## Stormwater Management Report

for 127 Clark Street Residence Newton, Massachusetts

Prepared For Tim & Lauren Fagerberg

July 2021



Prepared by: J. Veillette, E.I.T. Revied by: G. Munden, P.E.



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### Section 1 Introduction & Regulatory Compliance

On behalf of the project applicant and current property owners, Tim & Lauren Fagerberg, Munden Engineering has prepared the following Stormwater Management Report to support the proposed building addition to a single-family dwelling located at 127 Clark Street in the City of Newton, Massachusetts.

A drainage analysis of the pre-development and post-development site was performed to determine the impacts of the proposed project to peak discharge rates and stormwater runoff volumes. The pre- and post-development drainage calculations were prepared utilizing HydroCAD Release 10.00-26, the U.S. Soil Conservation Service National Engineering Hydrology Handbook, design rainfall data determined by the City of Newton, and accepted engineering design practices. HydroCAD Release 10.00-26 is a hydrology and hydraulics software using Technical Release (TR) 20 and TR-55 methodologies. Applying the regulatory standard to the HydroCAD software, a representative model of the existing and proposed stormwater conditions were generated. The HydroCAD Reports for both the pre- and post-development conditions for the design storm of 8.78 inches for 24-hr Type-III Rainfall are provided in the Appendices of this report.

### **Section 2 Project Summary**

The property is located on the West side of Clark Street and abuts residential properties to the North, West, and South. On the East side of the property, the lot has a 60-foot frontage along Clark Street.

The Site currently consists of a 2-story single family dwelling with a covered porch, driveway, brick walkways, and retaining walls on 0.25 acres of land within the MR1 residential zone. The proposed project includes the construction of a 2-story addition off the rear side of the existing dwelling.

A site location figure, orthophotograph, and priority resources figure of the subject property is provided in Appendix A. Site plans of the proposed work are provided separately.

### **Section 3 Existing Conditions**

The existing site consists of the 1,123 sq.ft. dwelling, 1,591 sq.ft. driveway, 169 sq.ft. walkways, 163 sq.ft. gravel area, and 91 sq.ft. stone retaining walls. Under existing conditions, the total impervious area is 3,261 sq.ft. The existing driveway pavement within the City limits includes approximately 108 sq.ft. area. The ground cover surrounding the existing house and other impervious areas are maintained as manicured lawn. The existing topography slopes downhill from the front of the lot to the rear, with slopes ranging from approximately 7 to 11 percent.

The NRCS soil data was obtained through the Web Soil Survey portal on the USDA NRCS website. The areas surrounding the property were queried for soil types according to the record soil survey maps maintained by NRCS. Soils within the project area, as published in the USDA Soil Survey Norfolk County, Version 20, dated June 9th, 2020, include Haven-Urban land complex. The NRCS Soils Mapping is provided in Appendix B.

The hydrologic soil group designation (HSG) for the subject property is listed as "A". The HSG rating is based on estimates of runoff potential. The soils in the United States are assigned to one of four groups (A, B, C, and D). HSG A indicates a very high infiltration rate with low runoff potential. A subsurface exploration program of the Site was conducted on December 28th, 2020 by Munden Engineering. The soils observed were made up predominantly of a silt loam. From the test pit explorations, the soils observed are more representative of HSG B, indicating moderate infiltration rates and moderate rates of water transmission. Therefore, HSG B was used in the calculations to depict a more accurate hydrologic model. The test pit locations are shown on the Plans.

Currently, there are no stormwater management practices employed on the property and all the stormwater runoff flows uncontrolled off-site. An Existing Conditions Drainage Area Map is provided as Figure 4 in the Appendix A.

For the purposes of producing a hydrologic model, one design point were analyzed for the predevelopment conditions. The sidewalk section of the driveway which is within the street layout has no proposed changes in size or direction, and is sloped towards the roadway pavement, and therefore does not affect the comparison of existing vs proposed. Therefore, that section is not included in calculations and area calculations represent ground covers within the property limits only. Based on the design storm of 8.78 inches for 24-hr Type-III Rainfall, the peak rate of runoff at Design Point DP-1 is 1.47 cfs.

The runoff curve numbers (CN) used in the calculation of the composite CN for each drainage area is based on the values provided in TR-55, Urban Hydrology for Small Watersheds. CN values vary depending on the type of ground cover and soil HSG. The CN used in the calculation of the composite CN for each drainage area is based on the values provided in Table 3.1.

Ground Cover Type	Runoff Curve Number
>75% Grass Cover, Good, HSG B	61
Roofs and Covered Porch, HSG B	98
Driveway, HSG B	98
Shed, HSG B	98
Gravel Surface, HSG B	96
Walkways, Conc. Pad and Walls, HSG B	98

**Table 3.1 Existing Conditions Runoff Curve Numbers** 

The existing conditions drainage area was delineated based on topography and stormwater discharge location. A summary of each existing conditions drainage area, its size and associated composite CN is provided in Table 3.2.

Table 3.2 Existing Conditions Drainage Area Summary
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Drainage Area	Drainage Area	Composite Runoff
Designation	Size (sq. ft.)	Curve Number (CN)
Drainage Area 1S 10,830		72
Total	10,830	72

### **Section 4 Proposed Site Conditions**

The proposed site development involves the construction of a building addition attached to the southwestern side of the existing house and reconfiguring the existing driveway. The project will

result in an increase of impervious ground surface. Currently, 3,261 sq.ft. of impervious surfaces exist within the project area. The total proposed impervious area for the project is 4,671 sq.ft., this includes the single-family dwelling, covered porch, the new building addition, raised deck, walkways, concrete pad, retaining walls and driveway. The difference in impervious areas is 1,410 sq. ft. between existing and proposed conditions. This design has been prepared in accordance with recommendations in the Massachusetts Department of Environmental Protection (MassDEP) Stormwater Handbook.

Under proposed conditions, the topography will not be altered, and stormwater runoff will continue to be directed to the back of the lot. A Proposed Conditions Drainage Area Map is provided as Figure 5 in the Appendix A.

Based on the design storm of 8.78 inches for 24-hr Type-III Rainfall, the peak rate of runoff at Design Point DP-1 is 1.38 cfs.

The CN used in the calculation of the composite CN for each drainage area is based on the values provided in Table 4.1.

Ground Cover Type	Runoff Curve Number
Roofs and Covered Porch, HSG B	98
Driveway, HSG B	98
>75% Grass Cover, Good, HSG B	61
Walkways, Pad and Ret. Walls, HSG B	98
Gravel surface, HSG B	96
Stone Patio, HSG B	98

 Table 4.1 Proposed Conditions Runoff Curve Numbers

A summary of each proposed conditions drainage area, its size and associated composite CN is provided in Table 4.2 on the following page.

Drainage Area Designation	Drainage Area Size (sq. ft.)	Composite Runoff Curve Number (CN)
Drainage Area 1S	9,147	75
Drainage Area 2S	1,683	98
Total	10,830	77

**Table 4.2 Proposed Conditions Drainage Area Summary** 

The proposed stormwater management system includes best management practices (BMP's) such as a catch basin, conveyance piping and infiltration chambers.

A brief description of the proposed Best Management Practices incorporated into the stormwater management system are as follows:

<u>Deep-Sump, Hooded Catch Basins:</u> The catch basin located at the driveway is provided to collect stormwater runoff from the driveway and/or parking areas and connect to the project's stormwater collection system. The deep-sump and hooded outlet provide runoff an opportunity to separate from solids and floatable pollutants prior to discharge and are used as a pretreatment device.

<u>Infiltration Chambers:</u> The proposed infiltration chambers are the collection point for the stormwater runoff from the proposed driveway (approximately 1,683 sq. ft.). The open-bottom, plastic chamber system is designed to receive stormwater from impervious areas such as driveways. The captured stormwater slowly infiltrates through the bottom and sides of the system into the surrounding sub-soil. The subsurface infiltration system has been designed in accordance with Massachusetts Stormwater Handbook to provide the required groundwater recharge for the project.

