

To: Mr. Giancarlo Micozzi Micozzi Management, Inc. 159 Cambridge Street Arlington, MA 02134 Date: October 10, 2017

Memorandum

Project #: 14019.00

From: Randall C. Hart, Principal

Kathleen Keen, EIT

Re: Proposed Langley Road Redevelopment

Newton, Massachusetts

Introduction

VHB, Inc. has conducted a traffic impact and access study to assess the potential traffic impacts associated with the proposed redevelopment located at 392-404 Langley Road in Newton, Massachusetts. The proposed redevelopment Project will involve the demolition of one existing building and the construction of an approximately 20-unit residential building, supported by sub-surface parking.

This memorandum includes an evaluation of the existing traffic operations and safety; assessment of future conditions without the Project; an estimate of projected traffic volumes for the Project; and its potential impact on future traffic operations in the area. As detailed herein, the proposed Project is expected to have a minor impact on local traffic operations.

Site Location and Proposed Development

The Project site is located at 392-404 Langley Road in Newton, Massachusetts. The site currently consists of four buildings accessed by three full-access driveways along Langley Road. The proposed redevelopment Project will involve the demolition of one existing building, located at 400 Langley Road, and the construction of an approximately 20-unit residential building with sub-surface parking, that will replace one existing building on-Site. The three other existing buildings will remain. As part of the Project, the existing full-access driveway to 400 Langley Road will be closed, and the two other existing driveways will remain. A conceptual site plan is included in the Attachments.

Existing Conditions

The following sections provide a description of the existing roadway network, roadway/intersection geometry, traffic control, existing daily and peak hour traffic volumes, and traffic safety conditions.

Study Area

The Project Site is located along Langley Road, which is described below.

Langley Road is a north-south urban collector under City of Newton jurisdiction in the vicinity of the Site.

Langley Road is a two-lane, undivided roadway with a posted speed limit of 25 miles per hour (mph). There are sidewalks along both sides of the roadway. Within the vicinity of the Site, on-street parking is allowed along the east side of Langley Road and no parking is allowed along the west side of Langley Road. The on-street parking along the east side of Langley Road is limited to two-hour parking from 7:00 AM to 7:00 PM, except weekends

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and holidays, south of the 392-396 Langley Road driveway, and to 15-minute parking from 6:00 AM to 9:00 PM north of the 392-396 Langley Road driveway. Land use consists of a mix of residential and commercial uses within the vicinity of the Site.

For the purposes of evaluating existing and future traffic conditions in the vicinity of the Site, a Project study area has been established and includes the following five intersections.

- Langley Road at John Street
- Langley Road at 392-396 Langley Road driveway
- Langley Road at 400 Langley Road driveway
- Langley Road at Jackson Street
- Langley Road at Route 9 (Boylston Street)

A figure showing the intersection lane geometry and traffic control at each study area intersection is included in the Attachments.

Traffic Volumes

To assess the existing operational conditions at study area intersections, automatic traffic recorder (ATR) counts were conducted from Tuesday, September 19, 2017 through Wednesday, September 20, 2017 along Langley Road in the vicinity of the Site. The average weekday traffic volume data are summarized below in Table 1 and the existing count data is included in the Attachments.

Table 1 Existing Traffic Volume Summary

	Weekday Daily	Weekday	Morning Peak	Hour	Weekday	Evening Peak	Hour
Location	Vol (vpd) ^a	Vol (vph) ^b	K Factor ^c	Dir. Dist.	Vol (vph)	K Factor	Dir. Dist.
Langley Road south of John Street	6,200	505	8.1%	53% SB	500	8.0%	58% SB

Source Automatic Traffic Recorder (ATR) counts conducted by VHB in September 2017.

- Daily traffic expressed in vehicles per day.
- b Peak hour volumes expressed in vehicles per hour.
- c Percent of daily traffic, which occurs during the peak hour.

As shown in Table 1, Langley Road carries approximately 6,200 vehicles per day on a typical weekday, with 8.1-percent during the morning peak hour and 8.0-percent during the evening peak hour. Langley Road traffic is slightly heavier in the southbound direction during both peak hours.

In addition, peak hour turning movement counts (TMCs) were conducted concurrent with the ATR counts at the study area intersections in September 2017 during the weekday morning peak period from 7:00 AM to 9:00 AM and weekday evening peak period from 4:00 PM to 6:00 PM. Based on a review of the count data, the weekday morning

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and weekday evening peak hours of vehicular activity were determined to be 8:00 AM to 9:00 AM and 5:00 PM to 6:00 PM, respectively. The traffic volume counts are provided in the Attachments.

Seasonal Variation

The peak hour traffic data collected for the Project was obtained during the month of September. To quantify the seasonal variation of traffic volumes in the area, historic traffic data available from MassDOT were reviewed. According to published MassDOT weekday seasonal factors, September traffic counts are generally higher than average month conditions. To present a conservative analysis, the traffic volumes were not reduced to reflect average month conditions. The 2017 Existing peak hour traffic volume networks are provided in the Attachments.

Crash Summary

To identify potential vehicle crash trends in the study area, vehicular crash data for the study area intersections were obtained from Massachusetts Department of Transportation (MassDOT) for the most recent five-year period available, 2010 through 2014. A summary of the MassDOT vehicular crash history is provided in Table 2 and the detailed crash data is provided in the Attachments.

The current MassDOT average crash rates for signalized and unsignalized intersections in District 6 (the MassDOT district for Newton) are 0.70 crashes per million entering vehicles and 0.53 crashes per million entering vehicles, respectively. In other words, on average, 0.70 crashes occurred per million vehicles entering signalized intersections, and 0.53 crashes occurred per million vehicles entering unsignalized intersections throughout District 6. The crash rate worksheets are included in the Attachments.

As shown in Table 2, none of the study area intersections had calculated crash rates above the MassDOT District 6 average crash rates. The majority of crashes that occurred at the study area intersections were rear-end collisions resulting in property damage only. None of the crashes resulted in fatal injuries. Crashes involving non-motorists (bike, pedestrian) occurred at the intersections of Langley Road at Jackson Street (one crash) and Langley Road at Route 9 (one crash).

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Table 2 Vehicular Crash Data (2010 - 2014)

	Langley Road at John Street	Langley Road at 392-396 Langley Road	Langley Road at 400 Langley Road	Langley Road at Jackson Street	Langley Road at Route 9
Signalized?	No	No	No	No	Yes
MassDOT Average Crash Rate	0.53	0.53	0.53	0.53	0.70
Calculated Crash Rate	0.10	0.20	0.00	0.37	0.37
Exceeds Average Crash Rate?	No	No	No	No	No
Year					
2010	1	0	0	1	6
2011	0	0	0	1	1
2012	0	1	0	1	4
2013	0	1	0	1	8
<u>2014</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>1</u>	<u>6</u>
Total	1	2	0	5	25
Average	0.20	0.40	0.00	1.00	5.00
Collision Type					
Angle	0	1	0	1	2
Rear-end	1	0	0	1	15
Sideswipe, same direction	0	0	0	2	7
Single vehicle crash	0	1	0	1	1
Crash Severity					
Fatal injury	0	0	0	0	0
Non-fatal injury	0	0	0	1	6
Property damage only (none injured)	0	2	0	2	19
Not Reported	1	0	0	2	0
Time of Day					
Weekday, 7:00 AM - 9:00 AM	0	0	0	1	1
Weekday, 4:00 PM - 6:00 PM	0	0	0	0	3
Saturday, 11:00 AM - 2:00 PM	0	1	0	0	1
Weekday, other time	0	1	0	4	18
Weekend, other time	1	0	0	0	2
Pavement Conditions					
Dry	1	2	0	4	20
Wet	0	0	0	1	5
Ice	0	0	0	1	0
Non-Motorist (Bike, Pedestrian)	0	0	0	1	1

Source: MassDOT vehicle crash data, accessed October 2017.

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Sight Distance

A sight distance analysis, in conformance with guidelines of the American Association of State Highway and Transportation Officials (AASHTO) was performed at the unsignalized intersection of Langley Road at the 392-396 Langley Road driveway. Sight distance considerations are generally divided into two categories: Stopping Sight Distance (SSD) and Intersection Sight Distance (ISD). Stopping Sight Distance (SSD) is the distance required for a vehicle approaching an intersection from either direction to perceive, react and come to a complete stop before colliding with an object in the road, in this case the exiting vehicle from a driveway. In this respect, SSD can be considered as the minimum visibility criterion for the safe operation of an unsignalized intersection.

Intersection Sight Distance (ISD) is based on the time required for perception, reaction and completion of the desired critical exiting maneuver once the driver on a minor street or driveway approach decided to execute the maneuver. Calculation for the critical ISD includes the time to (1) turn left, and to clear the half of the intersection without conflicting with the vehicles approaching from the left; and (2) accelerate to the operating speed of the roadway without causing approaching vehicles to unduly reduce their speed. In this context, ISD can be considered as a desirable visibility criterion for the safe operation of an unsignalized intersection. Essentially, while SSD is the minimum distance needed to avoid collisions, ISD is the minimum distance needed so that mainline motorists will not have to substantially reduce their speed due to turning vehicles. To maintain the safe operation of an unsignalized intersection, ISD only needs to be equal to the stopping sight distance, though it is desirable to meet ISD requirements by themselves.

To calculate the required SSD and ISD at the unsignalized intersection of Langley Road and 392-396 Langley Road driveway the 85th percentile speed measured by the ATR count described above was utilized. The 85th percentile speed along Langley Road was observed to be 29 mph northbound and 31 mph southbound. The posted speed limit along Langley Road is 25 mph in both the directions. Table 3 summarizes the sight distance analysis and the sight distance worksheets are included in the Attachments.

Table 3 Sight Distance Analysis Summary

	Stopping Sig	ht Distance (f	ft) ^a	Intersection	Sight Distand	ce (ft) ^a
Location	Traveling	Required	Measured	Looking	Desired	Measured
Langley Road at 392-396	Northbound	190	280	Left (south)	345	215
Langley Road Driveway	Southbound	210	270	Right (north)	345	475

Based on guidelines established in A Policy on the Geometric Design of Highways and Streets, Sixth Edition, American Association of State Highway and Transportation Officials (AASHTO), 2011 for an 85th percentile speed of 29 mph northbound and 31 mph southbound.

As shown in Table 3, at the unsignalized intersection of Langley Road at the 392-396 Langley Road driveway the required stopping sight distance is exceeded in both directions. The desired intersection sight distance is exceeded when looking right (north). The desired intersection sight distance is not met when looking left (south), however the measured intersection sight distance does exceed the required stopping sight distance, which is considered the minimum intersection sight distance.

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Future Conditions

To determine the impacts of the site-generated traffic volumes in the vicinity of the site, future traffic conditions were evaluated. A seven-year horizon (2024) was used for the evaluation consistent with MassDOT TIA requirements.

Traffic growth on area roadways is a function of the expected land development, environmental activity, and changes in demographics. A frequently used procedure is to identify estimated traffic generated by planned developments that would be expected to affect the project study area roadways. An alternative procedure is to estimate an annual percentage increase and apply that increase to study area traffic volumes. For this evaluation, <u>both</u> procedures were used. The following summarizes this traffic forecasting process.

Historic Growth

Based on a review of recent studies in the vicinity of the Site and discussions with the City of Newton planning department, a growth rate of one-percent per year was determined to be appropriate for the study.

Planned Developments

In addition to accounting for background growth, the traffic associated with other planned and/or approved developments near the Site was considered. Based on discussions with the City of Newton, it was determined that there is one in the vicinity of the site that are likely to influence traffic conditions.

• **Chestnut Hill Square:** The project is located at 200 Boylston Street (Route 9) to the east of Langley Road. Phase 2 of the project includes approximately 91 residential units and is expected to begin construction in 2018.

Background Transportation Projects

In assessing future traffic conditions, proposed roadway improvements within the study area were considered. Based on discussions with the City of Newton, there are no transportation projects that would impact the Project study area within the seven-year horizon.

No-Build Traffic Volumes

The 2024 No-Build traffic volumes were generated by consideration of the above described factors. The resulting 2024 No-Build peak hour traffic volume networks are provided in the Attachments.

Trip Generation

The proposed redevelopment Project will involve the construction of an approximately 20-unit residential building that will replace an existing building on-Site. To estimate the site-generated traffic, the Institute of Transportation Engineers' (ITE) publication *Trip Generation*, 10th Edition¹ was utilized. The number of vehicle trips generated by the proposed project were estimated based on ITE land use code (LUC) 221 (Mid-Rise Residential). Table 4 provides a trip generation summary and the worksheet is included in the Attachments.

¹ Trip Generation Manual, 10th Edition, Institute of Transportation Engineers, Washington D.C., 2017.

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Table 4 Trip Generation Summary

Time Period	Direction	New Residential Trips ^a
Weekday Daily	Enter	54
	<u>Exit</u>	<u>54</u>
	Total	108
Weekday Morning	Enter	2
	<u>Exit</u>	<u>5</u>
	Total	7
Weekday Evening	Enter	6
	<u>Exit</u>	<u>4</u>
	Total	10

Trip generation estimate based on ITE LUC 221 (Mid-Rise Residential) for 20 units

As shown in Table 4, the proposed Project is expected to increase vehicle trips to the site by approximately 7 (2 entering/5 exiting) vehicle trips during the weekday morning peak hour and approximately 10 (6 entering/4 exiting) vehicle trips during the weekday evening peak hour.

Trip Distribution

The directional distribution of the traffic approaching and departing the Site is a function of population densities, the location of employment opportunities, existing travel patterns, and the efficiency of the roadway system. Trips made from and to the Site during the peak hours are expected to be predominantly home-to-work and work-to-home trips in the weekday morning and weekday evening peak hours, respectively. Accordingly, the trip distribution for the proposed Project has been derived based on 2010 U.S. Census data. Table 5 summarizes the trip distribution. A figure and detailed trip distribution calculations are provided in the Attachments.

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Table 5 Trip Distribution

Travel Route	Direction (from/to)	Percent Site Traffic
Langley Road	north	49%
Route 9	west	29%
Route 9	<u>east</u>	<u>22%</u>
Total		100%

Build Traffic Volumes

The project-related traffic volumes shown in Table 4 are assigned to the study area roadway network based on the trip distribution patterns shown in Table 5 and added to the 2024 No-Build peak hour traffic volume networks to develop the 2024 Build peak hour traffic volume networks. The 2024 Build peak hour traffic volume networks and the Site-generated traffic volume networks are provided in the Attachments.

Traffic Operations Analysis

To assess quality of flow, intersection capacity analyses were conducted with respect to 2017 Existing, 2024 No-Build, and 2024 Build traffic volume conditions. Capacity analyses provide an indication of how well the roadway facilities serve the traffic demands placed upon them. Roadway operating conditions are classified by calculated levels-of-service.

The evaluation criteria used to analyze the signalized study area intersection in this traffic study is based on the percentile-delay method (SYNCHRO results). The evaluation criteria used to analyze the unsignalized study area intersections is based on the *2010 Highway Capacity Manual* (HCM)². Level–of-service (LOS) is the term used to denote the different operating conditions that occur on a given roadway segment under various traffic volume loads. It is a qualitative measure that considers a number of factors including roadway geometry, speed, travel delay, freedom to maneuver, and safety. Level-of-service provides an index to operational qualities of a roadway segment or an intersection. Level-of-service designations range from A to F, with LOS A representing the best operating conditions and LOS F representing the worst operating conditions.

Intersection Capacity Analysis

Levels-of-service analyses were conducted for the 2017 Existing, 2024 No-Build, and 2024 Build conditions for the study area intersections. Tables 6 and 7 summarize the capacity analysis results for the signalized and unsignalized study area intersections, respectively. The capacity analyses worksheets are included in the Attachments.

As shown in Tables 6 and 7, the Project is expected to have minimal impacts on traffic operations at the study area intersections. No changes to overall or individual movement level-of-service are expected between 2024 No-Build and 2024 Build conditions during both peak hours.

² Highway Capacity Manual, Transportation Research Board, Washington D.C., 2010.

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Table 6 Signalized Intersection Capacity Analysis

Location /		2017 Ex	cisting C	Condition	ıs	2	024 No	-Build	Condition	ons		2024 I	Build Co	ondition	iS
Movement	v/c ^a	Del ^b	LOS °	50 Q ^d	95 Q ^e	v/c	Del	LOS	50 Q	95 Q	v/c	Del	LOS	50 Q	95 Q
Langley Road at R	oute 9 (I	Boylston	Street)												
Weekday Evening															
EB T/R	0.59	12	В	79	#330	0.67	13	В	94	#400	0.67	13	В	94	#401
WB T	0.62	12	В	84	#357	0.72	14	В	105	#447	0.72	14	В	105	#448
SB L	0.44	23	C	34	105	0.45	23	C	35	113	0.45	23	C	35	115
SB L/T	0.44	23	C	34	105	0.45	23	C	35	113	0.45	23	C	35	114
SB R	0.24	7	Α	0	30	0.23	7	Α	0	34	0.23	7	Α	0	33
Overall		13	В				14	В				14	В		
Saturday Midday															
EB T/R	0.48	10	В	58	231	0.56	12	В	73	282	0.56	12	В	73	282
WB T	0.63	13	В	86	#372	0.73	15	В	110	#475	0.73	15	В	110	#475
SB L	0.44	23	С	34	112	0.45	23	С	37	117	0.45	23	С	37	117
SB L/T	0.44	23	С	35	113	0.46	23	С	37	120	0.46	23	С	37	120
SB R	0.29	7	Α	0	38	0.30	7	Α	0	39	0.30	7	Α	0	39
Overall		12	В				14	В				14	В		

a Volume to capacity ratio.

b Average total delay, in seconds per vehicle.

c Level-of-service.

d 50th percentile queue, in feet.

e 95th percentile queue, in feet.

^{# 95}th percentile volume exceeds capacity, queue may be longer.

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Table 7 **Unsignalized Intersection Capacity Analysis**

Location /		2017 E	xisting C	Condition	S		2024 No	-Build C	ondition	ıs		2024	Build Co	nditions	
Movement	D a	v/c ^b	Del ^c	LOS d	95 Q ^e	D	v/c	Del	LOS	95 Q	D	v/c	Del	LOS	95 Q
Langley Road at Jo	hn Stree	et													
Weekday Evening															
WB L/R	5	0.02	13	В	3	5	0.01	13	В	0	5	0.01	13	В	0
SB L	neg	0.00	8	Α	0	neg	0.00	8	Α	0	neg	0.00	8	Α	0
Saturday Midday															
WB L/R	5	0.03	12	В	3	5	0.01	12	В	0	5	0.01	12	В	0
SB L	neg	0.00	8	Α	0	neg	0.00	8	Α	0	neg	0.00	8	Α	0
Langley Road at 39	92-396 L	anglev R	oad Dri	vewav											
Weekday Evening															
WB L/R	5	0.03	13	В	3	5	0.01	13	В	0	10	0.03	13	В	3
SB L	neg	0.00	8	A	0	neg	0.00	8	A	0	neg	0.00	8	A	0
Saturday Midday															
WB L/R	10	0.03	11	В	3	10	0.02	11	В	3	10	0.02	11	В	3
SB L	neg	0.00	8	Α	0	neg	0.00	8	Α	0	5	0.00	8	Α	0
Langley Road at 40	00 Langle	ev Road	Drivewa	av.											
Weekday Evening		.,		-,											
WB L/R	neg	0.01	12	В	0	neg	0.01	12	В	0					
SB L	neg	0.00	8	A	0	neg	0.00	8	A	0	-				
Saturday Midday	9	0.00					0.00				Interse	ction doe			2024 Build
WB L/R	neg	0.01	9	Α	0	neg	0.00	10	Α	0			conditio	ns	
SB L	neg	-	0	A	0	neg	-	0	A	0	_				
	9														
Langley Road at Ja	ckson St	reet													
Weekday Evening															
EB L/R	90	0.20	14	В	18	100	0.23	15	В	23	100	0.23	15	В	23
WB L/T/R	260	0.32	11	В	35	280	0.35	11	В	40	280	0.35	11	В	40
Saturday Midday															
EB L/R	95	0.18	13	В	18	105	0.22	14	В	20	105	0.22	14	В	20
WB L/T/R	275	0.37	12	В	43	295	0.42	13	В	53	300	0.42	13	В	53

Demand, in vehicles

Conclusions

VHB has conducted a traffic impact and access study to assess the potential traffic impacts associated with the proposed redevelopment located at 392-404 Langley Road in Newton, Massachusetts. The proposed redevelopment Project will involve the demolition of one existing building and the construction of an approximately 20-unit residential building, supported by sub-surface parking.

b Volume to capacity ratio.

Average total delay, in seconds per vehicle. C

Level-of-service.

⁹⁵th percentile queue, in feet.

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The proposed redevelopment is expected to increase vehicle trips to the site by approximately 7 (2 entering/5 exiting) vehicle trips during the weekday morning peak hour and approximately 10 (6 entering/4 exiting) vehicle trips during the weekday evening peak hour.

Based on the intersection capacity analysis, it was determined that the project will have minimal impact upon intersection operations at the existing study area intersections.

