)
3.	Distances from:	Open Water Body	feet	 Drainage Way 	feet	Possible Wet Are	ea feet
		Property Line	feet	 Drinking Water \ 	Vell feet	Other	feet
4.	Parent Material:			Unsuital	ole Materials Prese	nt: 🗌 Yes	🗌 No
	If Yes:	Disturbed Soil	Il Material	Impervious Layer(s)	U Weather	red/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	🗌 No	If yes:	Depth Weeping fro	m Pit Depth S	Standing Water in Hole
	Estimated Deptl	h to High Groundwater:	inches	elevation			



C. On-Site Review (continued)

Deep Observation Hole Number:

Depth (in.)		Soil Matrix: Color-		Redoximorphic Features (mottles)		Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other
Depth (m.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	Other

Additional Notes:



D. Determination of High Groundwater Elevation

1. Method Used:

	Depth observed standing water in observation hole		Α.		B.	
		inches		inches		
	Depth weeping from side of observation	Α.		В.		
			inches		inches	
	Depth to soil redoximorphic features (mottles)		A. 72"*		В.	
			inches		inches	
	Groundwater adjustment (USGS method		Α.		В.	
			inches		inches	
2.						
	Index Well Number	Reading Date		Index Well Le	vel	
	Adjustment Factor Adjusted Groundwate		r Level			

E. Depth of Pervious Material

- 1. Depth of Naturally Occurring Pervious Material N/A
 - a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
 - 🗌 Yes 🗌 No
 - b. If yes, at what depth was it observed?

Upper boundary: inches

Lower boundary:

inches

* modeling was difficult to see because not as developed in the fill layer.



F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

 Signature of Soil Evaluator
 Date

 Justin Richardson / SE13688
 April 23, 2013

 Typed or Printed Name of Soil Evaluator / License #
 Date of Soil Evaluator Exam

Name of Board of Health Witness

Board of Health

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with <u>Percolation Test Form 12</u>.



Field Diagrams

Use this sheet for field diagrams:

SEE ATTACHED SKETCH



Α.	Facility Information						
	Two Wells Avenue LLC C/O BAL Management LLC	С					
	Owner Name						
	2 Wells Avenue Street Address				Marchatt		
	Newton		MA		Map/Lot # 02459		
	City		State		Zip Code		
B.	Site Information						
1.	(Check one) X New Construction	Upgrade	🗌 Rep	pair			
2.	Soil Survey Available?	□ No	If yes:			103B/656	
۷.	,		Source Source Source				
	Charlton-Hollis-Rock / Udorthents-Urban land Com Soil Name	Rocky Shallow Depth the groundwater Soil Limitations					
		□ No	2006	/USGS	1" =250k'		
3.	Surficial Geological Report Available? X Yes Glacio - Fluvial Deposits		Published/Source	Publication Scale Map Unit cludes outwash, eskers, kames			
	Geologic/Parent Material	Landform					
4.	Flood Rate Insurance Map						
	Above the 500-year flood boundary? Xes	🗌 No	Within the 100-y	ear flood boundary	y? 🗌 Yes	🔀 No	
	Within the 500-year flood boundary?	🗙 No	Within a velocity	zone?	Yes	📉 No	
5.	Wetland Area: Wetlands Conservancy Pr	oaram Man	N/A			Map (Charles River	
5.	Weiland Alea. Weilands Conservancy I I	07/14	Map Unit		Name Easterly V	/etland Area)	
6.	Current Water Resource Conditions (USGS):	Month/Year	Range: 🗌 Abo	ove Normal 🛛 🛛	Normal 🗌 Belov	w Normal	
7.	Other references reviewed:						



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserved disposal area*)

	Deep Observat	ion Hole Number:	#16	8/27/2014 Date	1:30 pm Time	Sunny (85 degre Weather	es)
1.	Location Ground Elevation	on at Surface of Hole:	tbd	Location (ident	ify on plan):	Test Hole #16 (See Atta	ched Plan)
2.	Land Use	Grassed area (e.g., woodland, agricultural f Grass	ield, vacant lot, etc.)	Outwash	None Clos Surface Stor		10% Slope (%)
3.	Distances from:	Vegetation Open Water Body Property Line	1000'* feet 50' feet	Landform Drainage W Drinking W	vay fee N	A Other	300'
4.	Parent Material:		Fill Material	Ur	nsuitable Materials yer(s)	Present: X Yes	No Bedrock
5.	Groundwater Ol Estimated Deptl	oserved: 🔀 Yes n to High Groundwater:	No 85" down inches	tbo	· ·	eping from Pit Depth S	Standing Water in Hole