#### Section 5 Compliance with Stormwater Management Standards

The project is required to comply with the ten MassDEP Massachusetts Stormwater Standards (Standards) under the Massachusetts Wetlands Protection Act and the City of Newton Stormwater Management Regulations.

### 5.1 Standard 1: No New Stormwater Conveyances of Untreated Stormwater or Erosion Offsite

The project will not result in any new stormwater conveyance discharging untreated stormwater directly to the waters of the Commonwealth. Stormwater discharges are limited to the discharges from the subsurface infiltration chambers. Furthermore, it is not anticipated that erosive stormwater velocities will be encountered post-construction. All discharges have been designed to meet the thresholds identified in Volume 3 of the Massachusetts Stormwater Handbook.

#### 5.2 Standard 2: Peak Discharge Rate Attenuation

The project will not substantially alter the existing stormwater flow paths at the site through the use of infiltration. Under proposed conditions, all of the driveway area runoff will be conveyed to the infiltration chamber while the roof runoff will continue to flow to the backyard.

A hydrologic analysis of the pre-development and post-development site was performed to determine the impacts of the proposed project to peak discharge rates and stormwater runoff volumes. HydroCAD is a hydrology and hydraulics software using Technical Release (TR) 20 and TR-55 methodologies for the determination of stormwater runoff quantities. The HydroCAD Reports for the pre- and post-development conditions for the City of Newton design storm event is provided in Appendix C and D.

Table 5.1 indicates that post-development peak discharge rates do not exceed pre-development peak discharge rates.

Storm Frequency	Existing Conditions Peak Rate of Runoff (cfs)	Proposed Conditions Peak Rate of Runoff Total (cfs) (Minimum Tc 6 min.)	Proposed Conditions Peak Rate of Runoff Total (cfs) (Minimum Tc 0 min.)
Design Storm 8.78" Type-III Rainfall	1.47	1.38	1.42

Note: Methodologies used in for the purposes of stormwater management calculations recommend that minimum of 6 minutes Time of Concentration is used. Additional Proposed Peak Rate of runoff calculation performed without minimum Tc is presented for comparison in case the minimum 6 min Tc number might be considered to underestimate the difference between existing as proposed conditions.

#### 5.3 Standard 3: Groundwater Recharge and Discharge Volume

The proposed project will allow treated stormwater runoff from the proposed project to infiltrate to groundwater. Discharges from the infiltration chambers are properly pretreated to remove at least 80% of the total suspended solids (TSS) load from the stormwater flow. Further documentation pertaining to stormwater treatment is provided in section 5.4.

The driveway runoff is directed towards a catch basin and conveyed through drainage piping to the subsurface infiltration chambers. The stormwater is stored in the Cultec chambers until the runoff exfiltrates through the stormwater system and back into the groundwater. Recharge calculations for compliance with standard 3 are provided below.

Required Recharge Volume  $(R_{\nu}) = (F) * (A_{IMP})$ 

Increase in impervious area  $(A_{IMP}) = 1,969$  sq. ft.

For Hydrologic Soil Group B = 0.35 in./sq.ft recharge is required.

*Recharge required:* 1,969 *sq.ft.* x 0.35 *in./sq.ft.* x (1 *ft./* 12 *in.*) = 57.4 *cu. ft.* 

Recharge provided: 766.2 cu.ft. for 10 CULTEC 280 HD chambers installed storage

#### 5.4 Standard 4: Water Quality

Standard 4 of the Massachusetts Stormwater Standards addresses stormwater quality requirements. This standard requires that new stormwater management systems be designed to remove 80% of the average annual post-construction load of Total Suspended Solids (TSS). MassDEP has published presumed removal rates for each of the BMP's featured in their design guidelines. Additionally, this standard addresses the required volume of stormwater runoff that is to be treated by the BMPs, as well as components of a long-term source control and pollution prevention plan.

The following treatment trains have been incorporated into the design of the stormwater management system:

**Treatment Train 1:** This treatment train consists of a deep-sump hooded catch basin and subsurface infiltration system. The overall TSS removal for this train is 85%.

The project has been designed such that all proposed impervious and gravel surfaces, excluding the building rooftop, pass through the previously described treatment trains, which results in the required TSS removal for the project. The MassDEP TSS Removal Worksheet is provided in Appendix D.

Stormwater quality calculations for compliance with standard 4 are provided below.

 $V_{WQ} = (D_{WQ}/12 \text{ inches/foot}) * (A_{IMP})$   $V_{WQ} = (\frac{1}{2}-\text{inch}/12 \text{ inches/foot}) * (1,969 \text{ sq.ft.})$  $V_{WQ} = 82 \text{ cubic feet}$ 

# 5.5 Standard 5: Land Uses with Higher Potential Pollutant Loads (LUHPPLs)

The proposed use is not considered a LUHPPL. Therefore, compliance with the additional requirements of Standard 5 is not required.

### 5.6 Standard 6: Critical Areas

The project will not result in any new discharges to wetlands, or surface waters. The project site is not located in critical areas based on the MassGIS Priority Resource Map in Appendix A.

#### 5.7 Standard 7: Redevelopment Projects

This site is considered a redevelopment project. The project has been designed to fully comply with all Standards.

### 5.8 Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A construction period Stormwater Management and Erosion Control Plan (SMECP) is included in the Site Plans. The SMECP presents the minimum soil erosion and sediment control practices to be used during construction. A Construction Site Inspection Log is included in Appendix E.

Additionally, there will be less than one acre of land disturbed as a result of this project; therefore, the construction will not be required to submit a Construction general permit (CGP).

#### 5.9 Standard 9: Long-Term Operation and Maintenance Plan

A Long-Term Stormwater Operations and Maintenance Plan is included in Appendix F of this report. The O&M plan indicates the responsible parties for the project, routine and non-routine maintenance tasks and inspection criteria.

#### 5.10 Standard 10: Prohibition of Illicit Discharges

Illicit discharges to the stormwater management system are discharges that are not entirely comprised of stormwater. Illicit discharge does not include discharges from the following activities or facilities: firefighting, water line flushing, landscape irrigation, uncontaminated groundwater, potable water sources, foundation drains, air conditioning, condensation, footing drains, individual resident car washing, flows from riparian habitats and wetlands, dechlorinated water from swimming pools, water used for street washing, and water used to clean residential buildings without detergents. The Owner is not aware of any existing illicit discharges to the Site and is not proposing any illicit discharges as part of the project. A signed Illicit Discharge Statement is provided on the following page of this report.



#### **Illicit Discharge Statement**

At no time will the owner or any: other individual utilize the stormwater management system for any purpose other than its intended use. The stormwater management system as shown on the attached site plan at no time shall receive discharges other than stormwater, this includes "wastewater discharges and discharges of stormwater contaminated by contact with process wastes, raw materials, raw materials, toxic pollutants, hazardous substances, oil or grease."