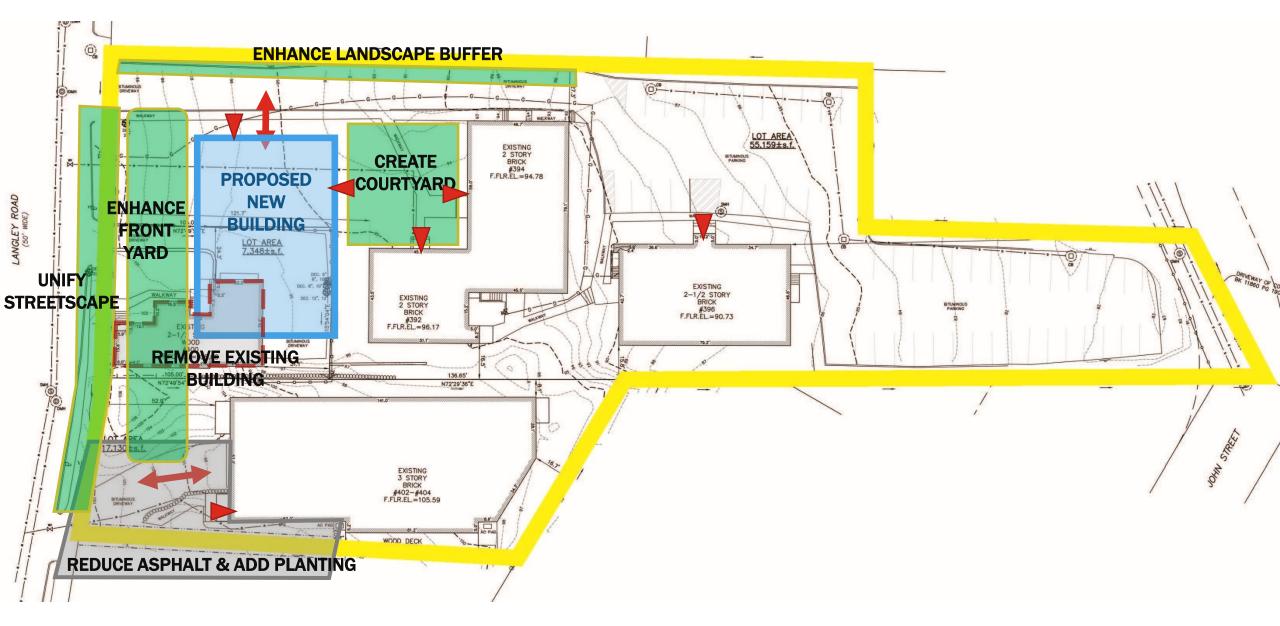


Attachments

- Conceptual Site Plan
- Traffic Volume Count Data
- Seasonal Adjustment Factors
- Vehicular Crash Data
- Sight Distance Worksheet
- Planned Developments
- Trip Generation
- Trip Distribution
- Intersection Capacity Analyses
- Figures



Conceptual Site Plan



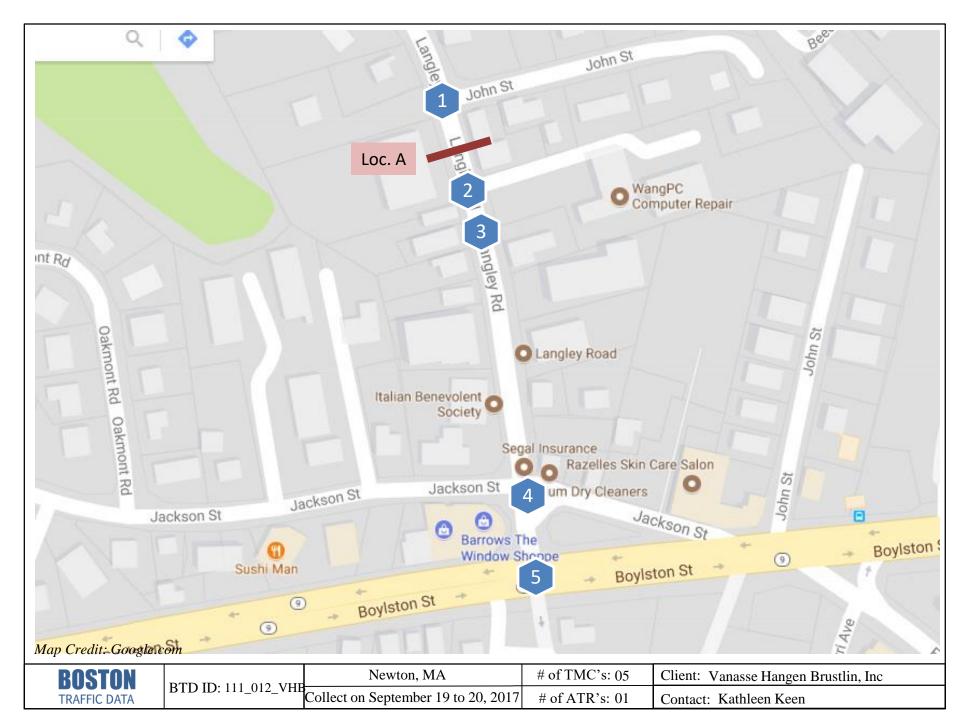
DESIGN CONSIDERATIONS







Traffic Volume Count Data



BTD #: Location 1
Location: Newton, MA
Street 1: Langley Road
Street 2: John Street
Count Date: 9/19/2017
Day of Week: Tuesday
Weather: Rain, 68°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

TOTAL (CARS & TRUCKS)

	Langley Road					Langle	y Road	•		•				John	Street	
		North	bound			South	bound			Easth	ound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
7:00 AM	0	0	30	0	0	1	54	0	0	0	0	0	0	1	0	1
7:15 AM	0	0	34	1	0	0	62	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	46	1	0	0	57	0	0	0	0	0	0	2	0	0
7:45 AM	0	0	47	0	0	0	61	0	0	0	0	0	0	0	0	1
8:00 AM	0	0	51	0	0	0	67	0	0	0	0	0	0	1	0	1
8:15 AM	0	0	60	1	0	1	80	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	61	1	0	0	76	0	0	0	0	0	0	1	0	0
8:45 AM	0	0	68	0	0	0	52	0	0	0	0	0	0	1	0	0

		Langle	y Road			Langle	y Road							John	Street	
		North	bound			South	bound			Easth	oound			Westl	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
4:00 PM	0	0	49	0	0	0	60	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	47	0	0	0	64	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	60	0	0	0	65	0	0	0	0	0	0	0	0	1
4:45 PM	0	0	43	1	0	0	67	0	0	0	0	0	0	1	0	0
5:00 PM	0	0	39	3	0	2	75	0	0	0	0	0	0	2	0	1
5:15 PM	0	0	53	1	0	0	71	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	48	2	0	0	70	0	0	0	0	0	0	1	0	0
5:45 PM	0	0	50	0	0	0	72	0	0	0	0	0	0	0	0	0

AM PEAK H	IOUR		Langle	y Road			Langle	y Road							John	Street	
8:00 AM	1		North	oound			South	bound			Easth	oound			Westl	bound	
to	U	J-Turn	Left	Thru	Right	Right U-Turn Left Thru Right				U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
9:00 AM	1	0	0	240	2	0	0 1 275 0				0	0	0	0	3	0	1
PHF		0.89 0.85							0.	00			0.	50			
HV %		0.0%	0.0%	0.8%	0.0%	0.0%	0.0%	0.7%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%

	PM PEAK HOUR		Langle	y Road			Langle	y Road							John	Street	
	5:00 PM		North	bound			South	bound			Easth	ound			Westl	oound	
	to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
	6:00 PM	0	0	190	6	0 2 288 0				0	0	0	0	0	3	0	1
-	PHF	0.91					0.	94			0.	00			0.	33	
	HV %	0.0%	0.0% 0.0% 0.0% 0.0%				0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%

BTD #: Location 1
Location: Newton, MA
Street 1: Langley Road
Street 2: John Street
Count Date: 9/19/2017
Day of Week: Tuesday
Weather: Rain, 68°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

TRUCKS

		Langle	y Road			Langle	y Road							John	Street	
			bound				bound			Easth	oound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

		Langle	y Road			Langle	y Road							John	Street	
		Northl	oound			South	bound			Eastl	oound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

AM PEAK HOUR		Langle	y Road			Langle	y Road							John	Street	
7:45 AM		North	bound			South	bound			Easth	oound			Westl	bound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
8:45 AM	0	0	2	0	0	0	2	0	0	0	0	0	0	0	0	0
PHF		0.	50			0.	50			0.	00			0.	00	

PM PEAK HOUR	7	Langle	y Road			Langle	y Road							John	Street	
4:00 PM		North	bound			South	bound			Eastl	oound			West	oound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
5:00 PM	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0
PHF		0.	25			0.	25			0.	00			0.	00	

BTD #: Location 1
Location: Newton, MA
Street 1: Langley Road
Street 2: John Street
Count Date: 9/19/2017
Day of Week: Tuesday
Weather: Rain, 68°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

PEDESTRIANS & BICYCLES

		L	angley Roa	ıd		L	angley Roa	ıd							John Street	t	
			Northbound				Southbound				Eastbound				Westbound		
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
7:00 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	2	
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
7:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	3	
8:00 AM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	2	
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	

		L	angley Roa	d		L	angley Roa	ıd							John Street		
			Northbound				Southbound	t			Eastbound				Westbound		
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
4:30 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
5:00 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	2	
5:15 PM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

AM PEAK HOUR1		L	angley Roa	d		L	angley Roa	d							John Street	İ	
8:00 AM			Northbound				Southbound	l			Eastbound				Westbound		
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
9:00 AM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	7	

PM PEAK HOUR ¹		L	angley Roa	d		Į.	angley Roa	d							John Street		
5:00 PM			Northbound				Southbound	i			Eastbound				Westbound		
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
6:00 PM	0	1	0	1	0	1	0	0	0	0	0	0	0	0	0	3	

Peak hours corresponds to vehicular peak hours.

BTD #: Location 2
Location: Newton, MA
Street 1: Langley Road

Street 2: 392-396 Langley Road Driveway

Count Date: 9/19/2017
Day of Week: Tuesday
Weather: Rain, 68°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

TOTAL (CARS & TRUCKS)

								(,						
		Langle	y Road			Langle	y Road						392-	396 Langle	y Road Driv	eway
		North					bound			Easth	oound				bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
7:00 AM	0	0	29	0	0	0	55	0	0	0	0	0	0	1	0	1
7:15 AM	0	0	35	1	0	0	62	0	0	0	0	0	0	3	0	0
7:30 AM	0	0	45	1	0	0	59	0	0	0	0	0	0	1	0	2
7:45 AM	0	0	46	1	0	1	60	0	0	0	0	0	0	2	0	1
8:00 AM	0	0	51	0	0	0	68	0	0	0	0	0	0	1	0	0
8:15 AM	0	0	61	0	0	1	79	0	0	0	0	0	0	3	0	0
8:30 AM	0	0	62	1	0	0	77	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	68	0	0	0	53	0	0	0	0	0	0	0	0	0

		Langle	y Road			Langle	y Road						392-	396 Langle	y Road Driv	eway
		North	bound			South	bound			Easth	oound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
4:00 PM	0	0	48	2	0	0	60	0	0	0	0	0	0	0	0	1
4:15 PM	0	0	45	1	0	1	63	0	0	0	0	0	0	1	0	2
4:30 PM	0	0	57	2	0	1	64	0	0	0	0	0	0	0	0	3
4:45 PM	0	0	43	1	0	0	68	0	0	0	0	0	0	0	0	1
5:00 PM	0	0	41	3	0	1	76	0	0	0	0	0	0	2	0	1
5:15 PM	0	0	52	1	0	0	71	0	0	0	0	0	0	2	0	2
5:30 PM	0	0	49	2	0	1	70	0	0	0	0	0	0	1	0	1
5:45 PM	0	0	50	1	0	0	72	0	0	0	0	0	0	1	0	0

AM PEAK H	IOUR		Langle	y Road			Langle	y Road						392-3	396 Langley	Road Driv	eway
8:00 AM	1		North	oound			South	bound			Easth	oound			Westl	bound	
to	U	J-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
9:00 AM	1	0	0	242	1	0	1	277	0	0	0	0	0	0	4	0	0
PHF			0.8	89			0.	87			0.	00			0.	33	
HV %		0.0%	0.0%	0.8%	0.0%	0.0%	0.0%	0.7%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%

	PM PEAK HOUR		Langle	y Road			Langle	y Road						392-	396 Langley	y Road Driv	eway
	5:00 PM		North	bound			South	bound			Easth	ound			Westl	bound	
	to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
	6:00 PM	0	0	192	7	0	2	289	0	0	0	0	0	0	6	0	4
•	PHF		0.	94			0.	94			0.	00			0.	63	
	HV %	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%

BTD #: Location 2
Location: Newton, MA
Street 1: Langley Road

Street 2: 392-396 Langley Road Driveway

Count Date: 9/19/2017
Day of Week: Tuesday
Weather: Rain, 68°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

TRUCKS

		Langle Northl	y Road bound				y Road bound			Fastl	oound		392-	396 Langley West	y Road Driv bound	eway
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

		Langle	y Road			Langle	y Road						392-	396 Langle _!	y Road Driv	eway
		Northl	bound				bound			Eastl	oound				bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

AM PEAK HOUR]	Langle	y Road			Langle	y Road						392-3	396 Langley	/ Road Drive	eway
7:45 AM		Northl	bound			South	bound			Easth	ound			West	oound	
to	Northbound U-Turn Left Thru Righ				U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
8:45 AM	0	0	2	0	0	0	2	0	0	0	0	0	0	0	0	0
PHF		0.	50			0.	50			0.	00			0.	00	

PM PEAK HOUR	1	Langle	y Road			Langle	y Road						392-3	396 Langley	y Road Drive	eway
4:00 PM		Northbound				South	bound			Easth	oound			West	bound	
to	U-Turn					Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
5:00 PM	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0
PHF		0.	25			0.	25			0.	00			0.	00	

BTD #: Location 2
Location: Newton, MA
Street 1: Langley Road

Street 2: 392-396 Langley Road Driveway

Count Date: 9/19/2017
Day of Week: Tuesday
Weather: Rain, 68°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

PEDESTRIANS & BICYCLES

		L	angley Roa	ıd		L	_angley Roa	ıd						392-396 L	angley Road	d Driveway	
			Northbound				Southbound				Eastbound				Westbound	i	
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	
7:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
8:00 AM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	3	
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	

			angley Roa				angley Roa							392-396 L	angley Roa	d Driveway	
			Northbound				Southbound	d			Eastbound				Westbound		
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
4:30 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
5:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

AM PEAK HOUR ¹ 8:00 AM			angley Roa			L	_angley Roa				Eastbound				angley Road Westbound	d Driveway I	
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
9:00 AM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	8	

PM PEAK HOUR ¹]		angley Roa			L	angley Roa							392-396 La		d Driveway	
5:00 PM			Northbound				Southbound	i			Eastbound				Westbound		
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
6:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	2	

Peak hours corresponds to vehicular peak hours.

BTD #: Location 3
Location: Newton, MA
Street 1: Langley Road

Street 2: Driveway at 400 Langley Road

Count Date: 9/19/2017
Day of Week: Tuesday
Weather: Rain, 68°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

TOTAL (CARS & TRUCKS)

							_			-,						
		Langle	y Road			Langle	y Road						PC Repa	ir Driveway	at 400 Lang	gley Road
		North	bound			South	bound			Easth	ound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
7:00 AM	0	0	29	0	0	0	56	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	36	0	0	0	65	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	46	0	0	0	60	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	47	0	0	0	62	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	51	0	0	1	68	0	0	0	0	0	0	1	0	0
8:15 AM	0	0	61	1	0	0	82	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	62	0	0	0	77	0	0	0	0	0	0	0	0	1
8:45 AM	0	0	68	0	0	1	52	0	0	0	0	0	0	1	0	0

		Langle	y Road			Langle	y Road						PC Repai	ir Driveway	at 400 Lang	gley Road
		North	bound			South	bound			Easth	ound			West	oound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
4:00 PM	0	0	50	0	0	0	60	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	46	1	0	0	64	0	0	0	0	0	0	1	0	0
4:30 PM	0	0	58	0	0	1	63	0	0	0	0	0	0	0	0	1
4:45 PM	0	0	43	1	0	0	68	0	0	0	0	0	0	1	0	1
5:00 PM	0	0	44	0	0	0	78	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	53	0	0	0	73	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	50	0	0	0	71	0	0	0	0	0	0	0	0	1
5:45 PM	0	0	51	0	0	0	73	0	0	0	0	0	0	0	0	0

AM PEAK HOU	R	Langle	y Road			Langle	y Road						PC Repai	ir Driveway	at 400 Lang	gley Road
8:00 AM		North	bound			South	bound			Eastl	oound			West	bound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
9:00 AM	0	0	242	1	0	2	279	0	0	0	0	0	0	2	0	1
PHF		0.	89			0.	86			0.	00			0.	75	
HV %	0.0%	0.0%	0.8%	0.0%	0.0%	0.0%	0.7%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%

PM PEAK HOUR		Langle	y Road			Langle	y Road						PC Repai	r Driveway	at 400 Lang	gley Road
5:00 PM		North	bound			South	bound			Easth	ound			West	oound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
6:00 PM	0	0	198	0	0	0	295	0	0	0	0	0	0	0	0	1
PHF		0.	93			0.	95			0.	00			0.	25	
HV%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%

BTD #: Location 3
Location: Newton, MA
Street 1: Langley Road

Street 2: Driveway at 400 Langley Road

Count Date: 9/19/2017
Day of Week: Tuesday
Weather: Rain, 68°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

TRUCKS

								_								
		Langle	y Road			Langle	y Road						PC Repa	ir Driveway	at 400 Lan	gley Road
		North	bound			South	bound			Eastl	oound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

		Langle	y Road			Langle	y Road						PC Repa	ir Driveway	at 400 Lang	gley Road
		Northl	bound				bound			Eastl	oound			Westl	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

AM PEAK HOUR		Langle	y Road			Langle	y Road						PC Repai	r Driveway	at 400 Lang	jley Road
7:45 AM		Northl	bound			South	bound			Easth	oound			West	bound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
8:45 AM	0	0	2	0	0	0	2	0	0	0	0	0	0	0	0	0
PHF		0.	50			0.	50			0.	00			0.0	00	

ſ	PM PEAK HOUR		Langle	y Road			Langle	y Road						PC Repair	r Driveway	at 400 Lang	ley Road
	4:00 PM		North	bound			South	bound			Eastb	ound			West	oound	
١	to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
	5:00 PM	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0
	PHF		0.	25			0.	25			0.	00			0.	00	

BTD #: Location 3
Location: Newton, MA
Street 1: Langley Road
Street 2: Driveway at 400 Langley Road

Count Date: 9/19/2017
Day of Week: Tuesday
Weather: Rain, 68°F



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PEDESTRIANS & BICYCLES

		L	angley Roa	ıd		L	angley Roa	ıd					PC	Repair Driv	eway at 400	D Langley R	load
			Northbound				Southbound	t			Eastbound				Westbound		
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	
7:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
8:00 AM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	3	
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	

			angley Roa				angley Roa				Eastbound		PC		eway at 400 Westbound		Road
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
4:30 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
5:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

AM PEAK HOUR ¹ 8:00 AM			angley Roa			l	_angley Roa Southbound				Eastbound		PC		eway at 400 Westbound		Road
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
9:00 AM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	8	

PM PEAK HOUR ¹		L	angley Roa	d		L	angley Roa	ıd					PC	Repair Driv	eway at 400) Langley R	oad
5:00 PM			Northbound				Southbound	t			Eastbound				Westbound		
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
6:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	2	

Peak hours corresponds to vehicular peak hours.