* -Charles River



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (continued)

Test Hole #16

Deep Observation Hole Number:

	Depth (in.)	Soil Horizon/	Soil Matrix: Color- Moist (Munsell)	Redoximorphic Features (mottles)		Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other	
	Depth (m.)	Layer		Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	other
	0"-8"	А	10YR3/2				Sandy Loam	5%	3%	Granular	Friable	
	8"-23"	Fill	10YR6/3				Loamy Sand	15%	5%	Massive	Friable	
1	23"-63"	Fill Peat	10YR3/3				Sandy Loam	2%	2%	Massive	Friable	
	63"-109"	Fill	10YR5/2	85"	Con- 10YR5/8	5%	Loamy Sand	20%	5%	Massive	Friable	
	109"-115"	Buried A	10YR3/3	Ļ	Ļ	↓	Sandy Loam	3%	3%	Massive	Friable	
	115"-121"	Buried B	10YR4/6	Ļ	Ļ	↓	Sandy Loam	5%	2%	Massive	Friable	
	121"-133"	С	10YR5/2	Ļ	Ļ	↓	Loamy Sand	20%	5%	Massive	Friable	

Additional Notes:

* -A lot of organics mixed in with the fill.

*1 -material was a mix, tree stumps apparent, old trees



C. On-Site Review (continued)

	Deep Observat	ion Hole Number: —		Date	Time	Weather	
1.	Location						
	Ground Elevation	on at Surface of Hole:		Location (identify on	plan): ——		
2.	Land Use	(e.g., woodland, agricultural field	I, vacant lot, etc.)		Surface Stones		Slope (%)
		Vegetation		Landform		Position on Landscape	e (attach sheet)
3.	Distances from:	Open Water Body	feet	 Drainage Way 	feet	Possible Wet Are	ea feet
		Property Line	feet	 Drinking Water \ 	Vell feet	Other	feet
4.	Parent Material:			Unsuital	ole Materials Prese	nt: 🗌 Yes	🗌 No
	If Yes:	Disturbed Soil	II Material	Impervious Layer(s)	U Weather	red/Fractured Rock	Bedrock
5.	Groundwater Ol	bserved: 🗌 Yes	🗌 No	If yes:	Depth Weeping fro	m Pit Depth S	Standing Water in Hole
	Estimated Deptl	h to High Groundwater:	inches	elevation			



C. On-Site Review (continued)

Deep Observation Hole Number:

Depth (in.)	Soil Horizon/ Layer	n/ Soil Matrix: Color- Moist (Munsell)	Redoximorphic Features (mottles)		Soil Texture	Coarse Fragments % by Volume		Soil	Soil Consistence	Other	
Depth (m.)			Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	other

Additional Notes:



D. Determination of High Groundwater Elevation

1. Method Used:

	Dopth obsorved standing water in obsorv	vation halo	<u>A.</u>		B.	
	Depth observed standing water in observation hole		inches		inches	
	Depth weeping from side of observation	Α.		В.		
			inches		inches	
	Depth to soil redoximorphic features (mottles)		A. 85"*		В.	
			inches		inches	
	Groundwater adjustment (USGS method		Α.		B.	
		ology)	inches		inches	
2.						
	Index Well Number	Reading Date		Index Well Lev	/el	
	Adjustment Factor Adjusted Groundwate		Level			

E. Depth of Pervious Material

- 1. Depth of Naturally Occurring Pervious Material N/A
 - a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
 - 🗌 Yes 🗌 No
 - b. If yes, at what depth was it observed?

Upper boundary: inches

Lower boundary:

inches

* modeling was difficult to see because not as developed in the fill layer.



F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator	Date	
Justin Richardson / SE13688	April 23, 2013	
Typed or Printed Name of Soil Evaluator / License #	Date of Soil Evaluator Exam	

Name of Board of Health Witness

Board of Health

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Field Diagrams

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SEE ATTACHED SKETCH