Timoty Fagerlug

(signature)

Tim Fagerberg (Property Owner)

(signature)

Lauren Fagerberg (Property Owner)

## **HELLOSIGN**

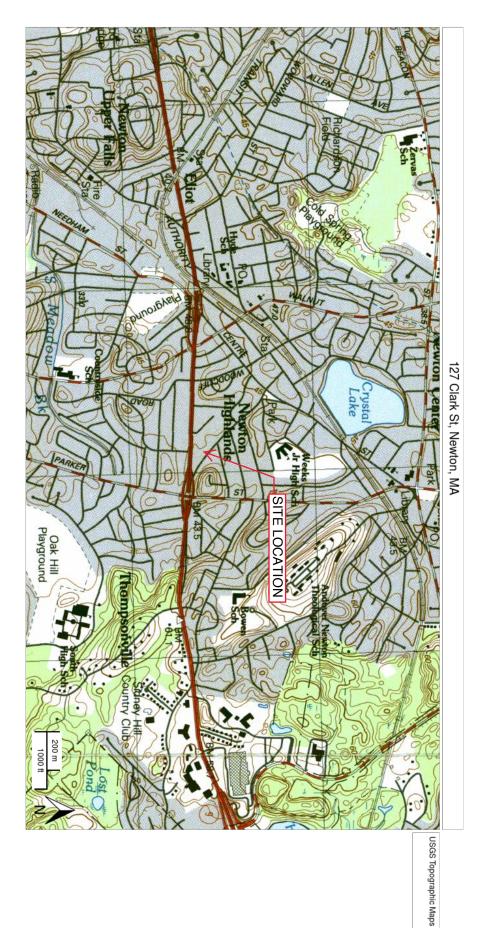
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AUDIT TRAIL DATE FORMAT	MM / DD / YYYY
STATUS	<ul> <li>Completed</li> </ul>

### Document History

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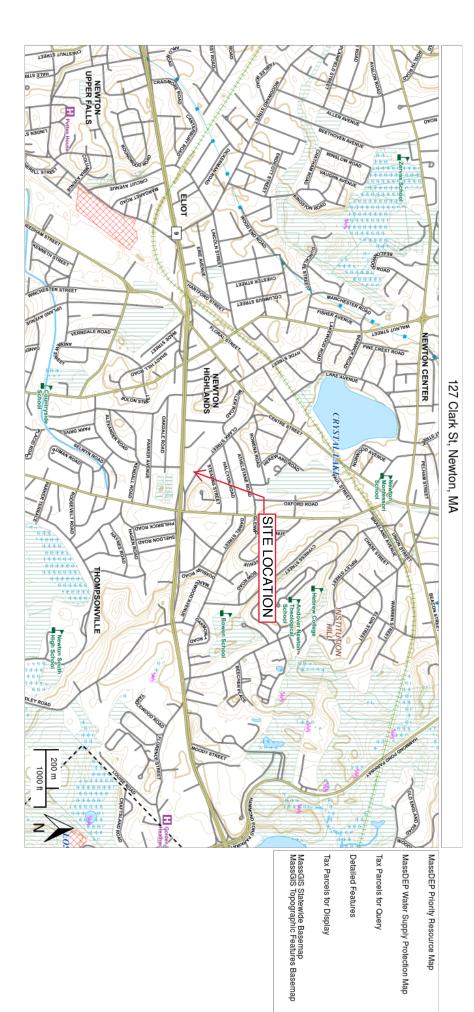


Appendix A Figures



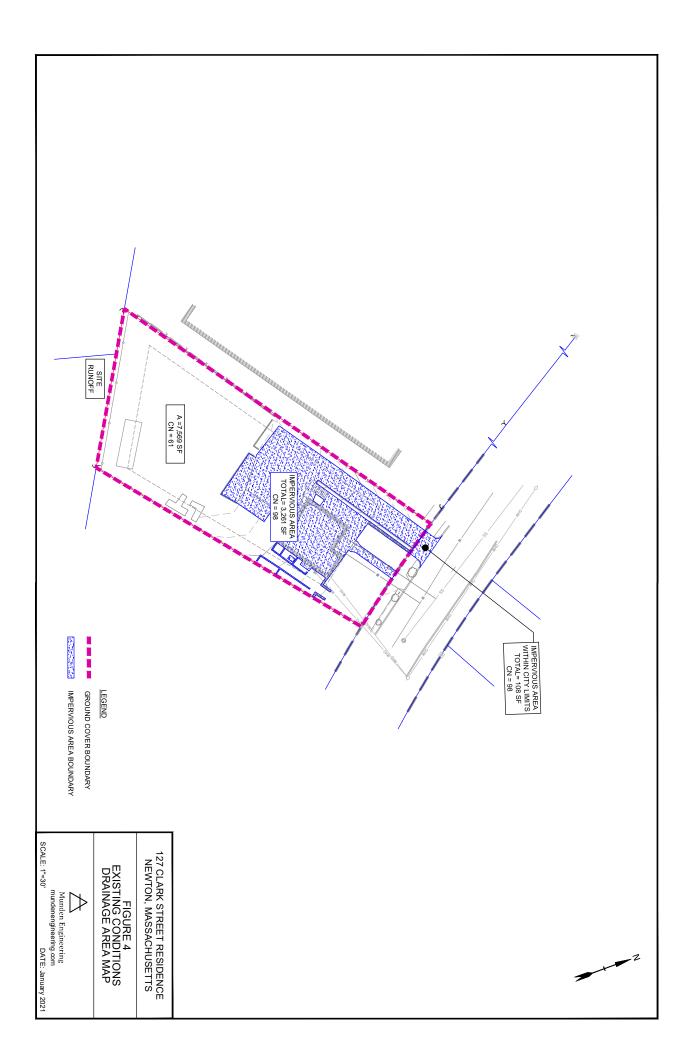
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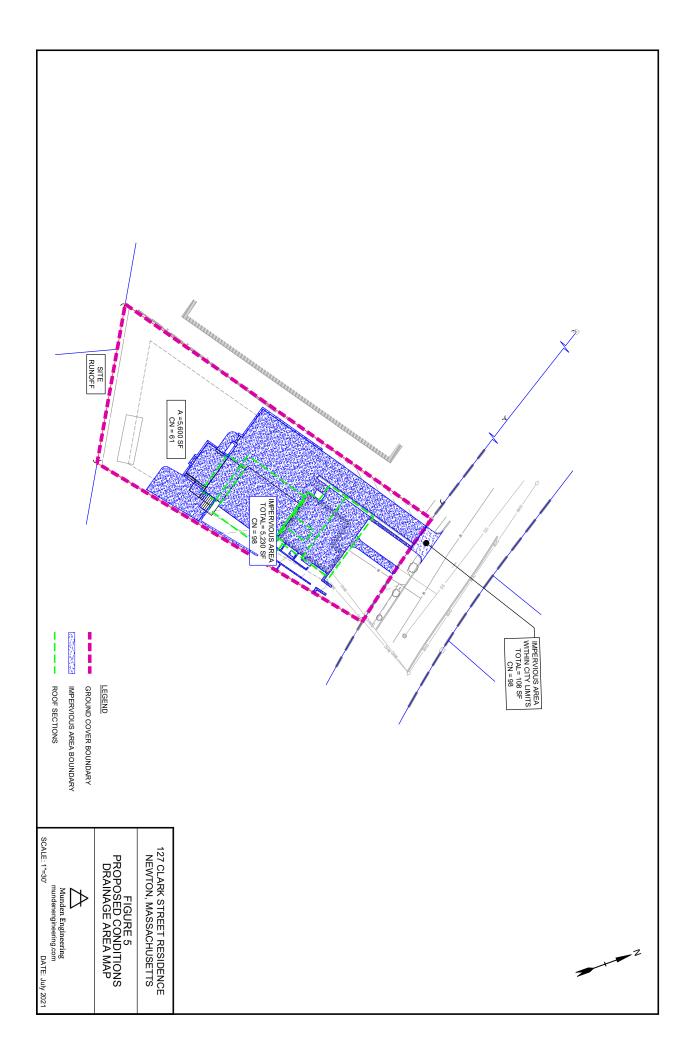
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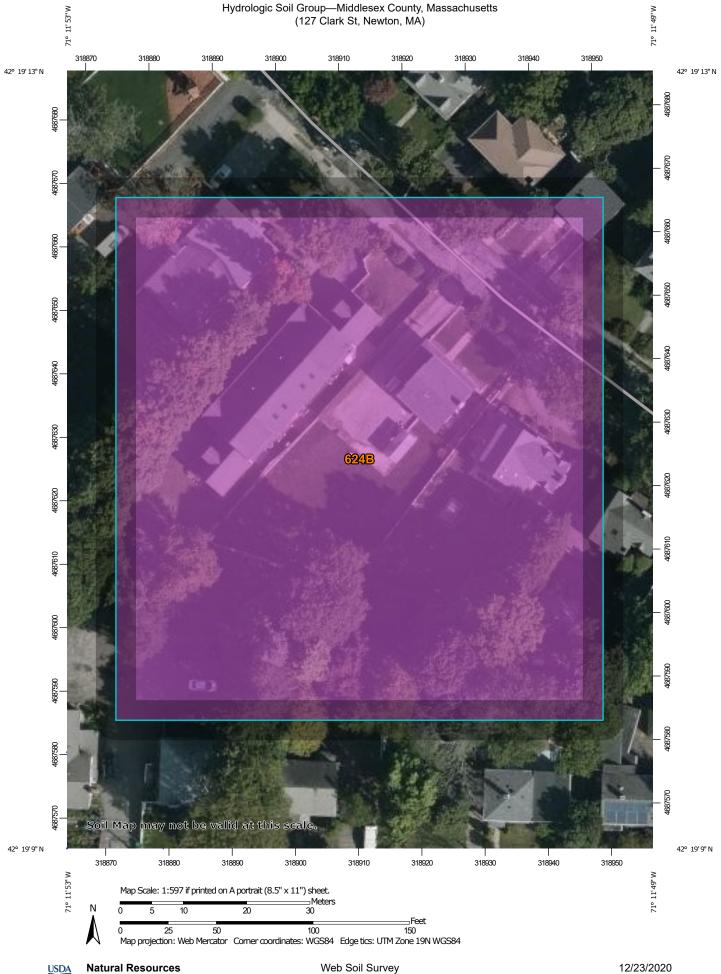
2019 Color Orthos (USGS) Tax Parcels for Query