BTD #: Location 4
Location: Newton, MA
Street 1: Langley Road
Street 2: Jackson Street
Count Date: 9/19/2017
Day of Week: Tuesday
Weather: Rain, 68°F



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TOTAL (CARS & TRUCKS)

							_			-,						
		Langle	y Road			Langle	y Road			Jackso	n Street			Jackso	n Street	
		North	bound			South	bound			Eastl	oound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
7:00 AM	0	0	0	0	0	0	54	2	0	4	0	6	0	5	5	25
7:15 AM	0	0	0	0	0	0	61	4	0	7	0	7	0	6	7	29
7:30 AM	0	0	0	0	0	0	54	6	0	9	0	8	0	7	6	37
7:45 AM	0	0	0	0	0	0	55	7	0	12	0	9	0	9	8	35
8:00 AM	0	0	0	0	0	0	60	9	0	14	0	10	0	10	7	37
8:15 AM	0	0	0	0	0	0	74	8	0	13	0	11	0	12	8	49
8:30 AM	0	0	0	0	0	0	70	7	0	12	0	9	0	11	6	50
8:45 AM	0	0	0	0	0	0	47	6	0	10	0	8	0	9	5	58

		Langle Northl					y Road bound				n Street oound				n Street bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
4:00 PM	0	0	0	0	0	0	53	7	0	14	0	8	0	14	16	36
4:15 PM	0	0	0	0	0	0	57	8	0	13	0	11	0	16	16	34
4:30 PM	0	0	0	0	0	0	54	9	0	12	0	13	0	17	15	46
4:45 PM	0	0	0	0	0	0	58	11	0	11	0	15	0	18	17	34
5:00 PM	0	0	0	0	0	0	69	9	0	9	0	16	0	19	14	35
5:15 PM	0	0	0	0	0	0	63	10	0	10	0	14	0	18	13	43
5:30 PM	0	0	0	0	0	0	62	8	0	11	0	12	0	16	12	39
5:45 PM	0	0	0	0	0	0	66	7	0	9	0	11	0	14	11	42

A	M PEAK HOUR		Langle	y Road			Langle	y Road			Jackso	n Street			Jackso	n Street	
	8:00 AM		Northl	bound			South	bound			Easth	oound			Westl	bound	
	to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
	9:00 AM	0	0	0	0	0	0	251	30	0	49	0	38	0	42	26	194
	PHF		0.	00			0.	86			0.	91			0.	91	
	HV %	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.8%	3.3%	0.0%	0.0%	0.0%	0.0%	0.0%	2.4%	0.0%	0.5%

PM PEAK HOUR		Langle	y Road			Langle	y Road			Jackso	n Street			Jackso	n Street	
4:30 PM		Northl	bound			South	bound			Eastl	oound			Westl	oound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
5:30 PM	0	0	0	0	0	0	244	39	0	42	0	58	0	72	59	158
PHF		0.	00			0.	91			0.	96			0.	93	
HV%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.4%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	1.4%	0.0%	0.6%

BTD #: Location 4
Location: Newton, MA
Street 1: Langley Road
Street 2: Jackson Street
Count Date: 9/19/2017
Day of Week: Tuesday
Weather: Rain, 68°F



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TRUCKS

		Langle	y Road			Langle	y Road			Jackso	n Street			Jackso	n Street	
			bound				bound			Easth	oound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
8:15 AM	0	0	0	0	0	0	1	1	0	0	0	0	0	1	0	0
8:30 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

			y Road				y Road				n Street				n Street	
		North	bound			South	bound			Eastl	oound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
4:45 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	1	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Γ	AM PEAK HOUR		Langle	y Road			Langle	y Road			Jacksoi	n Street			Jackso	n Street	
	7:30 AM		North	bound			South	bound			Easth	ound			Westl	bound	
	to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
	8:30 AM	0	0	0	0	0	0	1	1	0	0	0	1	0	2	1	1
_	PHF		0.	00			0.	25			0.	25			1.	00	

Γ	PM PEAK HOUR		Langle	y Road			Langle	y Road			Jacksoi	n Street			Jacksoi	n Street	
	4:00 PM		North	bound			South	bound			Easth	oound			West	bound	
	to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
L	5:00 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	1	0	1
_	PHF		0.	00			0.	25			0.	00			0.	50	

BTD #: Location 4
Location: Newton, MA
Street 1: Langley Road
Street 2: Jackson Street
Count Date: 9/19/2017
Day of Week: Tuesday
Weather: Rain, 68°F



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PEDESTRIANS & BICYCLES

		L	angley Roa	ıd		L	angley Roa	ıd		Já	ackson Stre	et		J	ackson Stre	et	
			Northbound	l			Southbound	b			Eastbound				Westbound	l	
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:15 AM	0	0	0	0	0	0	0	1	0	0	0	2	0	0	0	0	
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	
7:45 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	
8:00 AM	0	1	0	0	0	1	0	2	0	0	0	1	0	0	0	0	
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	
8:30 AM	0	0	0	0	0	0	0	1	0	0	0	2	0	0	0	0	
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

			angley Roa				angley Roa				ackson Stre				ackson Stre		
			Northbound				Southbound	i			Eastbound				Westbound		
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	
4:30 PM	0	0	0	0	0	0	0	1	0	0	0	2	0	1	0	0	
4:45 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	
5:00 PM	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	
5:15 PM	0	1	0	0	0	0	0	1	0	0	0	1	0	0	0	0	
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	

AM PEAK HOUR ¹ 8:00 AM			angley Roa Northbound				angley Roa			J	ackson Stre Eastbound				ackson Stre Westbound		
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
9:00 AM	0	1	0	0	0	1	0	3	0	0	0	4	0	0	0	0	

PM PEAK HOUR ¹ 4:30 PM			angley Roa				angley Roa				ackson Stre Eastbound			-	ackson Stre Westbound		
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
5:30 PM	0	1	0	0	0	1	0	3	0	0	0	4	0	1	0	0	

Peak hours corresponds to vehicular peak hours.

BTD #: Location 5
Location: Newton, MA
Street 1: Langley Road
Street 2: Route 9 (Boylston Street)

Count Date: 9/19/2017
Day of Week: Tuesday
Weather: Rain, 68°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

TOTAL (CARS & TRUCKS)

								•		,						
		Langle	y Road			Langle	y Road		R	Route 9 (Bo	ylston Stree	et)	F	Route 9 (Bo	ylston Stree	et)
		North	bound			South	bound			Eastl	oound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
7:00 AM	0	0	0	0	0	44	0	21	0	0	272	0	0	0	386	0
7:15 AM	0	0	0	0	0	51	1	22	0	0	313	0	0	0	397	0
7:30 AM	0	0	0	0	0	45	1	23	0	0	341	0	0	0	417	0
7:45 AM	0	0	0	0	0	51	0	22	0	0	379	0	0	0	421	0
8:00 AM	0	0	0	0	0	57	2	21	0	0	403	0	0	0	426	0
8:15 AM	0	0	0	0	0	71	2	24	0	0	396	0	0	0	415	0
8:30 AM	0	0	0	0	0	69	1	20	0	0	382	0	0	0	419	0
8:45 AM	0	0	0	0	0	47	0	17	0	0	372	0	0	0	407	0

			y Road bound				y Road bound		F		ylston Stree oound	t)	F		ylston Stree bound	et)
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
4:00 PM	0	0	0	0	0	64	2	9	0	0	327	0	0	0	382	0
4:15 PM	0	0	0	0	0	68	2	14	0	0	332	0	0	0	387	0
4:30 PM	0	0	0	0	0	65	1	18	0	0	324	1	0	0	411	0
4:45 PM	0	0	0	0	0	66	0	25	0	0	327	0	0	0	418	0
5:00 PM	0	0	0	0	0	72	1	31	0	0	318	0	0	0	429	0
5:15 PM	0	0	0	0	0	66	1	28	0	0	327	0	0	0	424	0
5:30 PM	0	0	0	0	0	65	0	25	0	0	323	0	0	0	430	0
5:45 PM	0	0	0	0	0	69	0	22	0	0	319	0	0	0	419	0

AM PEAK HOU	₹	Langle	y Road			Langle	y Road		R	Route 9 (Bo	ylston Stree	t)	R	oute 9 (Boy	ylston Stree	t)
7:45 AM		North	bound			South	bound			Eastl	oound			Westl	bound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
8:45 AM	0	0	0	0	0	248	5	87	0	0	1560	0	0	0	1681	0
PHF		0.	.00			0.	88			0.	97			0.	99	
HV%	0.0%	0.0%	0.0%	0.0%	0.0%	0.8%	0.0%	1.1%	0.0%	0.0%	1.0%	0.0%	0.0%	0.0%	0.8%	0.0%

PM PEAK HOUR	7	Langle	y Road			Langle	y Road		F	Route 9 (Boy	/Iston Stree	t)	R	Route 9 (Boy	/Iston Stree	t)
4:45 PM		North	bound			South	bound			Easth	ound			West	oound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
5:45 PM	0	0	0	0	0	269	2	109	0	0	1295	0	0	0	1701	0
PHF		0.	00			0.	91			0.	99			0.	99	
HV%	0.0%	0.0%	0.0%	0.0%	0.0%	0.4%	0.0%	0.9%	0.0%	0.0%	1.3%	0.0%	0.0%	0.0%	0.8%	0.0%

BTD #: Location 5
Location: Newton, MA
Street 1: Langley Road
Street 2: Route 9 (Boylston Street)

Count Date: 9/19/2017
Day of Week: Tuesday
Weather: Rain, 68°F



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TRUCKS

								,,,,	O, 10							
		Langle	y Road			Langle	y Road		F	Route 9 (Boy	ylston Stree	t)	F	Route 9 (Bo	ylston Stree	ŧt)
		North				South	bound				oound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
7:00 AM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	3	0
7:15 AM	0	0	0	0	0	1	0	0	0	0	6	0	0	0	4	0
7:30 AM	0	0	0	0	0	0	0	1	0	0	5	0	0	0	3	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	5	0	0	0	3	0
8:15 AM	0	0	0	0	0	1	0	1	0	0	4	0	0	0	2	0
8:30 AM	0	0	0	0	0	1	0	0	0	0	3	0	0	0	4	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	3	0

		Langle	y Road			Langle	y Road		F	Route 9 (Bo	ylston Stree	t)	R	Route 9 (Bo	ylston Stree	t)
		North	bound			South	bound			Eastl	oound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
4:00 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	3	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	3	0	0	0	2	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	3	0
4:45 PM	0	0	0	0	0	1	0	0	0	0	3	0	0	0	3	0
5:00 PM	0	0	0	0	0	0	0	1	0	0	5	0	0	0	4	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	2	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	5	0	0	0	4	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3	0

ſ	AM PEAK HOUR		Langle	y Road			Langle	y Road		R	oute 9 (Boy	ston Stree	t)	R	oute 9 (Boy	/Iston Stree	t)
	7:15 AM		Northl	bound			South	bound			Easth	ound			West	oound	
	to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
	8:15 AM	0	0	0	0	0	1	0	1	0	0	20	0	0	0	14	0
	PHF		0.	00	•		0.	50			0.	83	•		0.	88	

PM PEAK HOUR		Langle	y Road			Langle	y Road		R	Route 9 (Boy	/Iston Stree	t)	R	oute 9 (Boy	ylston Street	t)
4:45 PM		Northl	bound			South	bound			Easth	ound			Westl	bound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
5:45 PM	0	0	0	0	0	1	0	1	0	0	17	0	0	0	13	0
PHF		0.	00	-		0.	50		-	0.	85			0.	81	

BTD #: Location 5
Location: Newton, MA
Street 1: Langley Road
Street 2: Route 9 (Boylston Street)

Count Date: 9/19/2017 Day of Week: Tuesday Weather: Rain, 68°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

PEDESTRIANS & BICYCLES

		L	angley Roa	d		L	angley Roa	ıd		Route	9 (Boylston	Street)		Route	9 (Boylston	Street)	
			Northbound				Southbound	b			Eastbound				Westbound	l	
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:15 AM	0	0	0	0	0	0	0	1	0	0	0	2	0	0	0	0	
7:30 AM	0	0	0	1	0	0	0	0	0	0	0	1	0	0	0	0	
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	
8:00 AM	0	0	0	0	1	0	0	1	0	0	0	2	0	0	1	0	
8:15 AM	0	0	0	1	0	0	0	0	0	0	0	1	0	0	0	0	
8:30 AM	0	0	0	0	0	0	0	1	0	0	0	2	0	0	0	0	
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

			angley Roa Northbound				angley Roa				9 (Boylston Eastbound				9 (Boylston Westbound		
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	
4:30 PM	0	0	0	1	0	0	0	1	0	0	0	2	0	0	0	0	
4:45 PM	0	0	0	0	0	0	1	0	0	0	0	2	0	0	0	0	
5:00 PM	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	
5:15 PM	0	0	0	0	0	0	0	1	1	0	0	2	0	0	0	0	
5:30 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	

AM PEAK HOUR]	L	angley Roa	d		L	_angley Roa	d		Route	9 (Boylston	Street)		Route	9 (Boylston	Street)	
7:45 AM	1		Northbound				Southbound				Eastbound	•			Westbound		
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
8:45 AM	0	0	0	1	1	0	0	2	0	0	0	6	0	0	1	0	

PM PEAK HOUR ¹ 4:45 PM		L	angley Roa			L	angley Roa	ıd		Route	9 (Boylston Eastbound	,			9 (Boylston Westbound		
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	
5:45 PM	0	0	0	1	0	0	1	2	1	0	0	5	0	0	0	0	

Peak hours corresponds to vehicular peak hours.



Seasonal Adjustment Factors

MASSACHUSETTS HIGHWAY DEPARTMENT - STATEWIDE TRAFFIC DATA COLLECTION

2011 WEEKDAY SEASONAL FACTORS *

1105,1106,1107,1108,1113,1114,1116,2196,2197,2198

^{*} Note: These are weekday factors. The average of the factors for the year will not equal 1, as weekend data are not considered.

FACTOR GROUP	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	ОСТ	NOV	DEC
GROUP 1 - WEST INTERSTATE	0.98	0.93	0.90	0.89	0.90	0.88	0.91	0.90	0.89	0.89	0.93	0.95
Use group 2 for R5, R6, & R0 GROUP 2 - RURAL MAJOR COLLECTOR (R-5)	1.12	1.12	1.07	0.99	0.91	0.90	0.86	0.86	0.92	0.93	1.01	1.05
GROUP 3A - RECREATIONAL **(1-4) See below	1.26	1.25	1.20	1.06	0.96	0.89	0.76	0.76	0.92	0.99	1.08	1,14
GROUP 3B - RECREATIONAL ***(5) See below	1.22	1.26	1.22	1.06	0.96	0.90	0.72	0.74	0.97	1.02	1.14	1.15
GROUP 4 - I-495 INTERSTATE	1.02	1.00	1.00	0.96	0.92	0.89	0.85	0.83	0.93	0.96	1.01	1.03
GROUP 5 - EAST INTERSTATE	1.04	1.00	0.96	0.93	0.92	0.91	0.91	0.89	0.93	0.93	0.96	1.01
Use group 6 for U2, U3, U5, U6, U0, R2, & R3 GROUP 6 - URBAN ARTERIALS, COLLECTORS & RURAL ARTERIALS (R-2, R-3)	1.03	1.01	0.96	0.92	0.91	0.90	0.92	0.92	0.93	0.92	0.97	0.97
GROUP 7 - I-84 PROXIMITY (STAS. 17,3921)	1.24	1.24_	1.15	1.04	0.99	1.00	0.93	0.89_	1.05	1.05	1.05	1.12
GROUP 8 - I-295 PROXIMITY (STA. 6590)	1.00	0.99	0.95	0.92	0.94	0.91	0.93	0.92	0.95	0.94	0.97	0.95
GROUP 9 - I-195 PROXIMITY (STA. 7)	1.13	1.05	1.03	0.95	0.89	0.87	0.86	0.79	0.88	0.91	0.99	1.03

RECREATIONAL: (ALL YEARS)	2011 AXLE CORRECT	ION FACTORS	ROUND OFF
**GROUP 3A: 1. CAPE COD (ALL TOWNS) 2.PLYMOUTH(SOUTH OF RTE:3A)	ROAD INVENTORY FUNCTIONAL CLASSIFICATION RURAL	AXLE CORRECTION FACTOR	0 - 99910 > 1,000100
7014, 7079,7080,7090,7091,7092,7093,7094,7095,7096,7097,7108,7178 3.MARTHA'S VINEYARD 4.NANTUCKET	1 2 3 0,5,6	0.95 0.97 0.98 0.98	
***GROUP 3B: 5.PERMANENTS 2 & 189 1066,1067,1083,1084,1085,1086,1087,1088,1089,1090,1091,1092, 1093,1094,1095,1096,1097,1098,1099,1100,1101,1102,1103,1104,	URBAN 1 2 3 5 0.6	0.96 0.98 0.98 0.98 0.99	_ ·

I-84

Apply I-84 factor to stations: 3290,3929

0.90



Vehicular Crash Data



INTERSECTION CRASH RATE WORKSHEET

CITY/TOWN : Newton, M	<u>A</u>			COUNT DA	TE:	9/19/2017
DISTRICT: 6	UNSIGN	IALIZED :	Х	SIGNA	LIZED :	
		~ INT	0.53 TERSECTION	I DATA ~		0.70
MAJOR STREET :	Langley Roa		LNOLOTION	LAIA		
	'	<u>u</u>				
IINOR STREET(S):	John Street					
INTERSECTION DIAGRAM (Label Approaches)	North		Langley Roa	d	John Street	
			PEAK HOUF	R VOLUMES		
APPROACH:	1	2	3	4	5	Total Peak Hourly
DIRECTION:	NB	SB	EB	WB		Approach Volume
PEAK HOURLY VOLUMES (AM/ PM) :	200	290		5		495
"K" FACTOR:	0.090	INTERSI	ECTION ADT APPROACH	` '	AL DAILY	5,500
OTAL # OF CRASHES :	1	# OF YEARS :	5	CRASHES	GE#OF PERYEAR(.):	0.20
CRASH RATE CALCU	ILATION :	0.10	RATE =	<u>(A * 1,0</u>	000,000) * 365)	
Comments : <u>MassDOT</u>	Accident Data	a (2010-2014)				
oject Title & Date:	Micozzi New	ton, 14019.00				



INTERSECTION CRASH RATE WORKSHEET

CITY/TOWN : Newton, M	<u>IA</u>			COUNT DA	TE:	9/19/2017
DISTRICT: 6	UNSIGN	IALIZED :	Х	SIGNA	LIZED :	
		~ IN1	0.53 TERSECTION	I DATA ~		0.70
MAJOR STREET :	Langley Roa	d				
MINOR STREET(S):	Driveway at	392-396 Lang	ley Road			
INTERSECTION DIAGRAM (Label Approaches)	North		Langley Roa	d	Driveway	
			PEAK HOUF	R VOLUMES		
APPROACH:	1	2	3	4	5	Total Peak Hourly
DIRECTION:	NB	SB	EB	WB		Approach Volume
PEAK HOURLY VOLUMES (AM/ PM) :	200	295		10		505
"K" FACTOR:	0.090	INTERSI	ECTION ADT APPROACH		AL DAILY	5,611
OTAL # OF CRASHES :	2	# OF YEARS :	5	CRASHES	GE#OF PERYEAR(.):	0.40
CRASH RATE CALCU	ILATION :	0.20	RATE =	(A * 1,0 (V	000,000) * 365)	
Comments : MassDOT		•				_
Project Title & Date:	Micozzi New	ton, 14019.00				



INTERSECTION CRASH RATE WORKSHEET

CITY/TOWN : Newton, M	COUNT DATE : 9/19/2					
DISTRICT: 6	UNSIGNALIZED: X			SIGNA	LIZED :	0.70
		~ IN7	TERSECTION	I DATA ~		
MAJOR STREET :	Langley Roa	d				
MINOR STREET(S):	Driveway at	400 Langley F	Road			
INTERSECTION DIAGRAM (Label Approaches)	North		Langley Roa	d	Driveway	
			PEAK HOUF	VOLUMES		
APPROACH:	1	2	3	4	5	Total Peak Hourly
DIRECTION:	NB	SB	EB	WB		Approach Volume
PEAK HOURLY VOLUMES (AM/ PM) :	200	300		0		500
"K" FACTOR:	0.090	INTERSI	ECTION ADT APPROACH	` '	AL DAILY	5,556
OTAL # OF CRASHES :	0	# OF YEARS :	5	CRASHES	GE#OF PERYEAR(.):	0.00
CRASH RATE CALCU	ILATION :	0.00	RATE =	(A * 1,0	000,000) * 365)	
Comments : MassDOT	Accident Data	a (2010-2014))			
roject Title & Date:	Micozzi New	ton, 14019.00				



INTERSECTION CRASH RATE WORKSHEET

CITY/TOWN : Newton, M	<u>A</u>		COUNT DATE : 9/19/2017				
DISTRICT: 6	UNSIGN	ialized : ~ in ⁻	X 0.53 FERSECTION	•	ALIZED :	0.70	
MAJOR STREET :	Jackson Stre	eet					
MINOR STREET(S):	Langley Roa	d					
INTERSECTION DIAGRAM (Label Approaches)	North	Jackson Sti					
APPROACH :	1	2	PEAK HOUF	4	5	Total Peak Hourly	
DIRECTION :	NB	SB	EB	WB		Approach Volume	
PEAK HOURLY VOLUMES (AM/ PM) :		300	95	275		670	
"K" FACTOR:	0.090	INTERS	ECTION ADT APPROACH	` '	AL DAILY	7,444	
TOTAL # OF CRASHES :	5	# OF YEARS :	5	CRASHES	GE#OF PERYEAR(\(\):	1.00	
CRASH RATE CALCU	LATION :	0.37	RATE =	<u>(A * 1,</u> (000,000) * 365)		
Comments : <u>MassDOT</u> Project Title & Date:							
Tojour Tillo & Dato.	IVIIOOZZI INGW	170 10.00					



INTERSECTION CRASH RATE WORKSHEET

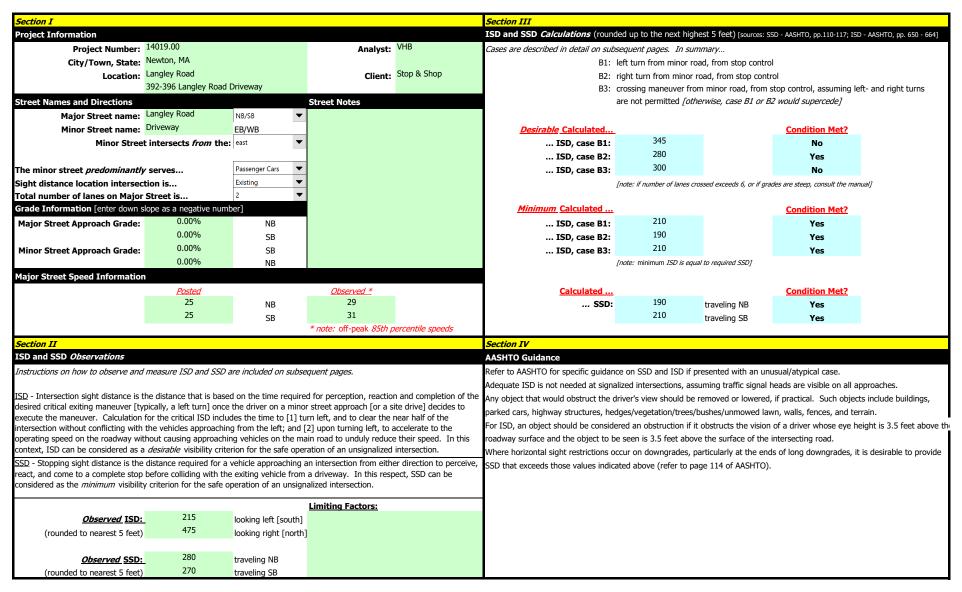
CITY/TOWN : Newton, MA					TE:	9/19/2017
DISTRICT: 6	UNSIGN	UNSIGNALIZED :			LIZED :	Х
			0.53			0.70
		~ IN	TERSECTION	I DATA ~		
IAJOR STREET :	Route 9 (Boy	ylston Street)				
IINOR STREET(S):	Langley Roa	ıd				
		1	5			
			Langley Roa	d 		
INTERSECTION	North					
DIAGRAM						
(Label Approaches)					Route 9	
			Driveway			
			Dirveway	l		
			PEAK HOUF	R VOLUMES		T-4-I DI
APPROACH:	1	2	3	4	5	Total Peak Hourly
DIRECTION:	NB	SB	EB	WB		Approach Volume
PEAK HOURLY VOLUMES (AM/ PM) :		385	1,285	1,700		3,370
"K" FACTOR:	0.090	INTERS	ECTION ADT APPROACH	` '	AL DAILY	37,444
OTAL # OF CRASHES :	25	# OF YEARS :	5	CRASHES	GE#OF PERYEAR(.):	5.00
CRASH RATE CALCU	JLATION :	0.37	RATE =	<u>(A * 1,0</u>	000,000) * 365)	
Comments : <u>MassDOT</u>	Accident Dat	a (2010-2014)	•			
oject Title & Date:	Micozzi New	rton, 14019.00	ı			