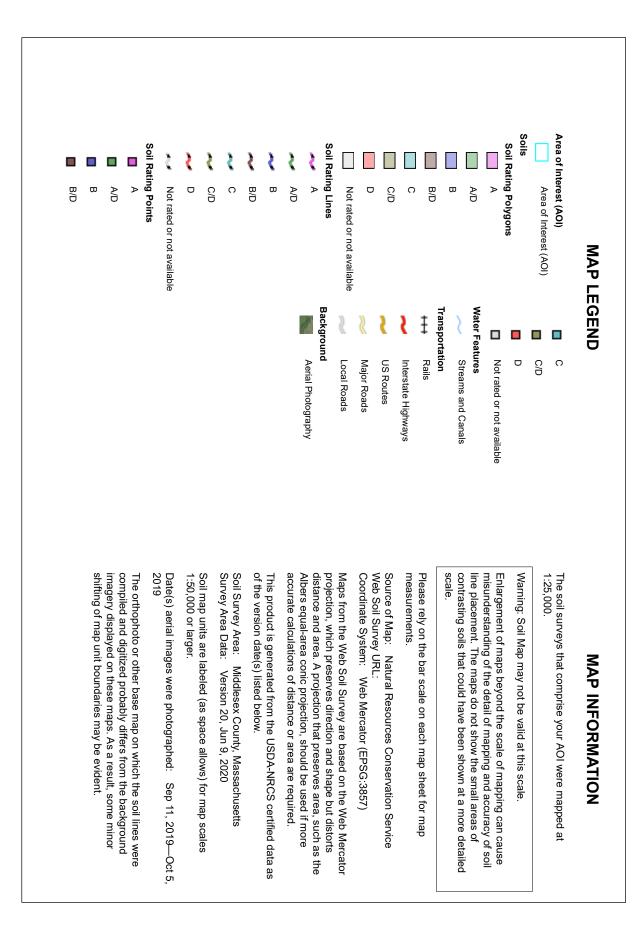


Appendix B NRCS Soils Information



Natural Resources Conservation Service Web Soil Survey National Cooperative Soil Survey





Natural Resources Conservation Service

### Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
624B	Haven-Urban land complex, 0 to 8 percent slopes	A	1.6	100.0%
Totals for Area of Interest		1.6	100.0%	

### Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

### **Rating Options**

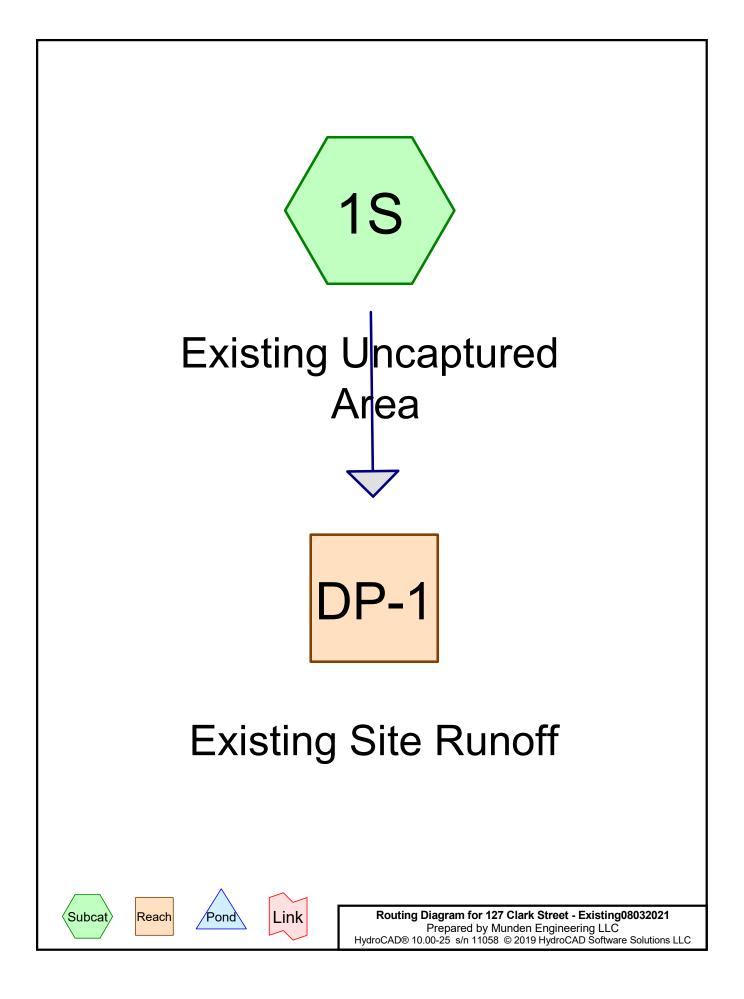
Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified Tie-break Rule: Higher





Appendix C Pre-Development Stormwater Calculations



**127 Clark Street - Existing08032021** Prepared by Munden Engineering LLC HydroCAD® 10.00-25 s/n 11058 © 2019 HydroCAD Software Solutions LLC

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#### Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
7,569	61	>75% Grass cover, Good, HSG B (1S)
168	98	Conc. Pad and Walkways, HSG B (1S)
1,591	98	Driveway, HSG B (1S)
163	96	Gravel surface, HSG B (1S)
1,123	98	Roofs and Covered Porch, HSG B (1S)
125	98	Shed, HSG B (1S)
91	98	Stone Ret. Walls, HSG B (1S)
10,830	72	TOTAL AREA

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#### Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
10,830	HSG B	1S
0	HSG C	
0	HSG D	
0	Other	
10,830		TOTAL AREA

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Ground Covers (all nodes)						
HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover
0	7,569	0	0	0	7,569	>75% Grass cover, Good
0	168	0	0	0	168	Conc. Pad and Walkways
0	1,591	0	0	0	1,591	Driveway
0	163	0	0	0	163	Gravel surface
0	1,123	0	0	0	1,123	Roofs and Covered Porch
0	125	0	0	0	125	Shed
0	91	0	0	0	91	Stone Ret. Walls
0	10,830	0	0	0	10,830	TOTAL AREA

### Ground Covers (all nodes)

127 Clark Street - Existing Type III 24-hr Design Storm Newton Rainfall=8.78"

127 Clark Street - Existing08032021

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Existing UncapturedRunoff Area=10,830 sf28.61% ImperviousRunoff Depth=5.39"Flow Length=174'Tc=7.5 minCN=72Runoff=1.47 cfs4,860 cf

**Reach DP-1: Existing Site Runoff** 

Inflow=1.47 cfs 4,860 cf Outflow=1.47 cfs 4,860 cf

Total Runoff Area = 10,830 sf Runoff Volume = 4,860 cf Average Runoff Depth = 5.39" 71.39% Pervious = 7,732 sf 28.61% Impervious = 3,098 sf **127 Clark Street - Existing08032021**Type III 24-hr Designering LLCPrepared by Munden Engineering LLCHydroCAD® 10.00-25 s/n 11058 © 2019 HydroCAD Software Solutions LLC

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#### Summary for Subcatchment 1S: Existing Uncaptured Area

Runoff = 1.47 cfs @ 12.11 hrs, Volume= 4,860 cf, Depth= 5.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr Design Storm Newton Rainfall=8.78"

	A	rea (sf)	CN [	Description			
*		125	98 3	Shed, HSG	В		
*		1,123	98 F	Roofs and (	Covered Po	orch, HSG B	
*		1,591	98 E	Driveway,	ISG B		
*		168	98 (	Conc. Pad a	and Walkw	ays, HSG B	
*		7,569	61 >	>75% Gras	s cover, Go	bod, HSG B	
*		91			Walls, HSG		
_		163	96 (	Gravel surfa	ace, HSG E	3	
		10,830		Veighted A			
		7,732	-		vious Area		
		3,098	28.61% Impervious Area				
	та	l e e este	Clana	Valasiti	Consolity	Description	
	Tc (min)	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	0.3	65	0.0769	3.58		Sheet Flow, Driveway	
	0.0	04	0.0400	0.04		Smooth surfaces n= 0.011 P2= 8.78"	
	0.2	24	0.0423	2.31		Sheet Flow, Driveway	
	0.1					Smooth surfaces n= 0.011 P2= 8.78"	
	0.1	95	0.0100	0.01		Direct Entry, Existing Roof	
	0.1 6.9	85	0.0100	0.21		Direct Entry, Existing Roof Sheet Flow, Backyard	
		85	0.0100 Total	0.21		Direct Entry, Existing Roof	

#### 127 Clark Street - Existing08032021

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## Hydrograph Type III 24-hr Design Storm Newton Rainfall=8.78" Runoff Area=10,830 sf Runoff Volume=4,860 cf Runoff Depth=5.39" Flow Length=174' Tc=7.5 min CN=72 0 2 4 6 8 10 12 14 16 18 20 22 42 62 80 30 23 43 68 840 42 44 46 84 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

#### Subcatchment 1S: Existing Uncaptured Area

**127 Clark Street - Existing08032021**Type III 24-hr Designering LLCPrepared by Munden Engineering LLCHydroCAD® 10.00-25 s/n 11058 © 2019 HydroCAD Software Solutions LLC

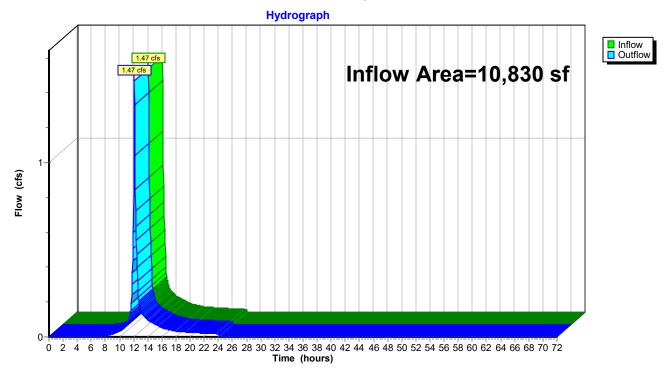
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#### Summary for Reach DP-1: Existing Site Runoff

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	ea =	10,830 sf, 28.61% Impervious,	Inflow Depth = 5.39" for Design Storm Newton event	
Inflow	=	1.47 cfs @ 12.11 hrs, Volume=	4,860 cf	
Outflow	=	1.47 cfs @ 12.11 hrs, Volume=	4,860 cf, Atten= 0%, Lag= 0.0 min	

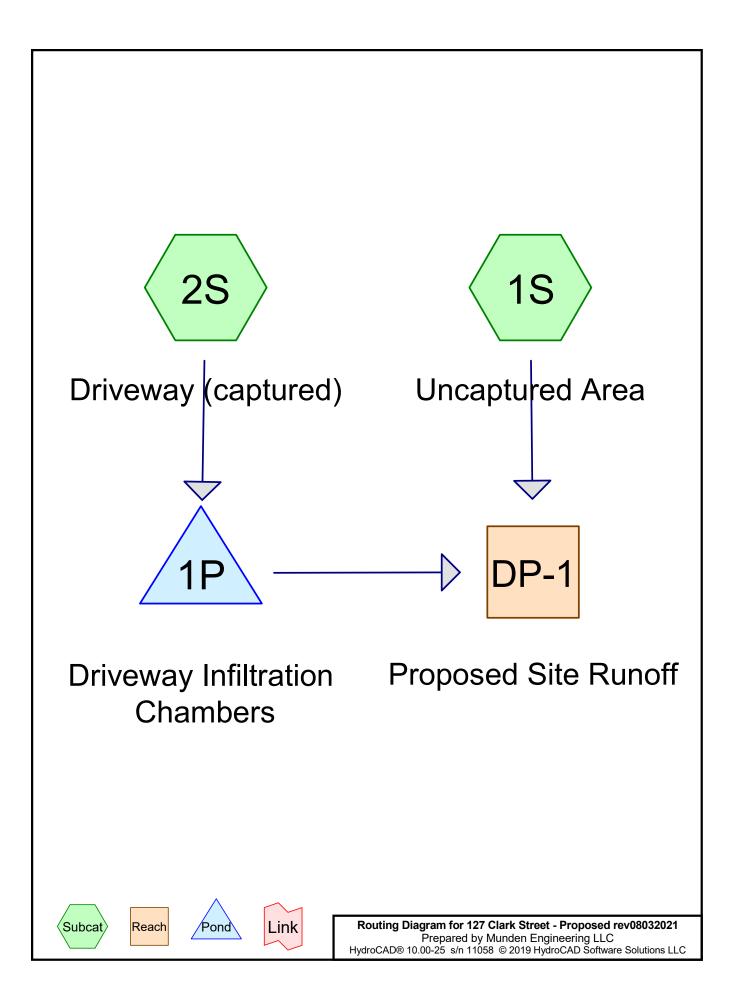
Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs



**Reach DP-1: Existing Site Runoff** 



Appendix D Post-Development Drainage Calculations



**127 Clark Street - Proposed rev08032021** Prepared by Munden Engineering LLC HydroCAD® 10.00-25 s/n 11058 © 2019 HydroCAD Software Solutions LLC

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## Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
5,600	61	>75% Grass cover, Good, HSG B (1S)
147	98	Conc. Pad and Walkways, HSG B (1S)
1,683	98	Driveway, HSG B (2S)
108	98	New RW, HSG B (1S)
400	98	New patio hardscape area, HSG B (1S)
18	98	New stone steps, HSG B (1S)
45	98	New walkway, HSG B (1S)
2,510	98	Roofs and Covered Porch (new roof overlaps exist. by 81 sf), HSG B (1S)
79	98	Stone Ret. Walls, HSG B (1S)
240	98	Stone patio under raised deck, HSG B (1S)
10,830	79	TOTAL AREA

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## Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
10,830	HSG B	1S, 2S
0	HSG C	
0	HSG D	
0	Other	
10,830		TOTAL AREA

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	Ground Covers (all nodes)									
HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground				
(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	Cover				
0	5,600	0	0	0	5,600	>75% Grass				
						cover, Good				
0	147	0	0	0	147	Conc. Pad and				
						Walkways				
0	1,683	0	0	0	1,683	Driveway				
0	108	0	0	0	108	New RW				
0	400	0	0	0	400	New patio				
						hardscape area				
0	18	0	0	0	18	New stone steps				
0	45	0	0	0	45	New walkway				
0	2,510	0	0	0	2,510	Roofs and				
						Covered Porch				
						(new roof				
						overlaps exist.				
						by 81 sf)				
0	79	0	0	0	79	Stone Ret. Walls				
0	240	0	0	0	240	Stone patio				
						under raised				
						deck				
0	10,830	0	0	0	10,830	TOTAL AREA				