Martin	Crash Number	Crash Date	Weekday	Crash Time City/Town	MassHighway District Crash Severity Number of NonFatal Injuries	Number of Fatal Injuries Manner of Collisio	vehicle Action Prior to Crash	Vehicle Travel Direction	ons Most Harmful Events	Vehicle Configuration Non Motorist	Type Road Surface	ce Ambient Light	Weather Condition	on Roadway	Distance And Direction From Intersection	Near Intersection Roadway	Police Agency RM	IV Document #	Report ID:
Martin M	Langley Road at John Str	eet											1		1	1			
Column C							V1: Travelling straight ahead / V2:Parked /	,		V1-(Passanger car) V2-(Passanger car)									ı
Mark 1			Sunday	12:04 AM NEWTON	6 Not Reported	0 Rear-end		V1:N / V2:N / V3:N			Dry	Dark - lighted roadway	Clear/Unknown	LANGLEY RD			Local police PW7	201035500815	10000013
March Marc	Langley Road at 392-396	Langley Road Dr	riveway																
Martin M					Property damage only				V1:(Collision with motor vehicle in traffic)										ı
Column C	2980108	3/20/2012	Tuesday	12:17 PM NEWTON	6 (none injured)	0 O Angle	V1: Travelling straight ahead / V2:Parked	V1:N / V2:8	V2:(Collision with motor vehicle in traffic)	V1:(Motorcycle) V2:(Passenger car)	Dry	Daylight	Clear	LANGLEY RD			Local police PW2	201208700703	12000004
State Stat	2269665	2/2/2012	Saturday	11-51 AM NEWTON		0 Single vehicle crash	V1- Travelling straight ahead	V1-5			Dou	Daylight	Clear/Clear	I ANGLEV PD			Local police PW	201207400621	I 1 2000000
March 180 18			Saturday	11.31 AW NEW YOR	a (none injurea)	o Single venicle crash	v1. Havening straight ahead	¥1.5	bulluling, turnier, etc.//	sport utility) with only rour tresj	ыу	Dayiigiit	ciedi/ciedi	DANGLET KD			cocai police PW2	.01307400031	1300000
March Marc																	$\overline{}$		
March Marc	2674592	12/20/2010	Monday	3:39 PM NEWTON	6 Not Reported	0 Rear-end		V1:S / V2:S		V1:(Passenger car) V2:(Passenger car)	Ice	Daylight	Snow/Cloudy	/ LANGLEY RD			Local police PW	201035800232	1000001
Martin M												Dark - roadway not							
Mary	2785719	10/18/2011	Tuesday	8:41 PM NEWTON	6 Not Reported	0 O Angle		V1:S / V2:W		V1:(Passenger car) V2:(Passenger car)	Dry		Clear/Clear	RD			Local police PW7	201129800608	1100001
Martin M										V1:(Light truck(van, mini-van, panel, pickup,									ı
Market M	3201799							V1:E			Dry	Daylight	Clear	RD					
Mark	3592130	9/9/2013	Monday	3:09 PM NEWTON	6 (none injured)	0 0 direction	V1: Not reported / V2:Parked	V1:8 / V2:N	traffic)	V1:() V2:(Passenger car) V1:(Passenger car) V2:(Light truck(van. mini-	Dry	Daylight	Clear/Clear	LANGLEY RD			Local police PW2	201326702629	1300000
	2947275	6/10/2014	Tuorday	2-00 DM NEWTON			1/1: Entaring traffic lane / 1/2: Parked	V1-5 / V2-9		van, panel, pickup, sport utility) with only	Dov	Daylight	Not Paparted	IACKSON ST		LANGLEY POAD	Local police PW	201416900622	I 1 4000000
March Marc		0/10/2014	ruesuay	3.00 FW NEW TON	o (none injurea)	o direction	V1. Entering traincrane / V2.raixed	V1.3/ V2.0	vz.(comsion with motor venice in dame)	iour triesy	Diy	Daylight	Not Reported	JACKSON ST		DANGLET ROAD	cocai police PW2	.01410800023	1400000
March Marc																			
The column	2606562	5/27/2010	Thursday	5:40 PM NEWTON		0 Rear-end		V1:W / V2:W			Dry	Daylight	Clear	STREET Rte 9		LANGLEY ROAD	State police PW	201016000513	2010-0H5 001639
Market M														Rto 9 /					
Marchan Marc	20000-	44 /0 /00 : -	Mandau	2.42 044 151 751			V1: Travelling straight ahead / V2:Travelling	ig	V2:(Collision with motor vehicle in traffic)	V1:(Passenger car) V2:(Passenger car)	146	Davlicht	Daia	LANGLEY			State mel'	201021 1001-	2010-0H5
March Marc	2659867	11/8/2010	nivionday	2:43 PM NEWTON		U Angle		v1:w / V2:W / V3:W		V1:(Light truck(van, mini-van, panel, pickup,	Wet	paylight	Kain	BOYLSTON			state police PW2	.01031400454	
Part	2668214	11/28/2010	Sunday	12:55 PM NEWTON		0 Rear-end		V1:E / V2:E			Dry	Daylight	Clear	STREET Rte 9		LANGLEY ROAD	State police PW	201033500309	2010-0H5 003807
March Marc																			
And					Proporty day are only		V1: Slowing or stone die troffie /		V1-/Collision with mater unhight in **** ****	sport utility) with only four tires) V2:(Light									lanta ove
March Marc	3287971	10/17/2012	Wednesday	2:00 PM NEWTON		0 Rear-end		V1:E / V2:E		utility) with only four tires)	Dry	Daylight	Clear	E		LANGLEY ROAD	State police PW7	201231400419	005215
And Model and					Property damage only		V1: Slowing or stopped in traffic /												2011-0H5
Second	3377431	8/2/2011	Tuesday	2:50 PM NEWTON		0 0 Rear-end		V1:E / V2:E	V2:(Collision with motor vehicle in traffic)	V1:(Passenger car) V2:(Passenger car)	Dry	Daylight	Clear	E BOYLSTON		LANGLEY ROAD	State police PW2	201308700406	002285
March Marc	2475962	6/15/2012	Saturday	7-45 DM NEWTON				V1-W//V2-W/		V1-/Parconner carl V2-(MORED)	Dou	Daylight	Clear	STREET Rte			State police DW	201217501426	2013-0H5
Martin M	3473303	0/13/2013	Saturday	7.45 FW NEWTON		o direction		V1.W/ V2.W	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	V1:(Light truck(van, mini-van, panel, pickup,	biy	Daylight	Clear	BOYLSTON			State ponce FW2	.01317301420	
1	3820564	5/28/2014	Wednesday	10:26 PM NEWTON		0 Rear-end		V1:E / V2:E			Dry	Dark - lighted roadway	Clear	E		LANGLEY ROAD	State police PW7	201415001311	
Part Column Part														Rte 9 W / LANGLEY					2013-0H5-
March Marc	3414439	4/10/2013	Wednesday	11:25 PM NEWTON	6 Non-fatal injury	1 0 Single vehicle crash	V1: Turning right	V1:W			Wet	Dark - lighted roadway	Rain				State police PW2	201313401311	J01867
Page 1985	3491317	6/27/2013	Thursday	3:46 PM NEWTON				v1·w / v2·w			Wet	Daylight	Cloudy				State police PW	201318301026	2013-0H5-
Property	3431317	0/27/2013	indisday	3.4011111112111011	o (none marca)	o direction	or orgin circus	72.17 72.17	remark in during	ion area	· · ·	Dayiigit	cioudy	LANGLEY			State ponce 1 W2	01310301020	1
Description Company							V1: Slowing or stopped in traffic /		V1:(Collision with motor vehicle in traffic)	V1:(Passenger car) V2:(Single-unit truck (2-				BOYLSTON					2013-0H5
State Stat	3502217	7/1/2013	Monday	1:49 PM NEWTON	6 (none injured)	0 0 direction		V1:S / V2:S	V2:(Collision with motor vehicle in traffic)		Dry	Daylight	Cloudy	BOYLSTON			State police PW2	:01318600804	J03721
STOTE Description Descri	3561454	8/17/2013	Saturday	1:10 AM NEWTON				V1:E / V2:E			Dry	Dark - lighted roadway	Blowing sand, snow			LANGLEY ROAD	State police PW	201323201005	2013-0H5 004891
State Stat							V1: Travelling straight shead / V2:Slowing			V1:(Light truck(van, mini-van, panel, pickup,									
1500 1500	3857631	5/30/2014	Friday	12:18 PM NEWTON		0 0 Rear-end		V1:W / V2:W		V2:(Passenger car)	Dry	Daylight	Clear	w	0 feet W of	LANGLEY ROAD	State police PW7	201417100405	002970
Part										van, panel, pickup, sport utility) with only									ı
Part	3901903	7/16/2014	Wednesday	10:07 AM NEWTON	6 (none injured)	0 0 Rear-end	straight ahead	V1:S / V2:S	V2:(Collision with motor vehicle in traffic)		Wet	Daylight	Rain	ST			Local police PW2	201422401054	14000007
1							V1: Slowing or stopped in traffic /		V1:(Collision with motor vehicle in traffic)	sport utility) with only four tires) V2:(Light				BOYI STON					ı
Part	2246774	12/26/2012	Modporday	12-05 DM NEWTON		0 Rearrand	V2:Slowing or stopped in traffic /	V1-E / V2-E / V2-9	V2:(Collision with motor vehicle in traffic)		Dov	Daylight	Cloudy			LANGLEY POAD	State police DW	201202000127	2012-0H5-
315755 17/2/57 Probability 5.44 Probertify 5.44 Proberti	3340274	12/20/2012	weunesday	12.03 PW NEWTON	a (none injured)	u near-enu		V1.E / V2.E / V3.8	vs.(collision with motor vehicle in traffic)		DIY	Daylight	Cloudy			DANGLET ROAD	State police PW2	.01302600127	JU0532
Part	3352158	1/31/2013	Thursday	5:44 PM NEWTON	Property damage only 6 (none injured)	0 Rear-end	v1: Slowing or stopped in traffic / V2:Travelling straight ahead	V1:W / V2:W	v1:(Collision with motor vehicle in traffic) V2:(Collision with motor vehicle in traffic)	four tires)	Dry	Dark - lighted roadway	Cloudy	W SIREET Rte 9	,	LANGLEY ROAD	State police PW:	201303900614	2013-0H5 000636
1/2 1/2																			- I
1965 1965							V1: Slowing or stopped in traffic / V2:Slowing or stopped in traffic /		V1:(Collision with motor vehicle in traffic) V2:(Collision with motor vehicle in traffic)	V2:(Passenger car) V3:(Light truck(van, mini-				BOYLSTON STREET Rte C					2013-0H5
355500 5-74/2036 Modesday 8.27 PM SENTON 6 Grow Support Jamage only 5 Grow Support Jamage only 5 Grow Support Jamage only 5 Grow Support Jamage only 6 Grow Support Jamage only 6 Grow Support Jamage only 6 Grow Support Jamage only 7 Grow	3708781	12/11/2013	Wednesday	4:17 PM NEWTON	6 Non-fatal injury	1 0 Rear-end	V3:Travelling straight ahead	V1:W / V2:W / V3:W			Dry	Dark - lighted roadway	Clear	w		LANGLEY ROAD	State police PW7		
385400 5/14/724 whethersides 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1										V1:(Light truck(van, mini-van, panel, pickup,									i
358003 574/2016 reduction 574/2016						Sideswipe, same	V1: Travelling straight ahead / V2:Travellin	ng .		truck(van, mini-van, panel, pickup, sport									2014-0H5
Sistence	3804303	5/14/2014	Wednesday	8:17 PM NEWTON		0 0 direction	straight ahead	V1:W / V2:W		V1:(Light truck(van, mini-van, panel, pickup,	Dry	Dark - lighted roadway	Clear			LANGLEY ROAD	State police PW2	:01414100513	J02636
Property damage only 10/14/2014 Thursday 9.17 AM NEWTON 9.17 AM	3910649	8/1/2014	Friday	2:25 PM NEWTON	6 Non-fatal injury		V1: Travelling straight ahead / V2:Changing			sport utility) with only four tires)	Drv	Davlight	Clear			LANGLEY ROAD	State police PW	201423700976	2014-0H5 004226
373614 1016/2014 Thursday 9.17 AM NEWTON 6 (none injured) 0 0 8 are-med ahead V.1.F./V.2.E V.2.(collision with motor vehicle in traffic) 2. V.2. Passenger carl V.2. P	2320049	-, -, 2014	,	,		Janeedon	V1: Turning right / V2:Texasiling stee*-bit	,			,	.,					1 102	,,	
Second 10.48 M NewTON 6 Non-fatal injury 1 0 Rear-end V1:Parked / V2:Travelling straight ahead V1:Parked / V2:Travelling straight V1:Parked / V2:Parked / V2:P	3973614	10/16/2014	Thursday	9:17 AM NEWTON		0 Rear-end		V1:E / V2:E		V2:(Passenger car)	Wet	Daylight	Cloudy	E		LANGLEY ROAD	State police PW7	201431703705	
2595406 4/29/2010 Thursday 9:13 PM NEWTON 6 Non-fatal injury 1 0 Angle shead 4/29/2010 Thursday 9:13 PM NEWTON 6 Non-fatal injury 1 0 Angle shead 4/29/2010 Thursday 9:13 PM NEWTON 6 Non-fatal injury 1 0 Angle shead 4/29/2010 Monday 1:58 PM NEWTON 6 (none injured) 0 0 Rear-end 9/2010 Monday 1:58 PM NEWTON 6 (none-fatal injury 1 1 0 direction shead 1/20 Travelling straight ahead / 12-Travelling straight shead / 12-Travelling straigh										van, panel, pickup, sport utility) with only				STREET Rte 9					2010-0H5
2595406 4/29/2010 Thursday 9:13 PM NEWTON 5 Non-fatal injury 1 0 Angle ahead V1:E/V2:W V2:Collision with motor vehicle in traffic) V2:Fpasenger car) V2:Fpas	2591802	4/21/2010	Wednesday	10:48 AM NEWTON	6 Non-fatal injury	1 0 Rear-end		V1:8 / V2:W		four tires)	Dry	Daylight	Clear			LANGLEY ROAD Rte LANGLE	State police PW2	:01011900130	J01209
Property damage only 2605545 5/24/2010 Monday 1:58 PM NEWTON 6 (none injured) 0 Rear-end V2:Slowing or stopped in traffic V1:E/V2:E V2:[collision with motor vehicle in traffic) V3:[passenger car] V2:[passenger car] V3:[passenger car	2595406	4/29/2010	Thursday	9:13 PM NEWTON	6 Non-fatal injury	1 O Angle		V1:E / V2:W	V1:(Collision with motor vehicle in traffic) V2:(Collision with motor vehicle in traffic)	V1:(Passenger car) V2:(Passenger car)	Drv	Dark - lighted roadway	Clear			LANGLEY ROAD	State police PW	201013000404	2010-0H5 001290
260554\$ 5/24/2010 Monday 1:58 PM NEWTON 6 (none injured) 0 0 Rear-end V2:5lowing or stopped in traffic V1:£ V2:E V2:(Collision with motor vehicle in traffic) V1:(Passenger car) V2:(Passenger car) V2:(Pas	2000.00	, 23, 2320	,			a 1					,	. January							
319970 7/6/2012 Friday 11:50 PM NEWTON 6 Non-fatal injury 1 Sideswipe, same V1: Changing lanes / V2: Travelling straight a head V2: Travelling straight a h	2605545	5/24/2010	Monday	1:58 PM NEWTON		0 Rear-end	V2:Slowing or stopped in traffic	V1:E / V2:E		V1:(Passenger car) V2:(Passenger car)	Dry	Daylight	Clear	ROAD			State police PW7	201015800649	
Property damage only 3293369 11/20/2012 Tuesday 8.25 AM NEWTON 6 (none injured) Property damage only 41: Travelling straight ahead / V2: Travelling straight									V1:(Collision with motor vehicle in traffic)										2012-0H5
sport vility) with norty or tries) V2: Travelling straight ahead / V2: Travelling straight ahe	3190970	7/6/2012	Friday	11:50 PM NEWTON	6 Non-fatal injury	1 0 direction	ahead	V1:W / V2:W	V2:(Collision with motor vehicle in traffic)	V1:(Passenger car) V2:(Passenger car)	Dry	Dark - lighted roadway	Not Reported	W	0 feet W of	LANGLEY ROAD	State police PW2	201220100508	303352
Property damage only V1: Travelling straight ahead / V2:Travelling V1: (Collision with motor vehicle in traffic) U1: (Collision with motor vehicle in traffic) U2: (V2: E V2: (Collision with motor vehicle in traffic) U1: (V2: E V2: (Collision with motor vehicle in traffic) U1: (V3: E V2: (Collision with motor vehicle in traffic) U2: (V3: E V3: (Collision with motor vehicle in traffic) U2: (V3: E V4: (Collision with motor vehicle in traffic) U2: (V3: E V4: (Collision with motor vehicle in traffic) U3: (V4: (Colli														BOYLSTON					İ
V1:(Passenger car) V2:(Light truck(van, mini- V1:Slowing or stopped in traffic / V1:(Collision with motor vehicle in traffic) van, panel, pickup, sport utility) with only V1:Slowing or stopped in traffic / V1:(Collision with motor vehicle in traffic) van, panel, pickup, sport utility) with only	2202250	11/20/2012	Tuerday	9-75 ABAINCHTON		0 000000		NA -E / NA -E		truck(van, mini-van, panel, pickup, sport	Der	Daylight	Not Poperted			LANGLEY BOAD	State police		2012-0H5
	3293369	11/20/2012	i desudy	0.23 AIWI NEW IUN		U kear-end		V1.E / V2.E		V1:(Passenger car) V2:(Light truck(van, mini-	DIY	Daylight	ivot neported	BOYLSTON		CHINGLET RUAD	state police PW2	v12331UUb22	
	3548884	7/19/2013	Friday	3:23 PM NEWTON		0 Rear-end		V1:E / V2:E	V1:(Collision with motor vehicle in traffic) V2:(Collision with motor vehicle in traffic)	van, panel, pickup, sport utility) with only four tires)	Dry	Daylight	Clear				State police PW:	201321900609	2013-0H5- 004208



Sight Distance Worksheet

Stopping Sight Distance and Intersection Sight Distance Calculator [v0.97] Based on 'A Policy on Geometric Design of Highways and Streets', AASHTO, 2004





Planned Developments

		BACKGROUND	DEVELOPMENT
INTERSECTION	MOVEMENT	CHESTNUT I	HILL SQUARE PM
1. Langley Road at John Street			
John Street	WB L		
	WB R		
Langley Road	NB T		
	NB R		
Langley Road	SB L		
	SB T		
2. Langley Road at Driveway (north)			
Driveway	WB L		
	WB R		
Langley Road	NB T		
_	NB R		
Langley Road	SB L		
	SB T		
3. Langley Road at Driveway (south)			
Driveway	WB L		
	WB R		
Langley Road	NB T		
	NB R		
Langley Road	SB L		
	SB T		
4. Langley Road at Jackson Street			
Jackson Street	EB L		
	EB R		
Jackson Street	WB L		
	WB T		
	WB R		
Langley Road	SB T		
	SB R		
5. Langley Road at Route 9 (Boylston Street)			
Route 9	EB T	3	12
Route 9	WB T	10	6
Langley Road	SB L		
	SB T		
	SB R		



Trip Generation

ITE TRIP GENERATION WORKSHEET

(10th Edition, Updated 2017)

LANDUSE: Mid-Rise Residential LANDUSE CODE: 221

SETTING/LOCATION: General Urban/Suburban JOB NAME: Langley Road Redevelopment JOB NUMBER: 14019.00

Independent Variable --- Number of Units

20 units

WEEKDAY

RATES:			To	otal Trip End	ls	Independ	dent Variabl	e Range	Direc Distrib	
	# Studies	R^2	Average	Low	High	Average	Low	High	Enter	Exit
DAILY	27	0.77	5.44	1.27	12.50	205	21	494	50%	50%
AM PEAK OF GENERATOR	48	0.69	0.32	0.06	0.77	225	21	1,168	27%	73%
PM PEAK OF GENERATOR	47	0.66	0.41	0.09	1.26	211	21	1,168	60%	40%
AM PEAK (ADJACENT ST)	53	0.67	0.36	0.06	1.61	207	26	703	26%	74%
PM PEAK (ADJACENT ST)	60	0.72	0.44	0.15	1.11	208	26	703	61%	39%

TRIPS:

DAILY AM PEAK (ADJACENT ST) PM PEAK (ADJACENT ST)

BY AVERAGE						
Total	Enter	Exit				
109	54	54				
7	2	5				
9	5	3				

BY REGRESSION						
Total	Enter	Exit				
107	54	54				
7	2	5				
9	6	4				

SATURDAY

RATES:

			To	otal Trip End	ls
	# Studies	R^2	Average	Low	High
DAILY	6	0.73	4.91	4.03	8.51
PEAK OF GENERATOR	8	0.89	0.44	0.34	0.73

Independent Variable Range							
Average	Low	High					
224	111	336					
264	111	462					

Directional Distribution Enter Exit 50% 50% 49% 51%

TRIPS:

DAILY
PEAK OF GENERATOR

BY AVERAGE						
Total	Enter	Exit				
98	49	49				
9	4	4				

BY	REGRESSIO	ON
Total	Enter	Exit
478	239	239
15	7	8

SUNDAY

RATES:

			To	otal Trip End	ls
	# Studies	R^2	Average	Low	High
DAILY	6		4.09	3.06	8.41
PEAK OF GENERATOR	6		0.39	0.26	1.07

	Indepen	dent Variable	e Range
	Average	Low	High
-	224	111	336
	224	111	336

Directional Distribution Enter Exit 50% 50% 62% 38%

TRIPS:

DAILY
PEAK OF GENERATOR

	BY AVERAGE		
Total	Enter	Exit	
82	41	41	
Q	5	3	

В	Y REGRESSIC	ON
Total	Enter	Exit
N/A	N/A	N/A
NΔ	NΔ	NΔ



Trip Distribution

Trip Distribution

	Workplace						Tota	l of Residence				Cumulativ	ve Total	
						Route 9		Langley Road					Langley Road	
State/U.S. Island Area/Foreign	1			Percent of	Cumulative	(to/from	Route 9	(to/from			Route 9	Route 9	(to/from	
Country	County	MCD	Count	Total	Total	west)	(to/from east)	north)	Total	CHECK	(to/from west)	(to/from east)	north)	Total
Massachusetts	Suffolk County	Boston city	11,815	35.9%	35.9%		50%	50%	100%		0.0%	18.0%	18.0%	35.9%
Massachusetts	Middlesex County	Newton city	10,155	30.9%	66.8%	45%	5%	50%	100%		13.9%	1.5%	15.4%	30.9%
Massachusetts	Middlesex County	Cambridge city	2,475	7.5%	74.3%			100%	100%		0.0%	0.0%	7.5%	7.5%
Massachusetts	Middlesex County	Waltham city	1,750	5.3%	79.6%	50%		50%	100%		2.7%	0.0%	2.7%	5.3%
Massachusetts	Norfolk County	Brookline town	940	2.9%	82.5%		100%		100%		0.0%	2.9%	0.0%	2.9%
Massachusetts	Middlesex County	Watertown Town city	820	2.5%	85.0%			100%	100%		0.0%	0.0%	2.5%	2.5%
Massachusetts	Norfolk County	Needham town	820	2.5%	87.5%	100%			100%		2.5%	0.0%	0.0%	2.5%
Massachusetts	Middlesex County	Framingham town	795	2.4%	89.9%	75%		25%	100%		1.8%	0.0%	0.6%	2.4%
Massachusetts	Norfolk County	Wellesley town	730	2.2%	92.1%	100%			100%		2.2%	0.0%	0.0%	2.2%
Massachusetts	Middlesex County	Burlington town	435	1.3%	93.4%	100%			100%		1.3%	0.0%	0.0%	1.3%
Massachusetts	Middlesex County	Natick town	375	1.1%	94.6%	100%			100%		1.1%	0.0%	0.0%	1.1%
Massachusetts	Middlesex County	Lexington town	355	1.1%	95.6%	50%		50%	100%		0.5%	0.0%	0.5%	1.1%
Massachusetts	Middlesex County	Woburn city	355	1.1%	96.7%	75%		25%	100%		0.8%	0.0%	0.3%	1.1%
Massachusetts	Norfolk County	Canton town	310	0.9%	97.7%	100%			100%		0.9%	0.0%	0.0%	0.9%
Massachusetts	Norfolk County	Quincy city	270	0.8%	98.5%	75%		25%	100%		0.6%	0.0%	0.2%	0.8%
Massachusetts	Middlesex County	Malden city	255	0.8%	99.3%			100%	100%		0.0%	0.0%	0.8%	0.8%
Massachusetts	Norfolk County	Norwood town	245	0.7%	100.0%	100%			100%		0.7%	0.0%	0.0%	0.7%
Totals			32,900	100.0%							29.2%	22.4%	48.5%	100.0%
						•	•		•		29.0%	22.0%	49.0%	100.0%



Intersection Capacity Analyses

	۶	→	•	•	←	•	1	†	/	>	ţ	4		
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Ø2	
ane Configurations		ተተ _ጉ			ተተተ					ሻ	4	7		
Fraffic Volume (vph)	0	1555	0	0	1665	0	0	0	0	250	5	80		
uture Volume (vph)	0	1555	0	0	1665	0	0	0	0	250	5	80		
deal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	5136	0	0	5136	0	0	0	0	1698	1706	1599		
Flt Permitted		0100			0100					0.950	0.954	1000		
Satd. Flow (perm)	0	5136	0	0	5136	0	0	0	0	1698	1706	1599		
Right Turn on Red	U	0100	Yes		0100	Yes	, ,		Yes	1000	1700	Yes		
Satd. Flow (RTOR)			100			100			100			94		
Link Speed (mph)		30			30			30			30	J-T		
Link Distance (ft)		760			318			215			145			
Fravel Time (s)		17.3			7.2			4.9			3.3			
\ /		17.3	1	1	1.2			4.3			3.3	5		
Confl. Peds. (#/hr)	0.96	0.96	0.96	0.98	0.98	0.98	0.92	0.92	0.92	0.85	0.85	0.85		
Peak Hour Factor	0.96	1%	0.96	0.98	1%	0.98	0.92	0.92	0.92	1%	0.85	1%		
Heavy Vehicles (%)	U%	170	U%	U%	170	U%	U%	U%	U%	49%	U%	1 70		
Shared Lane Traffic (%)	0	1600	0	^	1600	0	0	0	0		450	94		
ane Group Flow (vph)	0	1620	U	0	1699 NA	U	U	U	U	150	150			
Turn Type		NA								Split	NA	Prot	_	
Protected Phases		1			1					3	3	3	2	
Permitted Phases					,									
Detector Phase		1			1					3	3	3		
Switch Phase		4.0			4.0					4.0	4.0	4.0	4.0	
Minimum Initial (s)		4.0			4.0					4.0	4.0	4.0	4.0	
Minimum Split (s)		9.0			9.0					9.0	9.0	9.0	20.0	
Total Split (s)		30.0			30.0					30.0	30.0	30.0	20.0	
Total Split (%)		37.5%			37.5%					37.5%	37.5%	37.5%	25%	
Yellow Time (s)		4.0			4.0					4.0	4.0	4.0	2.0	
All-Red Time (s)		1.0			1.0					1.0	1.0	1.0	0.0	
_ost Time Adjust (s)		0.0			0.0					0.0	0.0	0.0		
Total Lost Time (s)		5.0			5.0					5.0	5.0	5.0		
_ead/Lag		Lead			Lead								Lag	
_ead-Lag Optimize?														
Recall Mode		Max			Max					None	None	None	None	
Act Effct Green (s)		26.0			26.0					9.8	9.8	9.8		
Actuated g/C Ratio		0.53			0.53					0.20	0.20	0.20		
//c Ratio		0.59			0.62					0.44	0.44	0.24		
Control Delay		11.8			12.3					23.1	23.0	7.2		
Queue Delay		0.0			0.0					0.0	0.0	0.0		
Total Delay		11.8			12.3					23.1	23.0	7.2		
_OS		В			В					С	С	Α		
Approach Delay		11.8			12.3						19.3			
Approach LOS		В			В						В			
Queue Length 50th (ft)		79			84					34	34	0		
Queue Length 95th (ft)		#330			#357					105	105	30		
nternal Link Dist (ft)		680			238			135			65			
Γurn Bay Length (ft)														
Base Capacity (vph)		2728			2728					902	906	893		
Starvation Cap Reductn		0			0					0	0	0		
Spillback Cap Reductn		0			0					0	0	0		
Storage Cap Reductn		0			0					0	0	0		
Reduced v/c Ratio		0.59			0.62					0.17	0.17	0.11		

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 48.9

Natural Cycle: 60

Control Type: Semi Act-Uncoord

Maximum v/c Ratio: 0.62

Intersection Signal Delay: 12.8

Intersection Capacity Utilization 47.6%

Intersection LOS: B ICU Level of Service A

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



Intersection						
Int Delay, s/veh	0.2					
		WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		^}	•		4
Traffic Vol, veh/h	5	1	245	2	1	275
Future Vol, veh/h	5	1	245	2	1	275
Conflicting Peds, #/hr	0	0	0	7	7	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	50	50	89	89	85	85
Heavy Vehicles, %	0	0	1	0	0	1
Mymt Flow	10	2	275	2	1	324
WWIII FIOW	10	2	213	2		324
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	609	283	0	0	285	0
Stage 1	283	_	_	_	-	-
Stage 2	326	_	_	-	_	
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	0.2	-	-	-4.1	-
	5.4	-	-			-
Critical Hdwy Stg 2				-		_
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	462	761	-	-	1289	-
Stage 1	770	-	-	-	-	-
Stage 2	736	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	458	756	-	-	1289	-
Mov Cap-2 Maneuver	458	-	-	-	-	-
Stage 1	765	_	_	_	_	_
Stage 2	735	_	_		_	_
Olage 2	700					
Approach	WB		NB		SB	
HCM Control Delay, s	12.5		0		0	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)		-	-	490	1289	-
HCM Lane V/C Ratio		-	-	0.024	0.001	-
HCM Control Delay (s)		-	-	12.5	7.8	0
HCM Lane LOS		-	-	В	A	A
HCM 95th %tile Q(veh)		_	_	0.1	0	-
HOW Jour Joune Q(Veri)		_		0.1	U	

Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
	WBL	WBR		NBK	SBL	<u>≥81</u>
Lane Configurations		0		1	1	280
Traffic Vol, veh/h Future Vol, veh/h	5 5	0	245 245	1	1	280
	0	0	245 0	8	8	280
Conflicting Peds, #/hr	-	-	_	-	Free	-
Sign Control RT Channelized	Stop	Stop	Free	Free None		Free
	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	33	33	89	89	87	87
Heavy Vehicles, %	0	0	1	0	0	1
Mvmt Flow	15	0	275	1	1	322
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	608	284	0	0	284	0
	284	204	-	-	204	-
Stage 1						
Stage 2	324	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	462	760	-	-	1290	-
Stage 1	769	-	-	-	-	-
Stage 2	738	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	458	754	-	-	1290	-
Mov Cap-2 Maneuver	458	-	-	-	-	-
Stage 1	763	-	-	-	-	-
Stage 2	737	-	-	-	-	-
Tings =						
	1105					
Approach	WB		NB		SB	
HCM Control Delay, s	13.1		0		0	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT	NIRD	WBLn1	SBL	SBT
				458	1290	
Capacity (veh/h)		-	-	0.033	0.001	-
		-	-			-
HCM Cantrol Palay (a)						
HCM Control Delay (s)		-	-	13.1	7.8	0
		-	-	13.1 B 0.1	7.8 A 0	0 A

•						
Intersection						
Int Delay, s/veh	0.1					
	MDI	MDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		^}		•	4
Traffic Vol, veh/h	2	1	245	1	2	285
Future Vol, veh/h	2	1	245	1	2	285
Conflicting Peds, #/hr	0	0	0	8	8	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0		0	-		0
Peak Hour Factor	75	75	89	89	86	86
Heavy Vehicles, %	0	0	1	0	0	1
Mymt Flow	3	1	275	1	2	331
WWIII FIOW	J	ļ.	213	ı	2	331
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	620	284	0	0	284	0
Stage 1	284		-	_		-
Stage 2	336	-	_	-	_	_
Critical Hdwy	6.4	6.2	_	_	4.1	-
Critical Hdwy Stg 1	5.4	0.2	-	-	4.1	-
	5.4					
Critical Hdwy Stg 2		-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	455	760	-	-	1290	-
Stage 1	769	-	-	-	-	-
Stage 2	728	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	451	754	-	-	1290	-
Mov Cap-2 Maneuver	451	-	-	-	-	-
Stage 1	763	_	_	_	_	-
Stage 2	727	-	_			_
Glage 2	121					
Approach	WB		NB		SB	
HCM Control Delay, s	12		0		0.1	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)		-	-	521	1290	-
HCM Lane V/C Ratio		-	-	0.008	0.002	-
HCM Control Delay (s)		-	-	12	7.8	0
HCM Lane LOS		-	-	В	A	Ä
HCM 95th %tile Q(veh)				0	0	-
HOW JOHN JOHN (VOII)		-	-	U	U	-

Intersection
Int Delay, s/veh 6.3
Movement EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SB
Lane Configurations 4
Traffic Vol. veh/h 50 0 40 40 25 195 0 0 0 255 3
Future Vol. veh/h 50 0 40 40 25 195 0 0 0 255 3
Conflicting Peds, #/hr 3 0 0 0 0 3 0 0 0 0
Sign Control Stop Stop Stop Stop Stop Free Free Free Free Free Free Free Fre
RT Channelized None None None None
Storage Length
Veh in Median Storage, # - 0 0 0
Grade. % - 0 0 0
Peak Hour Factor 91 91 91 91 91 91 92 92 92 86 86 8
Heavy Vehicles, % 0 0 0 2 0 1 0 0 0 1
Wynt Flow 55 0 44 44 27 214 0 0 0 0 297 3
00 0 11 11 21 211 0 0 0 201 0
Major/Minor Minor2 Minor1 Major2
,
Stage 2 124 0 - 336 335 Critical Hdwy 7.1 6.5 6.2 7.12 6.5 6.21
Critical Howy 7.1 6.5 6.2 7.12 6.5 6.21
Critical Hdwy Stg 2 - - - 6.12 5.5 - - - Follow-up Hdwy 3.5 4 3.3 3.518 4 3.309 - -
0169 2 010 010 010 010 010 010 010 010 010 0
**** *** ***
Stage 1 698 654
Stage 2 637 644
Approach EB WB SB
HCM Control Delay, s 13.9 10.9 0
HCM LOS B B
Minor Lane/Major Mvmt EBLn1 WBLn1 SBT SBR
Capacity (veh/h) 504 891
HCM Lane V/C Ratio 0.196 0.321
HCM Control Delay (s) 13.9 10.9
HOME LOO
HCM Lane LOS B B HCM 95th %tile Q(yeh) 0.7 1.4

	۶	→	•	1	+	4	•	†	/	/	 	4		
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Ø2	
Lane Configurations		ተተኈ			ተተተ					*	4	1		
Traffic Volume (vph)	0	1285	0	0	1700	0	0	0	0	275	2	110		
Future Volume (vph)	0	1285	0	0	1700	0	0	0	0	275	2	110		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	5136	0	0	5136	0	0	0	0	1715	1720	1599		
Flt Permitted	U	3130	U	U	3130	U	U	U	U	0.950	0.953	1000		
Satd. Flow (perm)	0	5136	0	0	5136	0	0	0	0	1715	1720	1599		
Right Turn on Red	U	3130	Yes	U	3130	Yes	U	U	Yes	1713	1720	Yes		
Satd. Flow (RTOR)			165			165			165			121		
Link Speed (mph)		30			30			30			30	121		
1 (1)		760			318			215			145			
Link Distance (ft)								4.9						
Travel Time (s)		17.3	4		7.2			4.9			3.3	4		
Confl. Peds. (#/hr)	0.00	0.00	1	1	0.00	0.00	0.00	0.00	0.00	0.04	0.04	4		
Peak Hour Factor	0.98	0.98	0.98	0.99	0.99	0.99	0.92	0.92	0.92	0.91	0.91	0.91		
Heavy Vehicles (%)	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	1%		
Shared Lane Traffic (%)		1011	•	_					_	50%	450	404		
Lane Group Flow (vph)	0	1311	0	0	1717	0	0	0	0	151	153	121		
Turn Type		NA			NA					Split	NA	Prot		
Protected Phases		1			1					3	3	3	2	
Permitted Phases										_	_	_		
Detector Phase		1			1					3	3	3		
Switch Phase														
Minimum Initial (s)		4.0			4.0					4.0	4.0	4.0	4.0	
Minimum Split (s)		9.0			9.0					9.0	9.0	9.0	20.0	
Total Split (s)		30.0			30.0					30.0	30.0	30.0	20.0	
Total Split (%)		37.5%			37.5%					37.5%	37.5%	37.5%	25%	
Yellow Time (s)		4.0			4.0					4.0	4.0	4.0	2.0	
All-Red Time (s)		1.0			1.0					1.0	1.0	1.0	0.0	
Lost Time Adjust (s)		0.0			0.0					0.0	0.0	0.0		
Total Lost Time (s)		5.0			5.0					5.0	5.0	5.0		
Lead/Lag		Lead			Lead								Lag	
Lead-Lag Optimize?														
Recall Mode		Max			Max					None	None	None	None	
Act Effct Green (s)		26.0			26.0					9.9	9.9	9.9		
Actuated g/C Ratio		0.53			0.53					0.20	0.20	0.20		
v/c Ratio		0.48			0.63					0.44	0.44	0.29		
Control Delay		10.3			12.7					22.8	22.8	6.9		
Queue Delay		0.0			0.0					0.0	0.0	0.0		
Total Delay		10.3			12.7					22.8	22.8	6.9		
LOS		В			В					С	С	Α		
Approach Delay		10.3			12.7						18.3			
Approach LOS		В			В						В			
Queue Length 50th (ft)		58			86					34	35	0		
Queue Length 95th (ft)		231			#372					112	113	38		
Internal Link Dist (ft)		680			238			135			65			
Turn Bay Length (ft)														
Base Capacity (vph)		2723			2723					909	912	904		
Starvation Cap Reductn		0			0					0	0	0		
Spillback Cap Reductn		0			0					0	0	0		
Storage Cap Reductn		0			0					0	0	0		
Reduced v/c Ratio		0.48			0.63					0.17	0.17	0.13		

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 49.1

Natural Cycle: 60

Control Type: Semi Act-Uncoord

Maximum v/c Ratio: 0.63

Intersection Signal Delay: 12.4

Intersection Capacity Utilization 48.8%

Intersection LOS: B
ICU Level of Service A

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



tersection t Delay, s/veh 0.5 ovement WBL WBR NBT NBR SBL SBT ane Configurations Y Image: SBI SBI SBI SBI SBI SBI SBI SBI SBI SBI
overment WBL WBR NBT NBR SBL SBT ane Configurations Y Image: Configuration of the processing of the proce
ane Configurations raffic Vol, veh/h 5 1 195 5 2 290 uture Vol, veh/h 5 1 195 5 2 290 onflicting Peds, #/hr 1 0 0 3 3 3 0 gn Control Stop Stop Free Free Free Free T Channelized - None - None - None torage Length 0 - 0 0 rade, % 0 - 0 0 rade, % 0 - 0 - 0 - 0 eak Hour Factor 33 33 91 91 94 94 eavy Vehicles, % 0 0 0 0 0 0 0 vmt Flow 15 3 214 5 2 309 ajor/Minor Minor Major Major2 onflicting Flow All 534 220 0 0 223 0 Stage 1 220 Stage 2 314
raffic Vol, veh/h 5 1 195 5 2 290 uture Vol, veh/h 5 1 195 5 2 290 onflicting Peds, #/hr 1 0 0 3 3 3 0 gn Control Stop Stop Free Free Free Free T Channelized - None - None - None torage Length 0 - 0 0 rade, % 0 - 0 - 0 - 0 rade, % 0 - 0 - 0 0 - 0 rade, % 0 0 - 0 0 0 0 0 rade, % 0 0 0 0 0 0 0 orade, % 0 0 0 0 0 0 0 orade, % 15 3 214 5 2 309 alor/Minor Minor Major Major 0 Stage 1 220 0 0 223 0 Stage 1 220 Stage 2 314
auture Vol, veh/h 5 1 195 5 2 290 conflicting Peds, #/hr 1 0 0 3 3 0 regn Control Stop Stop Free
onflicting Peds, #/hr 1 0 0 3 3 0 Ign Control Stop Stop Free
gn Control Stop Stop Free Free Free Free Free T Channelized - None - None - None torage Length 0 - 0 - 0 - 0 - 0 - 0 Arade, % 0 - 0 - 0 - 0 - 0 Arade, % 0 - 0 0 - 0 0 0 0 0 0 0 0 0 0 0 0 0 0
T Channelized - None - None - None torage Length 0 - 0 0 0 rade, % 0 - 0 - 0 - 0 - 0 rade, % 0 0 - 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
torage Length 0
eh in Median Storage, # 0 - 0 0 rade, % 0 - 0 0 eak Hour Factor 33 33 91 91 94 94 eavy Vehicles, % 0 0 0 0 0 0 0 vmt Flow 15 3 214 5 2 309 ajor/Minor Minor1 Major1 Major2 onflicting Flow All 534 220 0 0 223 0 Stage 1 220 Stage 2 314
rade, % 0 - 0 - 0 0 - 0 0 0 0 0 0 0 0 0 0 0 0
eak Hour Factor 33 33 91 91 94 94 94 eavy Vehicles, % 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
eavy Vehicles, % 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
vmt Flow 15 3 214 5 2 309 ajor/Minor Minor1 Major1 Major2 onflicting Flow All 534 220 0 0 223 0 Stage 1 220 - - - - - - Stage 2 314 - - - - - -
ajor/Minor Minor1 Major1 Major2 onflicting Flow All 534 220 0 0 223 0 Stage 1 220 - - - - - - - Stage 2 314 - - - - - -
onflicting Flow All 534 220 0 0 223 0 Stage 1 220 - - - - - - Stage 2 314 - - - - - -
onflicting Flow All 534 220 0 0 223 0 Stage 1 220 - - - - - - Stage 2 314 - - - - - -
onflicting Flow All 534 220 0 0 223 0 Stage 1 220 - - - - - - Stage 2 314 - - - - - -
Stage 1 220 -
Stage 2 314
ritical Hdwy 6.4 6.2 4.1 -
ritical Hdwy Stg 1 5.4
ritical Hdwy Stg 2 5.4
ollow-up Hdwy 3.5 3.3 2.2 -
ot Cap-1 Maneuver 510 825 1358 -
Stage 1 821
Stage 2 745
atoon blocked. %
ov Cap-1 Maneuver 507 823 1358 -
ov Cap-2 Maneuver 507
Stage 1 819
Stage 2 743
Stage 2 745
pproach WB NB SB
CM Control Delay, s 11.9 0 0.1
CM LOS B
inout and Major Muset NDT NDD WDL #4 CDL CDT
inor Lane/Major Mvmt NBT NBR WBLn1 SBL SBT
apacity (veh/h) 542 1358 -
apacity (veh/h) 542 1358 - CM Lane V/C Ratio 0.034 0.002 -
apacity (veh/h) 542 1358 - CM Lane V/C Ratio 0.034 0.002 - CM Control Delay (s) 11.9 7.7 0
apacity (veh/h) 542 1358 - CM Lane V/C Ratio 0.034 0.002 -