# Ground Covers (all nodes)

127 Clark st - Proposed

**127 Clark Street - Proposed rev08032021** Type III 24-hr Design Storm Newton Rainfall=8.78"

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Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Uncaptured Area<br/>Flow Length=55'Runoff Area=9,147 sf 38.78% Impervious<br/>Slope=0.0100 '/' Tc=6.0 min CN=75<br/>Runoff=1.38 cfs 4,383 cfSubcatchment 2S: Driveway (captured)Runoff Area=1,683 sf 100.00% Impervious<br/>Flow Length=100' Tc=6.0 min CN=98<br/>Runoff=0.33 cfs 1,198 cfReach DP-1: Proposed Site RunoffInflow=1.38 cfs 4,383 cfPond 1P: Driveway Infiltration ChambersPeak Elev=126.69'<br/>Storage=0.016 af<br/>Outflow=0.01 cfs 1,198 cfTotal Runoff Area = 10,830 sfRunoff Volume = 5,581 cfAverage Runoff Depth = 6.18"

51.71% Pervious = 5,600 sf 48.29% Impervious = 5,230 sf

127 Clark st - Proposed

**127 Clark Street - Proposed rev08032021**Type III 24-hr Design Storm Newton Rainfall=8.78"Prepared by Munden Engineering LLCHydroCAD® 10.00-25 s/n 11058 © 2019 HydroCAD Software Solutions LLCPage 6

# Summary for Subcatchment 1S: Uncaptured Area

Runoff = 1.38 cfs @ 12.09 hrs, Volume= 4,383 cf, Depth= 5.75"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr Design Storm Newton Rainfall=8.78"

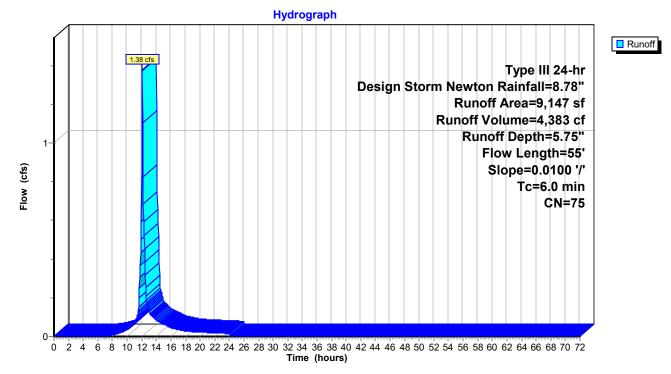
	Α	rea (sf)	CN	Description				
*		2,510	98	Roofs and (	Covered Po	orch (new roof overlaps ex	kist. by 81 sf), HSG B	
*		147	98	Conc. Pad a	and Walkw	ays, HSG B	- /	
*		5,600	61	>75% Gras	s cover, Go	od, HSG B		
*		79	98	Stone Ret.	Walls, HSG	Э В		
*		240	98	Stone patio	under raise	ed deck, HSG B		
*		18	98	New stone	steps, HSG	B		
*		45	98	New walkwa	ay, HSG B			
*		400	98	New patio h	nardscape a	area, HSG B		
*		108	98	New RW, F	ISG B			
		9,147	75	75 Weighted Average				
		5,600		61.22% Per	rvious Area			
		3,547	;	38.78% Imp	pervious Ar	ea		
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	0.1					Direct Entry, Roofs		
	4.8	55	0.0100	0.19		Sheet Flow, Backyard		
						Grass: Short n= 0.150	P2= 8.78"	
	4.9	55	Total,	Increased t	to minimum	Tc = 6.0 min		
			,					

127 Clark Street - Proposed rev08032021 Type III 24-hr Design Storm Newton Rainfall=8.78"

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## Subcatchment 1S: Uncaptured Area

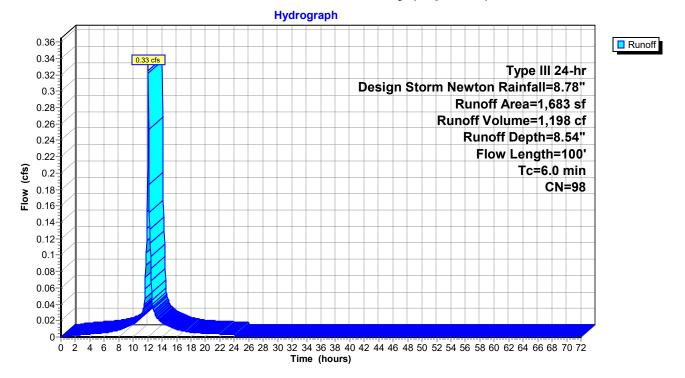
#### Summary for Subcatchment 2S: Driveway (captured)

Runoff = 0.33 cfs @ 12.09 hrs, Volume= 1,198 cf, Depth= 8.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr Design Storm Newton Rainfall=8.78"

	A	rea (sf)	CN E	Description		
*		1,683	98 E	Driveway,	ISG B	
		1,683	1	00.00% Im	npervious A	rea
(	Tc min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	0.3	65	0.0900	3.82	(0.0)	Sheet Flow, Driveway
	0.3	35	0.0200	1.85		Smooth surfaces n= 0.011 P2= 8.78" <b>Sheet Flow, Driveway</b> Smooth surfaces n= 0.011 P2= 8.78"
	0.6	100	Total, I	ncreased t	o minimum	Tc = 6.0 min

#### Subcatchment 2S: Driveway (captured)

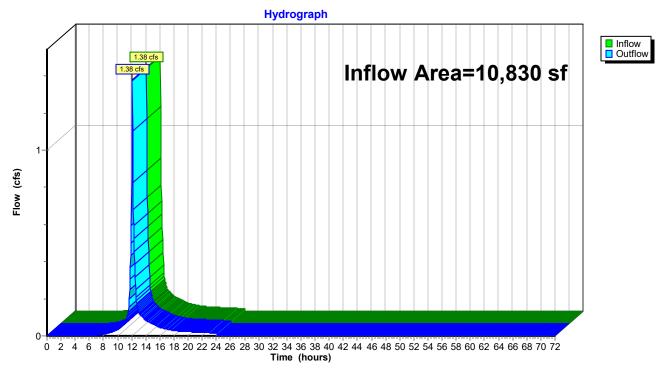


#### Summary for Reach DP-1: Proposed Site Runoff

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	a =	10,830 sf, 48.29% Impervious, I	nflow Depth = 4.86" for Design Storm Newton event
Inflow	=	1.38 cfs @ 12.09 hrs, Volume=	4,383 cf
Outflow	=	1.38 cfs @ 12.09 hrs, Volume=	4,383 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs



Reach DP-1: Proposed Site Runoff

127 Clark st - Proposed **127 Clark Street - Proposed rev08032021** *Type III 24-hr Design Storm Newton Rainfall=8.78"* Prepared by Munden Engineering LLC

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#### Summary for Pond 1P: Driveway Infiltration Chambers

Inflow Area =	1,683 sf,100.00% Impervious,	Inflow Depth = 8.54" for Design Storm Newton event
Inflow =	0.33 cfs @ 12.09 hrs, Volume=	1,198 cf
Outflow =	0.01 cfs @ 8.40 hrs, Volume=	1,198 cf, Atten= 97%, Lag= 0.0 min
Discarded =	0.01 cfs @ 8.40 hrs, Volume=	1,198 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 126.69' @ 16.00 hrs Surf.Area= 0.009 ac Storage= 0.016 af

Plug-Flow detention time= 625.9 min calculated for 1,198 cf (100% of inflow) Center-of-Mass det. time= 625.9 min (1,365.9 - 740.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	124.00'	0.008 af	10.33'W x 38.00'L x 3.21'H Field A
			0.029 af Overall - 0.010 af Embedded = 0.019 af x 40.0% Voids
#2A	124.50'	0.010 af	Cultec R-280HD x 10 Inside #1
			Effective Size= 46.9"W x 26.0"H => 6.07 sf x 7.00'L = 42.5 cf
			Overall Size= 47.0"W x 26.5"H x 8.00'L with 1.00' Overlap
			Row Length Adjustment= +1.00' x 6.07 sf x 2 rows
		0.018 of	Total Available Storage

0.018 af Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	124.00'	1.020 in/hr Exfiltration over Surface area

**Discarded OutFlow** Max=0.01 cfs @ 8.40 hrs HW=124.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

#### Pond 1P: Driveway Infiltration Chambers - Chamber Wizard Field A

#### Chamber Model = Cultec R-280HD (Cultec Recharger® 280HD)

Effective Size= 46.9"W x 26.0"H => 6.07 sf x 7.00'L = 42.5 cf Overall Size= 47.0"W x 26.5"H x 8.00'L with 1.00' Overlap Row Length Adjustment= +1.00' x 6.07 sf x 2 rows

47.0" Wide + 6.0" Spacing = 53.0" C-C Row Spacing

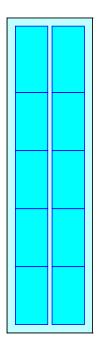
5 Chambers/Row x 7.00' Long +1.00' Row Adjustment = 36.00' Row Length +12.0" End Stone x 2 = 38.00' Base Length 2 Rows x 47.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 10.33' Base Width 6.0" Base + 26.5" Chamber Height + 6.0" Cover = 3.21' Field Height

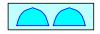
10 Chambers x 42.5 cf +1.00' Row Adjustment x 6.07 sf x 2 Rows = 437.2 cf Chamber Storage

1,259.8 cf Field - 437.2 cf Chambers = 822.6 cf Stone x 40.0% Voids = 329.1 cf Stone Storage

Chamber Storage + Stone Storage = 766.2 cf = 0.018 af Overall Storage Efficiency = 60.8% Overall System Size = 38.00' x 10.33' x 3.21'

10 Chambers 46.7 cy Field 30.5 cy Stone

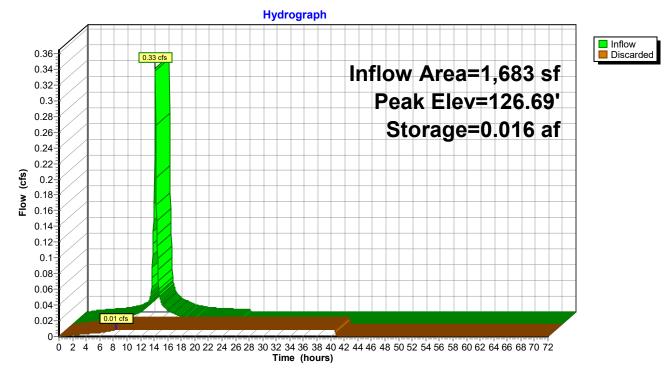




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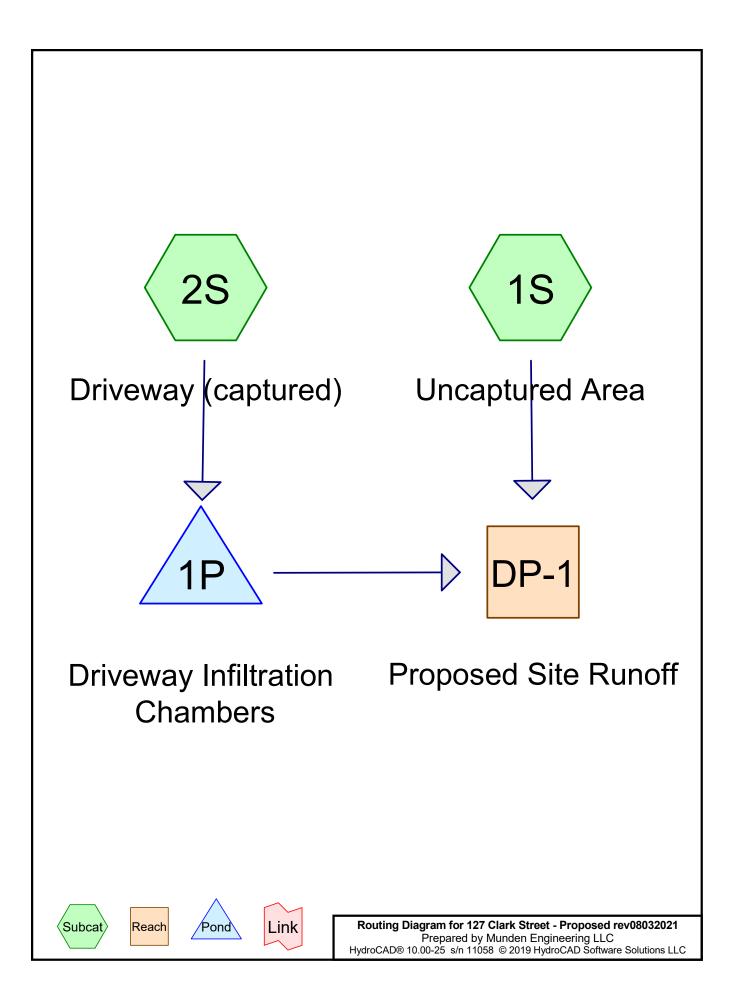
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## Pond 1P: Driveway Infiltration Chambers

Post-Construction Calculations with No Minimum Tc



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## Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
5,600	61	>75% Grass cover, Good, HSG B (1S)
147	98	Conc. Pad and Walkways, HSG B (1S)
1,683	98	Driveway, HSG B (2S)
108	98	New RW, HSG B (1S)
400	98	New patio hardscape area, HSG B (1S)
18	98	New stone steps, HSG B (1S)
45	98	New walkway, HSG B (1S)
2,510	98	Roofs and Covered Porch (new roof overlaps exist. by 81 sf), HSG B (1S)
79	98	Stone Ret. Walls, HSG B (1S)
240	98	Stone patio under raised deck, HSG B (1S)
10,830	79	TOTAL AREA

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### Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
10,830	HSG B	1S, 2S
0	HSG C	
0	HSG D	
0	Other	
10,830		TOTAL AREA

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	Ground Govers (an nodes)								
HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground			
(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	Cover			
0	5,600	0	0	0	5,600	>75% Grass			
						cover, Good			
0	147	0	0	0	147	Conc. Pad and			
						Walkways			
0	1,683	0	0	0	1,683	Driveway			
0	108	0	0	0	108	New RW			
0	400	0	0	0	400	New patio			
						hardscape area			
0	18	0	0	0	18	New stone steps			
0	45	0	0	0	45	New walkway			
0	2,510	0	0	0	2,510	Roofs and			
						Covered Porch			
						(new roof			
						overlaps exist.			
						by 81 sf)			
0	79	0	0	0	79	Stone Ret. Walls			
0	240	0	0	0	240	Stone patio			
						under raised			
						deck			
0	10,830	0	0	0	10,830	TOTAL AREA			

## Ground Covers (all nodes)

127 Clark St - Proposed No Min Tc

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127 Clark Street - Proposed rev08032021 Type III 24-hr Design Storm Newton Rainfall=8.78"

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> Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Uncaptured Area<br/>Flow Length=55'Runoff Area=9,147 sf 38.78% Impervious<br/>Slope=0.0100 '/' Tc=4.9 min CN=75<br/>Runoff=1.42 cfs 4,383 cfSubcatchment 2S: Driveway (captured)Runoff Area=1,683 sf 100.00% Impervious<br/>Flow Length=100' Tc=0.6 min CN=98<br/>Runoff=0.38 cfs 1,198 cfReach DP-1: Proposed Site RunoffInflow=1.42 cfs 4,383 cfPond 1P: Driveway Infiltration ChambersPeak Elev=126.69'<br/>Storage=0.016 af<br/>Outflow=0.01 cfs 1,198 cfTotal Runoff Area = 10,830 sfRunoff Volume = 5,581 cfAverage Runoff Depth = 6.18"