Intersection						
Int Delay, s/veh	0.4					
• •	WDI	WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Ä	-	105	_	0	4 295
Traffic Vol, veh/h	5	5	195	5	2	
Future Vol, veh/h	5	5	195	5	2	295
Conflicting Peds, #/hr	0	0	_ 0	_ 2	_ 2	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	63	63	94	94	94	94
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	8	8	207	5	2	314
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	530	212	0	0	215	0
Stage 1	212	-	-	-	210	-
Stage 2	318	_	-	-	_	_
Critical Hdwy	6.4	6.2	_	_	4.1	_
Critical Hdwy Stg 1	5.4	- 0.2	-	-	-	_
Critical Hdwy Stg 2	5.4		-			
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	5.5 513	833	-		1367	-
Stage 1	828	-	-	-	1307	-
Stage 2	o∠o 742	-	-	-	-	-
	142	-	-		-	-
Platoon blocked, %	E14	024		-	1267	-
Mov Cap-1 Maneuver	511	831	-	-	1367	-
Mov Cap-2 Maneuver	511	-	-	-	-	-
Stage 1	826	-	-	-	-	-
Stage 2	741	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	10.8		0		0.1	
HCM LOS	В				U. .	
		NET	NDD	11/01 4	ODI	007
Minor Lane/Major Mvmt		NBT		WBLn1	SBL	SBT
Capacity (veh/h)		-	-	633	1367	-
HCM Lane V/C Ratio		-	-	0.025	0.002	-
HCM Control Delay (s)		-	-	10.8	7.6	0
HCM Lane LOS		-	-	В	Α	Α
HCM 95th %tile Q(veh)		-	-	0.1	0	-

Intersection						
Intersection Int Delay, s/veh	0.1					

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y 4		eĵ.			4
Traffic Vol, veh/h	0	1	200	0	0	300
Future Vol, veh/h	0	1	200	0	0	300
Conflicting Peds, #/hr	0	0	0	2	2	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	25	25	93	93	95	95
Heavy Vehicles, %	0	0	0	0	0	0
Mymt Flow	0	4	215	0	0	316
WWIIICH IOW	0	7	210	U	U	310
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	533	217	0	0	217	0
Stage 1	217	-	-	-	-	-
Stage 2	316	-	-	-	-	-
Critical Hdwy	6.4	6.2	_	-	4.1	-
Critical Hdwy Stg 1	5.4	-	_	_	-	_
Critical Hdwy Stg 2	5.4	_	_	-	_	_
Follow-up Hdwy	3.5	3.3	_	-	2.2	_
Pot Cap-1 Maneuver	511	828	_	_	1365	_
Stage 1	824	- 020	_	_	1000	_
Stage 2	744	_				_
Platoon blocked, %	144	-	-	-	-	
	510	826			4005	
Mov Cap-1 Maneuver			-	-	1365	-
Mov Cap-2 Maneuver	510	-	-	-	-	-
Stage 1	822	-	-	-	-	-
Stage 2	744	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9.4		0		0	
HCM LOS	9.4 A		U		U	
TIOWI LUG	A					
Minor Lane/Major Mvmt		NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)		-	-	826	1365	-
HCM Lane V/C Ratio		-	-	0.005	-	-
HCM Control Delay (s)		-	-	9.4	0	-
HCM Lane LOS		-	_	Α	A	-
HCM 95th %tile Q(veh)		-	-	0	0	-
3041 /0410 (4 1011)				U	J	

Intersection												
Int Delay, s/veh	6.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4						\$	
Traffic Vol. veh/h	40	0	55	65	50	160	0	0	0	0	265	35
Future Vol, veh/h	40	0	55	65	50	160	0	0	0	0	265	35
Conflicting Peds, #/hr	2	0	0	0	0	2	0	0	0	0	0	3
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	_	_	-	-	_	-	-	-	-	-	_	-
Veh in Median Storage, #	_	0	_	_	0	_	_	_	_	_	0	_
Grade, %	_	0	-	-	0	_	-	0	_	-	0	_
Peak Hour Factor	92	92	92	93	93	93	92	92	92	94	94	94
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mymt Flow	43	0	60	70	54	172	0	0	0	0	282	37
												.
Major/Minor	Minor2			Minor1						Major2		
Conflicting Flow All	419	304	304	330	322	2				iviajuiz -		0
Stage 1	304	304	-	0	0					-	-	U
Stage 2	115	0	-	330	322	-				-	-	
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2				-	-	
Critical Hdwy Stg 1	6.1	5.5	- 0.2	7.1	- 0.5	0.2				-	-	-
Critical Hdwy Stg 2	0.1	3.3		6.1	5.5					-		
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3				_	_	-
Pot Cap-1 Maneuver	548	613	740	627	599	1088				0	_	_
Stage 1	710	667	-	-	-	-				0	_	_
Stage 2	-	-	_	687	655	_				0	_	_
Platoon blocked. %				001	000						_	-
Mov Cap-1 Maneuver	427	611	738	576	597	1086				_	_	-
Mov Cap-2 Maneuver	427	611	-	576	597	-				_	_	-
Stage 1	710	665	_	-	-	-				-	-	-
Stage 2	-	-	-	631	653	-				-	-	-
J					000							
Approach	EB			WB						SB		
HCM Control Delay, s	12.8			12.1						0		
HCM LOS	12.0 B			В								
Minor Lane/Major Mvmt		EBLn1	WBLn1	SBT	SBR							
Capacity (veh/h)		565	800	-	-							
HCM Lane V/C Ratio		0.183	0.37	_	-							
HCM Control Delay (s)		12.8	12.1	_	_							
HCM Lane LOS		12.0 B	В	-	-							
HCM 95th %tile Q(veh)		0.7	1.7	_	_							
110111 00111 /01110 ((1011)		0.1	1.7									

Act Effet Green (s) 26.0 26.0 9.8 9.8 9.8 Actuated g/C Ratio 0.53 0.53 0.20 0.20 0.20 v/c Ratio 0.67 0.72 0.45 0.45 0.23 Control Delay 13.2 14.1 23.2 23.1 7.2 Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 13.2 14.1 23.2 23.1 7.2 LOS B B C C A Approach Delay 13.2 14.1 19.4 Approach LOS B B B B Queue Length 50th (ft) 94 105 35 35 35 0 Queue Length 95th (ft) #400 #447 113 113 113 34 Internal Link Dist (ft) 680 238 135 65 5 Turn Bay Length (ft) 8 892 892 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0		•	→	•	•	+	•	•	†	<u> </u>	\	 	4		
Lane Configurations	Lane Group	FBI	FBT	FBR	WBI	WRT	WBR	NBI	NBT	NBR	SBI	SBT	SBR	Ø2	
Traffic Volume (uph) 0 1670 0 0 1795 0 0 0 0 0 275 5 85							11211	.,,,,,						~_	
Fulture Volume (vph)		0	1670	Λ	Λ		٥	0	٥	٥		~			
Islael Flow (polphi) 1900															
Sald. Flow (prof) Flow (prof) Flow (prof) Flow (prof) Sald. Flow (() /														
File Permitted															
Sald, Flow (perm)		U	3130	U	U	3130	U	U	U	U			1599		
Right Tum on Rad		0	5126	٥	۸	5126	٥	٥	٥	٥			1500		
Said. Flow (RTOR)		U	3130		U	3130		U	U		1090	1703			
Link Speader (mph)				163			163			163					
Link Distance (f) 760 318 215 145 Travel Time (s) 17.3 7.2 4.9 3.3 3 Confl. Peds. (s/hr) 1 1 1 3 3.3 5 Confl. Peds. (s/hr) 1 1 1 3 3.3 5 Confl. Peds. (s/hr) 1 1 1 3 3.3 5 Confl. Peds. (s/hr) 1 1 1 3 3.3 5 Confl. Peds. (s/hr) 1 5 0 0 192 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.			20			20			20			20	92		
Travel Time (s)	. (, ,														
Conf. Peds. (#fr/r)															
Peak Hour Factor	()		17.3	4	4	1.2			4.9			3.3	F		
Heavy Nehicles (%)		0.00	0.00	-		0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Shared Lane Traffic (%)															
Lane Group Flow (vph)		0%	1%	0%	0%	1%	0%	0%	0%	0%		0%	1%		
Turn Type NA NA NA Split NA Prot Protected Phases 1 1 1 3 3 3 3 2 Permitted Phases 0 1 1 1 3 3 3 3 3 2 Permitted Phases 0 1 1 1 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3		•	4045	_	_	4054	_	_	^	^		450	00		
Protected Phases 1 1 1 3 3 3 3 2 Permitted Phases Detector Phase 1 1 1 3 3 3 3 3 2 Permitted Phases Detector Phase 1 1 1 3 3 3 3 3 3 3 3 3 2 Permitted Phases		0		U	U		0	U	U	0					
Permitted Phases Detector Phase 1														_	
Detector Phase 1			1			1					3	3	3	2	
Switch Phase															
Minimum Initial (s) 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Mode of the process of t			1			1					3	3	3		
Minimum Split (s) 9.0 9.0 9.0 9.0 20.0 Total Split (s) 30.0 30.0 30.0 30.0 20.0 Total Split (%) 37.5% 37.5% 37.5% 37.5% 25% Yellow Time (s) 4.0 4.0 4.0 4.0 4.0 2.0 All-Red Time (s) 1.0 1.0 1.0 1.0 1.0 1.0 1.0 0															
Total Split (s) 30.0 30.0 30.0 30.0 30.0 20.0 Total Split (%) 37.5% 37.5% 37.5% 25% Yellow Time (s) 4.0 4.0 4.0 4.0 4.0 4.0 4.0 2.0 All-Red Time (s) 1.0 1.0 1.0 1.0 1.0 1.0 0.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.0 5.0 5.0 5.0 5.0 Lead/Lag Lead Lead Lead Lead-Lag Optimize? Recall Mode Max Max None None None None None Recall Mode Recall Mode Part (Recall Mode None None None None None None None Non															
Total Split (%) 37.5% 37.5% 37.5% 37.5% 37.5% 25% Yellow Time (s) 4.0 4.0 4.0 4.0 4.0 2.0 All-Red Time (s) 1.0 1.0 1.0 1.0 1.0 1.0 0.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.0 5.0 5.0 5.0 5.0 5.0 5.0 Lead/Lag Lead Lead Lead Lead Lead-Lag Lead-Lag Coptimize? Recall Mode Max Max None None None None Act Effet Green (s) 26.0 26.0 9.8 9.8 9.8 Actuated g/C Ratio 0.53 0.53 0.53 0.20 0.20 0.20 0.20 0.20 0.00 0.00 0.0															
Yellow Time (s) 4.0 4.0 4.0 4.0 4.0 2.0 All-Red Time (s) 1.0 1.0 1.0 1.0 1.0 0.0 Lost Time Agbiast (s) 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.0 5.0 5.0 5.0 5.0 Lead/Lag Quead (Lead (
All-Red Time (s) 1.0 1.0 1.0 1.0 1.0 1.0 1.0 0.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Lead/Lag															
Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 Lag Lead/Lag Optimize? Recall Mode Max Max None None <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>															
Total Lost Time (s) 5.0 5.0 5.0 5.0 5.0 5.0 Lead/Lag Lead Lea														0.0	
Lead/Lag Lead Lead Lag Lead-Lag Optimize? Volume 1800 None None None None Recall Mode Max Max None None None None Act Leffet Green (s) 26.0 26.0 9.8 9.0 9.2 9.3 9.3 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2 9.2															
Lead-Lag Optimize? Recall Mode Max Max None None None None Act Effet Green (s) 26.0 26.0 9.8 9.8 9.8 Actuated g/C Ratio 0.53 0.53 0.20 0.20 0.20 v/c Ratio 0.67 0.72 0.45 0.45 0.23 Control Delay 13.2 14.1 23.2 23.1 7.2 Queue Delay 0.0 0.0 0.0 0.0 Total Delay 13.2 14.1 23.2 23.1 7.2 LOS B B C C A Approach Delay 13.2 14.1 19.4 Approach LOS B B B Queue Length 50th (ft) 94 105 35 35 0 Queue Length 95th (ft) #400 #447 113 113 34 Internal Link Dist (ft) 680 238 135 65 5 Turn Bay Length (ft) 88 2727 901 905 892 Starvation Cap Reductn 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0											5.0	5.0	5.0		
Recall Mode Max Max Max None None None None Act Effct Green (s) 26.0 26.0 9.8 9.8 9.8 Actuated g/C Ratio 0.53 0.53 0.20 0.20 0.20 Vic Ratio 0.67 0.72 0.45 0.45 0.23 Control Delay 13.2 14.1 23.2 23.1 7.2 Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 13.2 14.1 23.2 23.1 7.2 LOS B B B C C A Approach Delay 13.2 14.1 19.4 4			Lead			Lead								Lag	
Act Effet Green (s) 26.0 26.0 9.8 9.8 9.8 Actuated g/C Ratio 0.53 0.53 0.20 0.20 0.20 v/c Ratio 0.67 0.72 0.45 0.45 0.23 Control Delay 13.2 14.1 23.2 23.1 7.2 Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 13.2 14.1 23.2 23.1 7.2 LOS B B C C A Approach Delay 13.2 14.1 19.4 Approach LOS B B B B Queue Length 50th (ft) 94 105 35 35 35 0 Queue Length 95th (ft) #400 #447 113 113 113 34 Internal Link Dist (ft) 680 238 135 65 5 Turn Bay Length (ft) 8 892 892 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0															
Actuated g/C Ratio 0.53 0.53 0.20 0.20 0.20 v/c Ratio 0.67 0.72 0.45 0.45 0.23 Control Delay 13.2 14.1 23.2 23.1 7.2 Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 13.2 14.1 23.2 23.1 7.2 LOS B B C C A Approach Delay 13.2 14.1 19.4 Approach LOS B B B Queue Length 50th (ft) 94 105 35 35 0 Queue Length 95th (ft) #440 #447 113 113 34 Internal Link Dist (ft) 680 238 135 65 Turn Bay Length (ft) Base Capacity (vph) 2727 2727 901 905 892 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0	Recall Mode													None	
v/c Ratio 0.67 0.72 0.45 0.45 0.23 Control Delay 13.2 14.1 23.2 23.1 7.2 Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 13.2 14.1 23.2 23.1 7.2 LOS B B C C A Approach Delay 13.2 14.1 19.4 Approach LOS B B B Queue Length 50th (ft) 94 105 35 35 0 Queue Length 95th (ft) #440 #447 113 113 34 Internal Link Dist (ft) 680 238 135 65 Turn Bay Length (ft) 88 135 65 Turn Bay Length (ft) 88 92 892 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0	Act Effct Green (s)														
Control Delay 13.2 14.1 23.2 23.1 7.2 Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 13.2 14.1 23.2 23.1 7.2 LOS B B C C A Approach Delay 13.2 14.1 19.4 Approach LOS B B B Queue Length 50th (ft) 94 105 35 35 0 Queue Length 95th (ft) #440 #447 113 113 34 Internal Link Dist (ft) 680 238 135 65 Turn Bay Length (ft) 680 238 135 65 Turn Bay Length (ft) 8ase Capacity (vph) 2727 901 905 892 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0	Actuated g/C Ratio		0.53			0.53					0.20	0.20	0.20		
Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 13.2 14.1 23.2 23.1 7.2 LOS B B C C A Approach Delay 13.2 14.1 19.4 Approach LOS B B B Queue Length 50th (ft) 94 105 35 35 0 Queue Length 95th (ft) #440 #447 113 113 34 Internal Link Dist (ft) 680 238 135 65 5 Turn Bay Length (ft) Base Capacity (vph) 2727 901 905 892 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0	v/c Ratio		0.67			0.72					0.45	0.45	0.23		
Total Delay 13.2 14.1 23.2 23.1 7.2 LOS B B C C A Approach Delay 13.2 14.1 19.4 Approach LOS B B B B Queue Length 50th (ft) 94 105 35 35 0 Queue Length 95th (ft) #440 #447 113 113 34 Internal Link Dist (ft) 680 238 135 65 5 Turn Bay Length (ft) 8ase Capacity (vph) 2727 901 905 892 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0 0 0 0	Control Delay		13.2			14.1					23.2	23.1	7.2		
LOS B B C C A Approach Delay 13.2 14.1 19.4 Approach LOS B B B Queue Length 50th (ft) 94 105 35 35 0 Queue Length 95th (ft) #400 #447 113 113 34 Internal Link Dist (ft) 680 238 135 65 Turn Bay Length (ft) Base Capacity (vph) 2727 901 905 892 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0	Queue Delay		0.0			0.0					0.0	0.0	0.0		
Approach Delay 13.2 14.1 19.4 Approach LOS B B B Queue Length 50th (ft) 94 105 35 35 0 Queue Length 95th (ft) #400 #447 113 113 34 Internal Link Dist (ft) 680 238 135 65 Turn Bay Length (ft) Base Capacity (vph) 2727 2727 901 905 892 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0	Total Delay		13.2			14.1					23.2	23.1	7.2		
Approach LOS B B Queue Length 50th (ft) 94 105 35 35 0 Queue Length 95th (ft) #400 #447 113 113 34 Internal Link Dist (ft) 680 238 135 65 Turn Bay Length (ft) Base Capacity (vph) 2727 2727 901 905 892 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0	LOS		В			В					С	С	Α		
Approach LOS B B Queue Length 50th (ft) 94 105 35 35 0 Queue Length 95th (ft) #400 #447 113 113 34 Internal Link Dist (ft) 680 238 135 65 Turn Bay Length (ft) Base Capacity (vph) 2727 2727 901 905 892 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0	Approach Delay		13.2			14.1						19.4			
Queue Length 95th (ff) #400 #447 113 113 34 Internal Link Dist (ft) 680 238 135 65 Turn Bay Length (ft) Base Capacity (vph) 2727 2727 901 905 892 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0	Approach LOS					В						В			
Queue Length 95th (ft) #400 #447 113 113 34 Internal Link Dist (ft) 680 238 135 65 Turn Bay Length (ft) Base Capacity (vph) 2727 2727 901 905 892 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0	Queue Length 50th (ft)		94			105					35	35	0		
Internal Link Dist (ft) 680 238 135 65 Turn Bay Length (ft) Base Capacity (vph) 2727 2727 901 905 892 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0			#400								113		34		
Turn Bay Length (ft) Base Capacity (vph) 2727 2727 901 905 892 Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0 Storage Cap Reductn 0 0 0 0	Internal Link Dist (ft)								135						
Base Capacity (vph) 2727 2727 901 905 892 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0	()														
Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0 0			2727			2727					901	905	892		
Spillback Cap Reductn 0 0 0 0 0 Storage Cap Reductn 0 0 0 0 0															
Storage Cap Reductn 0 0 0 0						-					-	-	-		
	Reduced v/c Ratio		0.67			0.72					0.17	0.17	0.10		

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 48.9

Natural Cycle: 60

Control Type: Semi Act-Uncoord

Maximum v/c Ratio: 0.72

Intersection Signal Delay: 14.2

Intersection Capacity Utilization 50.8%

Intersection LOS: B
ICU Level of Service A

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



Intersection						
Int Delay, s/veh	0.2					
		WDD	NDT	NDD	ODI	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		1			स्
Traffic Vol, veh/h	5	1	265	2	1	295
Future Vol, veh/h	5	1	265	2	1	295
Conflicting Peds, #/hr	0	0	0	7	7	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	50	50	89	89	85	85
Heavy Vehicles, %	0	0	1	0	0	1
Mymt Flow	10	2	298	2	1	347
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	655	306	0	0	307	0
Stage 1	306	-	-	-	-	-
Stage 2	349	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	434	739	-	-	1265	-
Stage 1	751	-	-	-	-	-
Stage 2	719	_	_	_	_	_
Platoon blocked. %				_		
Mov Cap-1 Maneuver	431	734	_	_	1265	_
Mov Cap-1 Maneuver	431	7.54	_	-	1200	-
Stage 1	746	-	-	-	-	-
			-	-	-	-
Stage 2	718	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	13		0		0	
HCM LOS	В		-		-	
TIOM EGG						
Minor Lane/Major Mvmt		NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)		-	-	463	1265	-
HCM Lane V/C Ratio		-	-	0.026	0.001	-
HCM Control Delay (s)		-	-	13	7.8	0
HCM Lane LOS		-	-	В	Α	Α
HCM 95th %tile Q(veh)		-	-	0.1	0	-

Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
		WDR		INDIX	SDL	
Lane Configurations	W	0	}	4	4	₽
Traffic Vol, veh/h	5	0	265	1	1	300
Future Vol, veh/h	5	0	265	1	1	300
Conflicting Peds, #/hr	0	0	_ 0	_ 8	_ 8	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	33	33	89	89	87	87
Heavy Vehicles, %	0	0	1	0	0	1
Mymt Flow	15	0	298	1	1	345
MATTICE TOW	10	- 0	200	-	-	0-10
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	653	306	0	0	307	0
Stage 1	306	-	-	-	-	-
Stage 2	347	-	-	-	-	-
Critical Hdwy	6.4	6.2	_	_	4.1	_
Critical Hdwy Stg 1	5.4	-	_	_		
Critical Hdwy Stg 2	5.4	_			_	
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
		739			1265	
Pot Cap-1 Maneuver	435		-	-		-
Stage 1	751	-	-	-	-	-
Stage 2	720	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	431	733	-	-	1265	-
Mov Cap-2 Maneuver	431	-	-	-	-	-
Stage 1	745	-	_	-	-	-
Stage 2	719	_	_	_	_	_
Clago 2	7.10					
Approach	WB		NB		SB	
HCM Control Delay, s	13.7		0		0	
HCM LOS	В					
Min 1 /N4-i N4		NDT	NDD	WDI 4	ODI	ODT
Minor Lane/Major Mvmt		NBT		WBLn1	SBL	SBT
Capacity (veh/h)		-	-	431	1265	-
HCM Lane V/C Ratio		-	-	0.035	0.001	-
HCM Control Delay (s)		-	-	13.7	7.8	0
HCM Lane LOS		-	-	В	Α	Α
HCM 95th %tile Q(veh)		-	-	0.1	0	-
300. 700.0 4(1311)				V.1		

-						
Intersection						
Int Delay, s/veh	0.1					
		WDD	NDT	NDD	ODI	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		f)		_	4
Traffic Vol, veh/h	2	1	265	1	2	305
Future Vol, veh/h	2	1	265	1	2	305
Conflicting Peds, #/hr	0	0	0	8	8	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	_	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	75	75	89	89	86	86
Heavy Vehicles, %	0	0	1	0	0	1
Mymt Flow	3	1	298	1	2	355
WWITTIOW	3		230		2	000
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	665	306	0	0	307	0
Stage 1	306	-	-	-	-	-
Stage 2	359	-	-	-	-	-
Critical Hdwy	6.4	6.2	_	_	4.1	_
Critical Hdwy Stg 1	5.4	-			-	
Critical Hdwy Stg 2	5.4	_	_	_	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	_
Pot Cap-1 Maneuver	428	739			1265	
Stage 1	751	139	-	-	1200	-
Stage 2	711	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	424	733	-	-	1265	-
Mov Cap-2 Maneuver	424	-	-	-	-	-
Stage 1	745	-	-	-	-	-
Stage 2	710	-	-	-	-	-
Annroach	WB		NB		SB	
Approach						
HCM Control Delay, s	12.4		0		0.1	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)		-	-	493	1265	-
HCM Lane V/C Ratio			_	0.008	0.002	-
HCM Control Delay (s)		-	_	12.4	7.9	0
		-	-			
HCM Lane LOS		-	-	В	A	Α
HCM 95th %tile Q(veh)		-	-	0	0	-

Intersection												
Int Delay, s/veh	6.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4						\$	
Traffic Vol., veh/h	55	0	45	45	25	210	0	0	0	0	275	30
Future Vol, veh/h	55	0	45	45	25	210	0	0	0	0	275	30
Conflicting Peds, #/hr	3	0	0	0	0	3	0	0	0	0	0	4
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	_	_	-	_	_	-	_	_	-	_	_	-
Veh in Median Storage, #		0	_	_	0	_	_	_		_	0	_
Grade, %	-	0	_	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	92	92	92	86	86	86
Heavy Vehicles, %	0	0	0	2	0	1	0	0	0	0	1	3
Mymt Flow	60	0	49	49	27	231	0	0	0	0	320	35
WWW. TOW	00	- 0	43	43	21	201	U	U	U	- 0	320	- 33
Majar/Minar	Minario			Minaut						Maiago		
Major/Minor	Minor2	0.17	644	Minor1	0.50					Major2		
Conflicting Flow All	473	341	341	362	359	3				-	-	0
Stage 1	341	341	-	0	0	-				-	-	-
Stage 2	132	0	-	362	359	-				-	-	-
Critical Hdwy	7.1	6.5	6.2	7.12	6.5	6.21				-	-	-
Critical Hdwy Stg 1	6.1	5.5	-	-	-	-				-	-	-
Critical Hdwy Stg 2	-	-	-	6.12	5.5	-				-	-	-
Follow-up Hdwy	3.5	4	3.3	3.518	4	3.309				-	-	-
Pot Cap-1 Maneuver	505	584	706	594	571	1084				0	-	-
Stage 1	678	642	-	-	-	-				0	-	-
Stage 2	-	-	-	657	631	-				0	-	-
Platoon blocked, %											-	-
Mov Cap-1 Maneuver	380	582	703	552	569	1081				-	-	-
Mov Cap-2 Maneuver	380	582	-	552	569	-				-	-	-
Stage 1	678	640	-	-	-	-				-	-	-
Stage 2	-	-	-	611	629	-				-	-	-
Approach	EB			WB						SB		
HCM Control Delay, s	14.7			11.3						0		
HCM LOS	В			В								
Minor Lane/Major Mvmt		EBLn1	WBLn1	SBT	SBR							
Capacity (veh/h)		479	876	-	- ODIT							
HCM Lane V/C Ratio		0.229	0.351	-	_							
HCM Control Delay (s)		14.7	11.3	-	-							
HCM Lane LOS		14.7 B	11.3 B	-	-							
		0.9	1.6	-	-							
HCM 95th %tile Q(veh)		0.9	0.1	-	-							

	•	-	\rightarrow	•	←	•	1	†	/	>	ţ	4		
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Ø2	
ane Configurations		ተተኈ			ተተተ					ሻ	4	7		
Fraffic Volume (vph)	0	1390	0	0	1830	0	0	0	0	295	2	120		
uture Volume (vph)	0	1390	0	0	1830	0	0	0	0	295	2	120		
deal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	5136	0	0	5136	0	0	0	0	1715	1720	1599		
Flt Permitted		0100			0100					0.950	0.953	1000		
Satd. Flow (perm)	0	5136	0	0	5136	0	0	0	0	1715	1720	1599		
Right Turn on Red	U	0100	Yes	·	0100	Yes	Ū		Yes	17.10	1720	Yes		
Satd. Flow (RTOR)			100			100			100			130		
Link Speed (mph)		30			30			30			30	100		
Link Distance (ft)		760			318			215			145			
Fravel Time (s)		17.3			7.2			4.9			3.3			
\ /		17.3	1	1	1.2			4.9			ა.ა	4		
Confl. Peds. (#/hr)	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92		
Peak Hour Factor			0.92							0.92	0.92			
Heavy Vehicles (%)	0%	1%	U%	0%	1%	0%	0%	0%	0%	50%	υ%	1%		
Shared Lane Traffic (%)	^	1511	0	0	1000	0	0	0	0		100	120		
ane Group Flow (vph)	0	1511	0	0	1989	0	0	0	0	160	163	130		
Turn Type		NA			NA					Split	NA	Prot	_	
Protected Phases		1			1					3	3	3	2	
Permitted Phases														
Detector Phase		1			1					3	3	3		
Switch Phase														
Minimum Initial (s)		4.0			4.0					4.0	4.0	4.0	4.0	
Minimum Split (s)		9.0			9.0					9.0	9.0	9.0	20.0	
Total Split (s)		30.0			30.0					30.0	30.0	30.0	20.0	
Fotal Split (%)		37.5%			37.5%					37.5%	37.5%	37.5%	25%	
Yellow Time (s)		4.0			4.0					4.0	4.0	4.0	2.0	
All-Red Time (s)		1.0			1.0					1.0	1.0	1.0	0.0	
ost Time Adjust (s)		0.0			0.0					0.0	0.0	0.0		
Total Lost Time (s)		5.0			5.0					5.0	5.0	5.0		
_ead/Lag		Lead			Lead								Lag	
_ead-Lag Optimize?														
Recall Mode		Max			Max					None	None	None	None	
Act Effct Green (s)		26.1			26.1					10.2	10.2	10.2		
Actuated g/C Ratio		0.53			0.53					0.21	0.21	0.21		
/c Ratio		0.56			0.73					0.45	0.46	0.30		
Control Delay		11.5			14.8					22.9	23.0	6.7		
Queue Delay		0.0			0.0					0.0	0.0	0.0		
Total Delay		11.5			14.8					22.9	23.0	6.7		
_OS		В			В					С	С	Α		
Approach Delay		11.5			14.8						18.3			
Approach LOS		В			В						В			
Queue Length 50th (ft)		73			110					37	37	0		
Queue Length 95th (ft)		282			#475					117	120	39		
nternal Link Dist (ft)		680			238			135			65			
Turn Bay Length (ft)														
Base Capacity (vph)		2708			2708					904	906	904		
Starvation Cap Reductn		0			0					0	0	0		
Spillback Cap Reductn		0			0					0	0	0		
Storage Cap Reductn		0			0					0	0	0		
Reduced v/c Ratio		0.56			0.73					0.18	0.18	0.14		

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 49.4

Natural Cycle: 60

Control Type: Semi Act-Uncoord

Maximum v/c Ratio: 0.73

Intersection Signal Delay: 13.9

Intersection Capacity Utilization 51.9%

Intersection LOS: B
ICU Level of Service A

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WDL WDL	וטיי	10N ♣	וטוו	ODL	- 1
Traffic Vol, veh/h	'T '	1	210	5	2	315
Future Vol, veh/h	5	1	210	5	2	315
Conflicting Peds, #/hr	1	0	210	3	3	0
			Free	Free	Free	Free
Sign Control RT Channelized	Stop	Stop None	Free -	None	Free -	None
	0	None		None		None
Storage Length			-		-	
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	100	92	92
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	5	1	228	5	2	342
Major/Minor	Minor1		Major1		Major2	
		224		^		0
Conflicting Flow All	582	234	0	0	236	
Stage 1	234	-	-	-	-	-
Stage 2	348	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	479	810	-	-	1343	-
Stage 1	810	-	-	-	-	-
Stage 2	719	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	476	808	-	-	1343	-
Mov Cap-2 Maneuver	476	-	-	-	-	-
Stage 1	808	_	_	_	_	_
Stage 2	717	_	_	_	_	_
Olaye Z	7 17					-
Approach	WB		NB		SB	
HCM Control Delay, s	12.1		0		0	
HCM LOS	В					
Mineral and IM 1 March		NOT	NDD	WDI 4	CDI	057
Minor Lane/Major Mvmt		NBT		WBLn1	SBL	SBT
Capacity (veh/h)		-	-	511	1343	-
HCM Lane V/C Ratio		-	-	0.013	0.002	-
HCM Control Delay (s)		-	-	12.1	7.7	0
HCM Lane LOS		-	-	В	Α	Α
HCM 95th %tile Q(veh)		-	-	0	0	-
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-						
Intersection						
Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
	WDL	WDK	1ND I	INDIX	SDL	
Lane Configurations		-		_	0	4
Traffic Vol, veh/h	5	5	210	5	2	320
Future Vol, veh/h	5	5	210	5	2	320
Conflicting Peds, #/hr	0	0	_ 0	_ 2	_ 2	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0
Mymt Flow	5	5	228	5	2	348
MATTICE TOW	3	- 3	220	- 3		0-10
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	585	233	0	0	236	0
Stage 1	233	-	-	-	-	-
Stage 2	352	-	_	-	_	_
Critical Hdwy	6.4	6.2	_	-	4.1	_
Critical Hdwy Stg 1	5.4	- 0.2	_	_	-	_
Critical Hdwy Stg 2	5.4	_			-	
						_
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	477	811	-	-	1343	-
Stage 1	810	-	-	-	-	-
Stage 2	716	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	475	809	-	-	1343	-
Mov Cap-2 Maneuver	475	-	-	-	-	-
Stage 1	808	_	_	_	_	_
Stage 2	715	_	_		_	
Olago 2	710					
Approach	WB		NB		SB	
HCM Control Delay, s	11.1		0		0	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)		-	-	599	1343	-
HCM Lane V/C Ratio		-	-	0.018	0.002	-
HCM Control Delay (s)		-	-	11.1	7.7	0
HCM Lane LOS		-	-	В	Α	Α
HCM 95th %tile Q(veh)		_	-	0.1	0	-
110111 00til /0tilo Q(V011)				0.1	J	

Intersection						
Intersection Int Delay, s/veh	0					
•						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		f)			ર્ન
Traffic Vol, veh/h	0	1	215	0	0	325
Future Vol, veh/h	0	1	215	0	0	325
Conflicting Peds, #/hr	0	0	0	2	2	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0
Mymt Flow	0	1	234	0	0	353
IVIVIIIC I IOVV	U	1	204	U	- 0	000
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	589	236	0	0	236	0
Stage 1	236	-	-	-	-	-
Stage 2	353	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	_
Critical Hdwy Stg 1	5.4	-	_	_	-	_
Critical Hdwy Stg 2	5.4	_	_		_	_
Follow-up Hdwy	3.5	3.3	_	_	2.2	_
Pot Cap-1 Maneuver	474	808	_	_	1343	_
Stage 1	808	-	_	_	1070	
Stage 2	716	-	-	-	-	-
	710	-	-	-	-	-
Platoon blocked, %	470	000	-	-	4040	
Mov Cap-1 Maneuver	473	806	-	-	1343	-
Mov Cap-2 Maneuver	473	-	-	-	-	-
Stage 1	806	-	-	-	-	-
Stage 2	716	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9.5		0		0	
HCM Control Delay, s HCM LOS			0		U	
HOINI LOS	Α					
Minor Lane/Major Mvmt		NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)		_	-	806	1343	-
HCM Lane V/C Ratio		-	_	0.001	-	-
HCM Control Delay (s)		_	_	9.5	0	-
HCM Lane LOS		-	-	J.5	A	-
HCM 95th %tile Q(veh)		_	_	0	0	
HOW JOHN JOHN Q(VEII)		_	_	U	U	_

Intersection												
Int Delay, s/veh	7.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
	CDL		EDK	VVDL		WDK	INDL	INDI	INDK	SDL	<u>381</u>	SDK
Lane Configurations	45	₩,	00	70	4	470	^	0	0	0		40
Traffic Vol, veh/h	45	0	60	70	55	170	0	0	0	0	285	40
Future Vol, veh/h	45	0	60	70	55	170	0	0	0	0	285	40
Conflicting Peds, #/hr	2	0	0	0	0	2	0	_ 0	_ 0	0	0	_ 3
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	-	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	49	0	65	76	60	185	0	0	0	0	310	43
Major/Minor	Minor2			Minor1						Major2		
Conflicting Flow All	459	335	335	364	356	2				-	-	0
Stage 1	335	335	-	0	0	_				_	_	-
Stage 2	124	0	_	364	356	_				_	_	_
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2				_	_	_
Critical Hdwy Stg 1	6.1	5.5	- 0.2	- 7.1	-	- 0.2				_	_	_
Critical Hdwy Stg 2	-	5.5	-	6.1	5.5	_					_	_
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3				_	_	_
Pot Cap-1 Maneuver	516	589	712	596	573	1088				0		
Stage 1	683	646	712	-	5/5	1000				0		-
Stage 2	003	040	-	659	633	_				0	-	-
Platoon blocked, %	-	-	-	009	000	-				U	-	-
Mov Cap-1 Maneuver	392	587	710	541	571	1086						-
Mov Cap-1 Maneuver	392	587	710	541	571	1000				-	-	-
	683	644	-		5/1	-				-	-	-
Stage 1	683	644	_	- 598	631	-				-	-	-
Stage 2	-	-	-	296	031	-				-	-	-
Approach	EB			WB						SB		
HCM Control Delay, s	13.7			12.9						0		
HCM LOS	В			В								
Minor Lane/Major Mvmt		EBLn1	WBLn1	SBT	SBR							
Capacity (veh/h)		527	772	-	-							
HCM Lane V/C Ratio		0.217	0.415	-	-							
HCM Control Delay (s)		13.7	12.9	_	_							
HCM Lane LOS		В	12.3 B	_	_							
HCM 95th %tile Q(veh)		0.8	2.1	_	_							
HOW JOHN JOHN & (VOII)		0.0	۷.۱									

	•	→	•	•	←	•	4	†	1	\	↓	1		
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Ø2	
Lane Configurations		ተተ _ጉ			ተተተ					ሻ	4	7		
Traffic Volume (vph)	0	1670	0	0	1795	0	0	0	0	280	5	85		
Future Volume (vph)	0	1670	0	0	1795	0	0	0	0	280	5	85		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	5136	0	0	5136	0	0	0	0	1698	1705	1599		
Flt Permitted										0.950	0.954			
Satd. Flow (perm)	0	5136	0	0	5136	0	0	0	0	1698	1705	1599		
Right Turn on Red			Yes			Yes			Yes			Yes		
Satd. Flow (RTOR)												92		
Link Speed (mph)		30			30			30			30			
Link Distance (ft)		760			318			215			145			
Travel Time (s)		17.3			7.2			4.9			3.3			
Confl. Peds. (#/hr)			1	1								5		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92		
Heavy Vehicles (%)	0%	1%	0%	0%	1%	0%	0%	0%	0%	1%	0%	1%		
Shared Lane Traffic (%)		.,.			.,,					49%				
Lane Group Flow (vph)	0	1815	0	0	1951	0	0	0	0	155	154	92		
Turn Type		NA			NA					Split	NA	Prot		
Protected Phases		1			1					3	3	3	2	
Permitted Phases					•									
Detector Phase		1			1					3	3	3		
Switch Phase		•			•									
Minimum Initial (s)		4.0			4.0					4.0	4.0	4.0	4.0	
Minimum Split (s)		9.0			9.0					9.0	9.0	9.0	20.0	
Total Split (s)		30.0			30.0					30.0	30.0	30.0	20.0	
Total Split (%)		37.5%			37.5%					37.5%	37.5%	37.5%	25%	
Yellow Time (s)		4.0			4.0					4.0	4.0	4.0	2.0	
All-Red Time (s)		1.0			1.0					1.0	1.0	1.0	0.0	
Lost Time Adjust (s)		0.0			0.0					0.0	0.0	0.0	0.0	
Total Lost Time (s)		5.0			5.0					5.0	5.0	5.0		
Lead/Lag		Lead			Lead								Lag	
Lead-Lag Optimize?													9	
Recall Mode		Max			Max					None	None	None	None	
Act Effct Green (s)		26.0			26.0					9.9	9.9	9.9		
Actuated g/C Ratio		0.53			0.53					0.20	0.20	0.20		
v/c Ratio		0.67			0.72					0.45	0.45	0.23		
Control Delay		13.2			14.2					23.3	23.1	7.1		
Queue Delay		0.0			0.0					0.0	0.0	0.0		
Total Delay		13.2			14.2					23.3	23.1	7.1		
LOS		В			В					C	C	Α		
Approach Delay		13.2			14.2					_	19.5			
Approach LOS		В			В						В			
Queue Length 50th (ft)		94			105					35	35	0		
Queue Length 95th (ft)		#401			#448					115	114	33		
Internal Link Dist (ft)		680			238			135			65			
Turn Bay Length (ft)					,									
Base Capacity (vph)		2723			2723					900	904	891		
Starvation Cap Reductn		0			0					0	0	0		
Spillback Cap Reductn		0			0					0	0	0		
Storage Cap Reductn		0			0					0	0	0		
Reduced v/c Ratio		0.67			0.72					0.17	0.17	0.10		
TOURDOU VIO ITALIO		0.07			0.12					0.17	0.17	0.10		

Area Type: Other

Cycle Length: 80 Actuated Cycle Length: 49

Natural Cycle: 60

Control Type: Semi Act-Uncoord

Maximum v/c Ratio: 0.72

Intersection Signal Delay: 14.3 Intersection Capacity Utilization 50.9% Intersection LOS: B
ICU Level of Service A

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



Intersection						
Int Delay, s/veh	0.2					
		MDD	NOT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	À		\$	•		4
Traffic Vol, veh/h	5	1	265	2	1	295
Future Vol, veh/h	5	1	265	2	1	295
Conflicting Peds, #/hr	0	0	0	7	7	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	50	50	89	89	85	85
Heavy Vehicles, %	0	0	1	0	0	1
Mymt Flow	10	2	298	2	1	347
WWIIICI IOW	10	2	230	2		341
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	655	306	0	0	307	0
Stage 1	306	-	_	-	-	-
Stage 2	349	-	_	-	_	
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	0.2	-	-	-4.1	-
	5.4					-
Critical Hdwy Stg 2		-	-	-	-	_
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	434	739	-	-	1265	-
Stage 1	751	-	-	-	-	-
Stage 2	719	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	431	734	-	-	1265	-
Mov Cap-2 Maneuver	431	-	-	-	-	-
Stage 1	746	_	_	_	_	_
Stage 2	718	_	_	_	_	_
Olaye Z	7 10					
Approach	WB		NB		SB	
HCM Control Delay, s	13		0		0	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)		-	-	463	1265	-
HCM Lane V/C Ratio		-	-	0.026	0.001	-
HCM Control Delay (s)		-	_	13	7.8	0
HCM Lane LOS		-	_	В	A	A
HCM 95th %tile Q(veh)		-	_	0.1	0	- '.
HOW JOHN JOHN (VEII)		_	_	0.1	U	-

-						
Intersection						
Int Delay, s/veh	0.8					
	WDI	WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		f >			र्स
Traffic Vol, veh/h	10	2	265	2	2	300
Future Vol, veh/h	10	2	265	2	2	300
Conflicting Peds, #/hr	0	0	0	8	8	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	33	33	89	89	87	87
Heavy Vehicles, %	0	0	1	0	0	1
Mymt Flow	30	6	298	2	2	345
WWITETIOW	00	U	200	_		040
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	656	307	0	0	308	0
Stage 1	307	-	-	-	-	-
Stage 2	349	-	-	-	-	-
Critical Hdwy	6.4	6.2	_	_	4.1	_
Critical Hdwy Stg 1	5.4	-	_		-	
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	433	738	_		1264	
Stage 1	751	730	-	-	1204	-
			-		-	
Stage 2	719	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	429	732	-	-	1264	-
Mov Cap-2 Maneuver	429	-	-	-	-	-
Stage 1	745	-	-	-	-	-
Stage 2	718	-	-	-	-	-
·						
Anaroach	WB		NB		SB	
Approach						
HCM Control Delay, s	13.5		0		0.1	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT	NRR	WBLn1	SBL	SBT
Capacity (veh/h)		-	- NDIX	461	1264	- 301
HCM Lane V/C Ratio			-	0.079	0.002	
		-	-			-
HCM Control Delay (s)		-	-	13.5	7.9	0
HCM Lane LOS		-	-	В	A	Α
HCM 95th %tile Q(veh)		-	-	0.3	0	-

Intersection												
Int Delay, s/veh	6.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		.,,,,,	4				11511	022	1>	05.1
Traffic Vol., veh/h	55	0	45	45	25	210	0	0	0	0	280	30
Future Vol, veh/h	55	0	45	45	25	210	0	0	0	0	280	30
Conflicting Peds, #/hr	3	0	0	0	0	3	0	0	0	0	0	4
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	_	_	-	_	_	-	_	_	-	_	_	-
Veh in Median Storage, #	_	0	-	_	0	_	_	_	_	_	0	_
Grade, %	-	0	_	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	92	92	92	86	86	86
Heavy Vehicles, %	0	0	0	2	0	1	0	0	0	0	1	3
Mymt Flow	60	0	49	49	27	231	0	0	0	0	326	35
WWITH FIUW	00	U	49	43	21	201	U	U	U	U	320	33
Major/Minor	Minor2			Minor1						Major2		
Conflicting Flow All	479	347	347	368	364	3				-	-	0
Stage 1	347	347	-	0	0	-				-	-	-
Stage 2	132	0	-	368	364	-				-	-	-
Critical Hdwy	7.1	6.5	6.2	7.12	6.5	6.21				-	-	-
Critical Hdwy Stg 1	6.1	5.5	-	-	-	-				-	-	-
Critical Hdwy Stg 2	-	-	-	6.12	5.5	-				-	-	-
Follow-up Hdwy	3.5	4	3.3	3.518	4	3.309				-	-	-
Pot Cap-1 Maneuver	500	580	701	588	567	1084				0	-	-
Stage 1	673	638	-	-	-	-				0	-	-
Stage 2	-	-	-	652	627	-				0	-	-
Platoon blocked, %											-	-
Mov Cap-1 Maneuver	376	578	698	546	565	1081				-	-	-
Mov Cap-2 Maneuver	376	578	-	546	565	-				-	-	-
Stage 1	673	636	-	-	-	-				-	-	-
Stage 2	-	-	-	606	625	-				-	-	-
Approach	EB			WB						SB		
HCM Control Delay, s	14.8			11.4						0		
HCM LOS	В			В								
Minor Lane/Major Mvmt		EBLn1	WBLn1	SBT	SBR							
Capacity (veh/h)		475	872		JUIN -							
HCM Lane V/C Ratio		0.231	0.353	-	-							
HCM Control Delay (s)		14.8	11.4	-	-							
HCM Lane LOS		14.6 B	11.4 B	-	-							
		0.9	1.6	-	-							
HCM 95th %tile Q(veh)		0.9	1.0	-	-							

	۶	→	•	1	←	4	1	†	<u> </u>	/	 	4		
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Ø2	
Lane Configurations		ተተኈ			ተተተ					*5	4	7		
Traffic Volume (vph)	0	1390	0	0	1830	0	0	0	0	295	2	120		
Future Volume (vph)	0	1390	0	0	1830	0	0	0	0	295	2	120		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	5136	0	0	5136	0	0	0	0	1715	1720	1599		
Flt Permitted		0100			0100					0.950	0.953	1000		
Satd. Flow (perm)	0	5136	0	0	5136	0	0	0	0	1715	1720	1599		
Right Turn on Red		0100	Yes		0100	Yes			Yes	1110	1120	Yes		
Satd. Flow (RTOR)			100			100			100			130		
Link Speed (mph)		30			30			30			30	100		
Link Distance (ft)		760			318			215			145			
Travel Time (s)		17.3			7.2			4.9			3.3			
Confl. Peds. (#/hr)		17.0	1	1	1.2			7.0			0.0	4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92		
Heavy Vehicles (%)	0.92	1%	0.92	0.92	1%	0.92	0.92	0.92	0.32	0.32	0.92	1%		
Shared Lane Traffic (%)	0 70	1 /0	0 /0	0 /0	1 /0	0 /0	0 70	0 /0	0 70	50%	0 /0	1 /0		
Lane Group Flow (vph)	0	1511	0	0	1989	0	0	0	0	160	163	130		
Turn Type	U	NA	U	U	NA	U	U	U	U	Split	NA	Prot		
Protected Phases		1 1			1 1					Spiit 3	3	3	2	
Permitted Phases		ı			- 1					3	3	J	2	
		1			1					3	3	3		
Detector Phase		1			1					3	3	3		
Switch Phase		4.0			4.0					4.0	4.0	4.0	4.0	
Minimum Initial (s)		4.0			4.0					4.0	4.0	4.0	4.0	
Minimum Split (s)		9.0			9.0					9.0	9.0	9.0	20.0	
Total Split (s)		30.0			30.0					30.0	30.0	30.0	20.0	
Total Split (%)		37.5%			37.5%					37.5%	37.5%	37.5%	25%	
Yellow Time (s)		4.0			4.0					4.0	4.0	4.0	2.0	
All-Red Time (s)		1.0			1.0					1.0	1.0	1.0	0.0	
Lost Time Adjust (s)		0.0			0.0					0.0	0.0	0.0		
Total Lost Time (s)		5.0			5.0					5.0	5.0	5.0		
Lead/Lag		Lead			Lead								Lag	
Lead-Lag Optimize?														
Recall Mode		Max			Max					None	None	None	None	
Act Effct Green (s)		26.1			26.1					10.2	10.2	10.2		
Actuated g/C Ratio		0.53			0.53					0.21	0.21	0.21		
v/c Ratio		0.56			0.73					0.45	0.46	0.30		
Control Delay		11.5			14.8					22.9	23.0	6.7		
Queue Delay		0.0			0.0					0.0	0.0	0.0		
Total Delay		11.5			14.8					22.9	23.0	6.7		
LOS		В			В					С	С	Α		
Approach Delay		11.5			14.8						18.3			
Approach LOS		В			В						В			
Queue Length 50th (ft)		73			110					37	37	0		
Queue Length 95th (ft)		282			#475					117	120	39		
Internal Link Dist (ft)		680			238			135			65			
Turn Bay Length (ft)														
Base Capacity (vph)		2708			2708					904	906	904		
Starvation Cap Reductn		0			0					0	0	0		
Spillback Cap Reductn		U												
		0			0					0	0	0		
Storage Cap Reductn										0	0	0		

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 49.4

Natural Cycle: 60

Control Type: Semi Act-Uncoord

Maximum v/c Ratio: 0.73

Intersection Signal Delay: 13.9

Intersection Capacity Utilization 51.9%

Intersection LOS: B ICU Level of Service A

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 5: Driveway/Langley Road & Route 9 (Boylston Street)



-						
Intersection						
Int Delay, s/veh	0.1					
	WD	WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Ä		}	-	•	4
Traffic Vol, veh/h	5	1	210	5	2	320
Future Vol, veh/h	5	1	210	5	2	320
Conflicting Peds, #/hr	1	0	0	3	3	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	100	92	92
Heavy Vehicles, %	0	0	0	0	0	0
Mymt Flow	5	1	228	5	2	348
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	587	234	0	0	236	0
Stage 1	234	-	-	-	-	-
Stage 2	353	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	475	810	-	-	1343	-
Stage 1	810	-	-	-	-	-
Stage 2	716	_	_	_	-	-
Platoon blocked, %	710		_	_		
Mov Cap-1 Maneuver	472	808	_	_	1343	_
Mov Cap-1 Maneuver	472	-	-	-	1040	
	808	-	_	-	-	-
Stage 1	714		-	-	-	-
Stage 2	/14	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	12.2		0		0	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT	NBR		SBL	SBT
Capacity (veh/h)		-	-	507	1343	-
HCM Lane V/C Ratio		-	-	0.013	0.002	-
HCM Control Delay (s)		-	-	12.2	7.7	0
HCM Lane LOS		-	-	В	Α	Α
HCM 95th %tile Q(veh)		-	-	0	0	-
				•		

Intersection						
Int Delay, s/veh	0.3					
		WDD	NDT	NDD	ODI	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		₽		_	4
Traffic Vol, veh/h	5	5	210	10	5	320
Future Vol, veh/h	5	5	210	10	5	320
Conflicting Peds, #/hr	0	0	0	2	2	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	=	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0
Mymt Flow	5	5	228	11	5	348
WWITETIOW	3	3	220	- 11	J	070
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	595	236	0	0	241	0
Stage 1	236	-	-	-	-	-
Stage 2	359	-	-	-	-	-
Critical Hdwy	6.4	6.2	_	-	4.1	-
Critical Hdwy Stg 1	5.4	-	_	_		_
Critical Hdwy Stg 2	5.4	_	_		_	
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	470	808	-	-	1337	-
	808	000				
Stage 1			-	-	-	-
Stage 2	711	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	467	806	-	-	1337	-
Mov Cap-2 Maneuver	467	-	-	-	-	-
Stage 1	806	-	-	-	-	-
Stage 2	707	-	-	-	-	-
A may a a a b	WD		ND		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	11.2		0		0.1	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT	NRD	WBLn1	SBL	SBT
Capacity (veh/h)		-	- NDIX	591	1337	-
Capacity (ven/n)						
HCM Lane V/C Ratio		-	-	0.018	0.004	-
HCM Control Delay (s)		-	-	11.2	7.7	0
HCM Lane LOS		-	-	В	Α	Α
HCM 95th %tile Q(veh)		-	-	0.1	0	-

Intersection												
Int Delay, s/veh	7.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4						f)	
Traffic Vol., veh/h	45	0	60	70	55	175	0	0	0	0	285	40
Future Vol. veh/h	45	0	60	70	55	175	0	0	0	0	285	40
Conflicting Peds, #/hr	2	0	0	0	0	2	0	0	0	0	0	3
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	-	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	49	0	65	76	60	190	0	0	0	0	310	43
Major/Minor	Minor2			Minor1						Major2		
Conflicting Flow All	462	335	335	364	356	2				-	_	0
Stage 1	335	335	-	0	0	-				-	-	-
Stage 2	127	0	-	364	356	-					-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2				-	-	-
Critical Hdwy Stg 1	6.1	5.5	-	-	-	-					-	-
Critical Hdwy Stg 2	-	-	-	6.1	5.5	-				-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3				-	-	-
Pot Cap-1 Maneuver	513	589	712	596	573	1088				0	-	-
Stage 1	683	646	-	-	-	-				0	-	-
Stage 2	-	-	-	659	633	-				0	-	-
Platoon blocked, %											-	-
Mov Cap-1 Maneuver	387	587	710	541	571	1086				-	-	-
Mov Cap-2 Maneuver	387	587	-	541	571	-				-	-	-
Stage 1	683	644	-	-	-	-				-	-	-
Stage 2	-	-	-	598	631	-				-	-	-
ŭ												
Approach	EB			WB						SB		
HCM Control Delay, s	13.8			13						0		
HCM LOS	В			В								
Minor Lane/Major Mvmt		EBLn1	WBLn1	SBT	SBR							
Capacity (veh/h)		523	775	-	-							
HCM Lane V/C Ratio		0.218	0.421	-	-							
HCM Control Delay (s)		13.8	13	-	-							
HCM Lane LOS		В	В	-	-							
HCM 95th %tile Q(veh)		0.8	2.1	-	_							
300 /00 ((.011)		0.0										



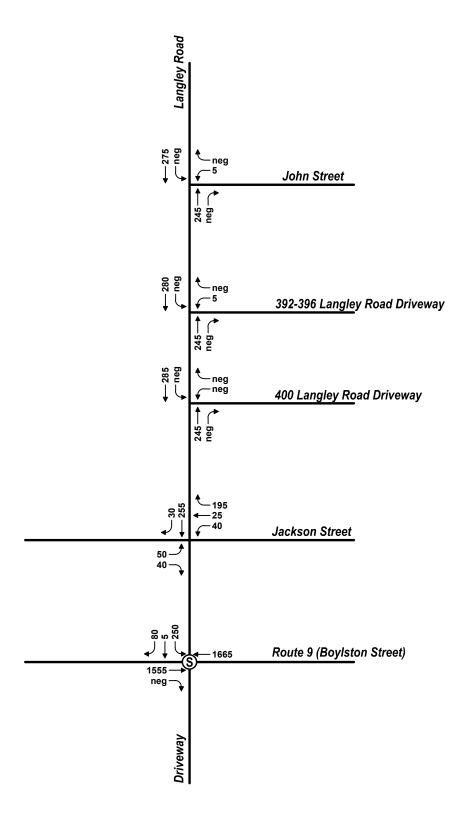
Figures

Stop Controlled Intersection



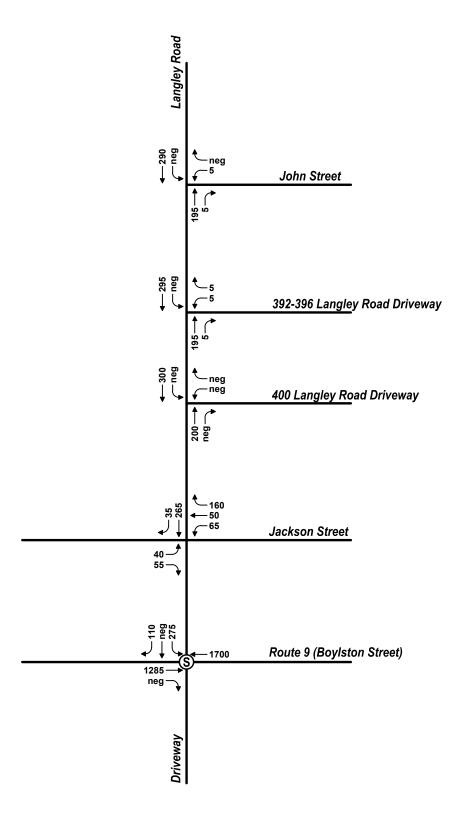


neg Negligible

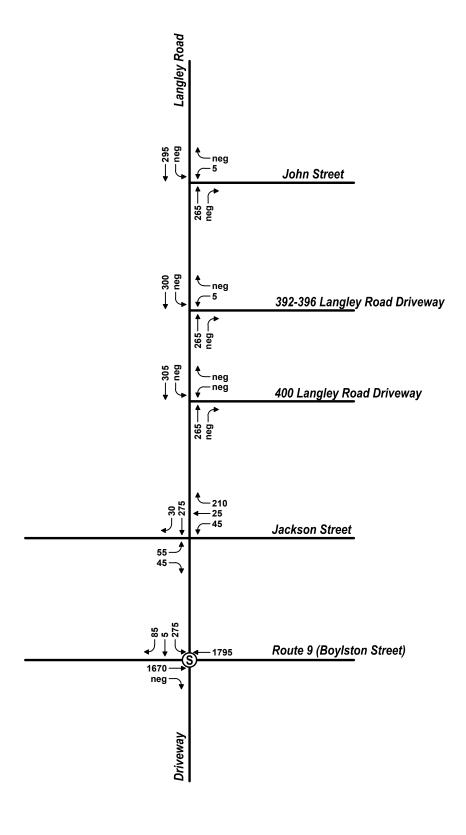




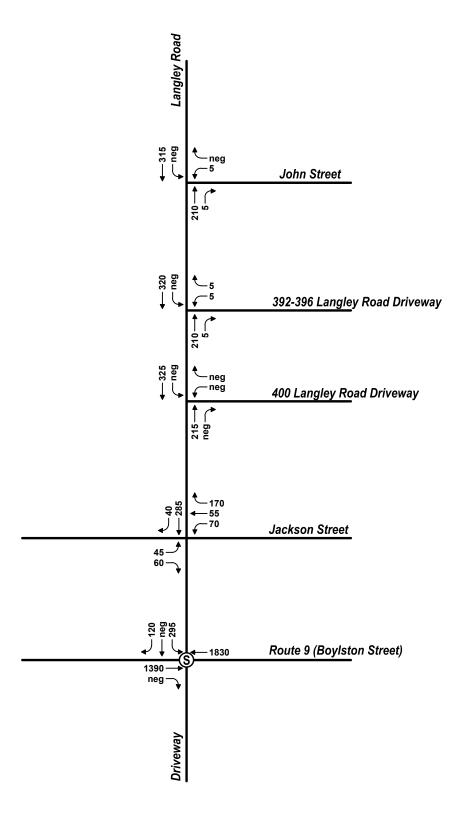
2017 Existing Conditons Weekday Morning Peak Hour Traffic Volumes Langley Road Redevelopment Newton, Massachusetts







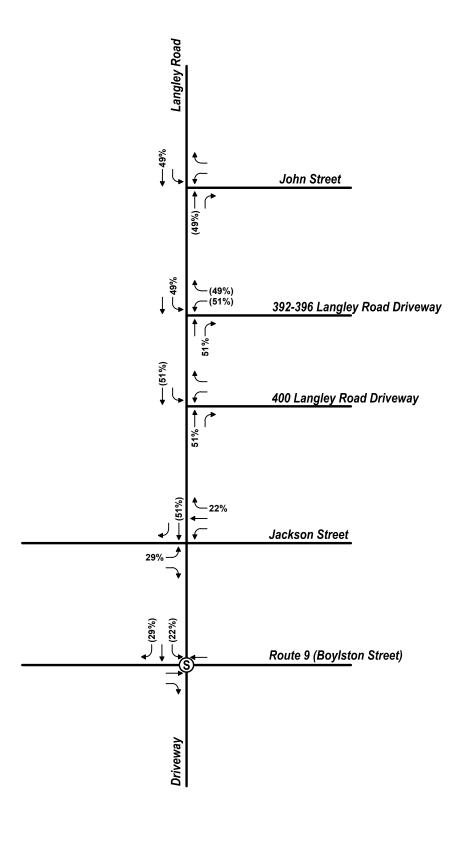






XX% = Entering Trips

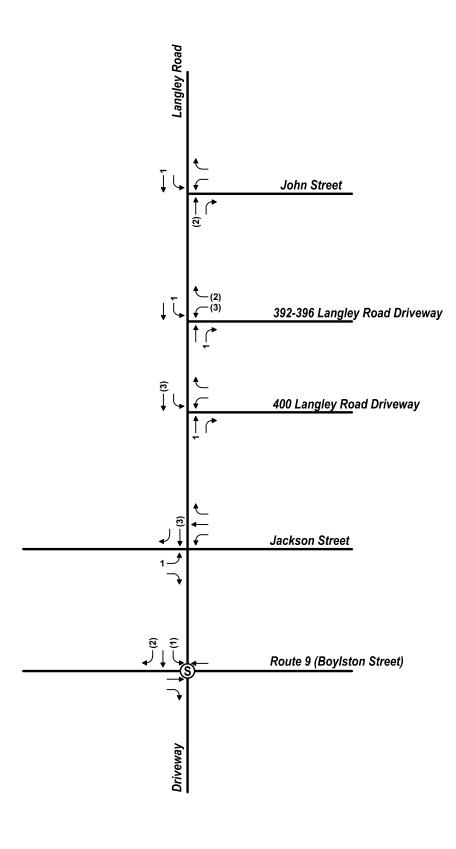
(XX%) = Exiting Trips



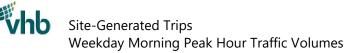


XX = Entering Trips

(XX) = Exiting Trips



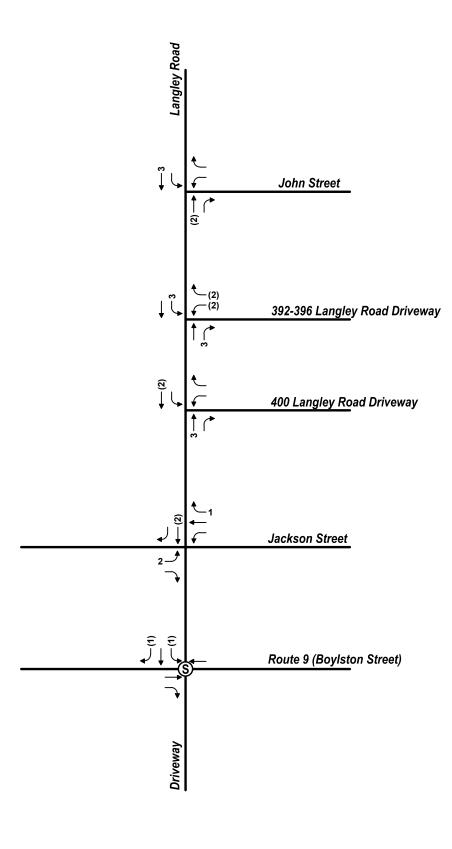




Langley Road Redevelopment Newton, Massachusetts

XX = Entering Trips

(XX) = Exiting Trips





Site-Generated Trips Weekday Evening Peak Hour Traffic Volumes Langley Road Redevelopment Newton, Massachusetts