51.71% Pervious = 5,600 sf 48.29% Impervious = 5,230 sf

#### Summary for Subcatchment 1S: Uncaptured Area

[49] Hint: Tc<2dt may require smaller dt

Runoff = 1.42 cfs @ 12.07 hrs, Volume= 4,383 cf, Depth= 5.75"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr Design Storm Newton Rainfall=8.78"

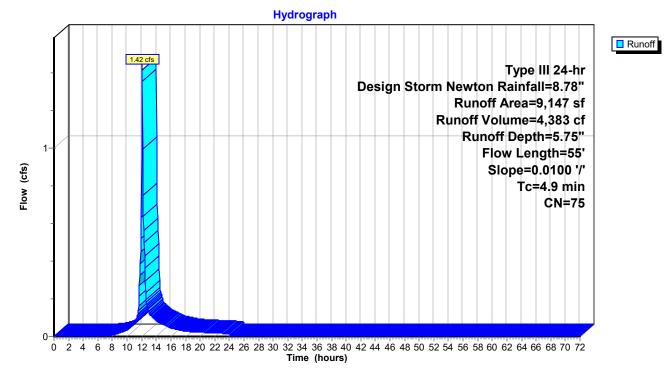
	A	rea (sf)	CN [	Description				
*		2,510	98 F	Roofs and Covered Porch (new roof overlaps exist. by 81 sf), HSG B				
*		147	98 (	Conc. Pad a	and Walkw	ays, HSG B		
*		5,600	61 >	>75% Gras	s cover, Go	bod, HSG B		
*		79	98 3	Stone Ret.	Walls, HSC	G B		
*		240	98 3	Stone patio	under raise	ed deck, HSG B		
*		18	98 I	New stone	steps, HSG	3 B		
*		45	98 I	New walkway, HSG B				
*		400	98 I	vew patio h	ardscape a	area, HSG B		
*		108	98 1	<mark>lew</mark> RW, F	ISG B			
		9,147	75 \	Veighted A	verage			
		5,600	6	61.22% Per	vious Area	l		
		3,547	3	38.78% Imp	pervious Ar	ea		
	Тс	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	0.1					Direct Entry, Roofs		
	4.8	55	0.0100	0.19		Sheet Flow, Backyard		
						Grass: Short n= 0.150 P2= 8.78"		
	4.9	55	Total					

127 Clark Street - Proposed rev08032021 Type III 24-hr Design Storm Newton Rainfall=8.78"

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### Subcatchment 1S: Uncaptured Area

#### Summary for Subcatchment 2S: Driveway (captured)

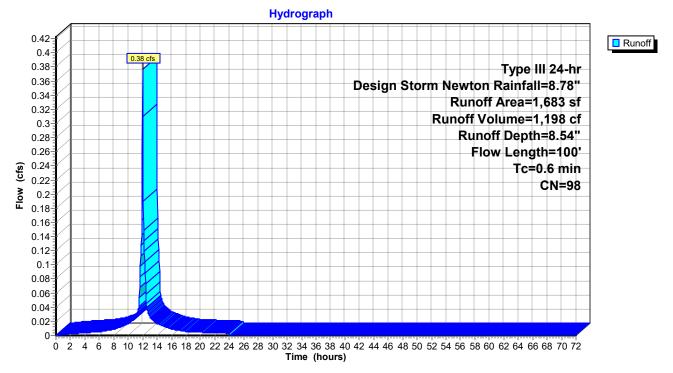
[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.38 cfs @ 12.01 hrs, Volume= 1,198 cf, Depth= 8.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type III 24-hr Design Storm Newton Rainfall=8.78"

_	A	rea (sf)	CN E	Description		
*		1,683	98 E	Driveway, H	ISG B	
		1,683	1	00.00% In	npervious A	rea
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	0.3	65	0.0900	3.82		Sheet Flow, Driveway
						Smooth surfaces n= 0.011 P2= 8.78"
	0.3	35	0.0200	1.85		Sheet Flow, Driveway
_						Smooth surfaces n= 0.011 P2= 8.78"
	0.6	100	Total			

#### Subcatchment 2S: Driveway (captured)

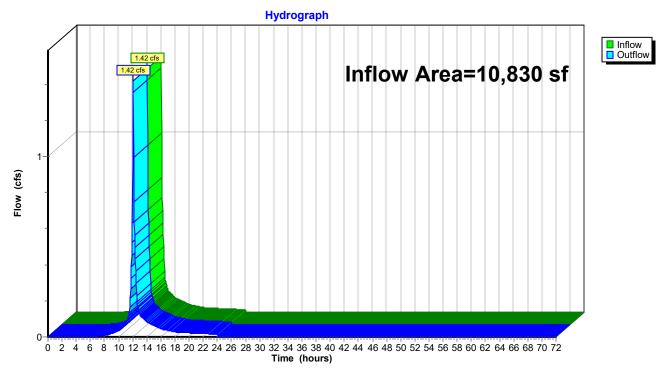


#### Summary for Reach DP-1: Proposed Site Runoff

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	a =	10,830 sf, 48.29% Impervious, Inflow Depth = 4.86" for Design Storm Newton event
Inflow	=	1.42 cfs @ 12.07 hrs, Volume= 4,383 cf
Outflow	=	1.42 cfs @ 12.07 hrs, Volume= 4,383 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs



**Reach DP-1: Proposed Site Runoff** 

#### Summary for Pond 1P: Driveway Infiltration Chambers

Inflow Area =	1,683 sf,100.00% Impervious,	Inflow Depth = 8.54" for Design Storm Newton event
Inflow =	0.38 cfs @ 12.01 hrs, Volume=	1,198 cf
Outflow =	0.01 cfs @ 8.35 hrs, Volume=	1,198 cf, Atten= 98%, Lag= 0.0 min
Discarded =	0.01 cfs $\overline{@}$ 8.35 hrs, Volume=	1,198 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 126.69' @ 15.91 hrs Surf.Area= 0.009 ac Storage= 0.016 af

Plug-Flow detention time= 625.6 min calculated for 1,197 cf (100% of inflow) Center-of-Mass det. time= 625.9 min (1,361.0 - 735.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	124.00'	0.008 af	10.33'W x 38.00'L x 3.21'H Field A
			0.029 af Overall - 0.010 af Embedded = 0.019 af x 40.0% Voids
#2A	124.50'	0.010 af	Cultec R-280HD x 10 Inside #1
			Effective Size= 46.9"W x 26.0"H => 6.07 sf x 7.00'L = 42.5 cf
			Overall Size= 47.0"W x 26.5"H x 8.00'L with 1.00' Overlap
			Row Length Adjustment= +1.00' x 6.07 sf x 2 rows
-		0.018 of	Total Available Storage

0.018 af Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	124.00'	1.020 in/hr Exfiltration over Surface area
<b>D</b> '		A	

**Discarded OutFlow** Max=0.01 cfs @ 8.35 hrs HW=124.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

#### Pond 1P: Driveway Infiltration Chambers - Chamber Wizard Field A

#### Chamber Model = Cultec R-280HD (Cultec Recharger® 280HD)

Effective Size= 46.9"W x 26.0"H => 6.07 sf x 7.00'L = 42.5 cf Overall Size= 47.0"W x 26.5"H x 8.00'L with 1.00' Overlap Row Length Adjustment= +1.00' x 6.07 sf x 2 rows

47.0" Wide + 6.0" Spacing = 53.0" C-C Row Spacing

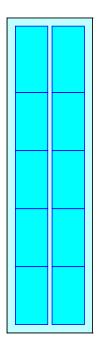
5 Chambers/Row x 7.00' Long +1.00' Row Adjustment = 36.00' Row Length +12.0" End Stone x 2 = 38.00' Base Length 2 Rows x 47.0" Wide + 6.0" Spacing x 1 + 12.0" Side Stone x 2 = 10.33' Base Width 6.0" Base + 26.5" Chamber Height + 6.0" Cover = 3.21' Field Height

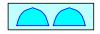
10 Chambers x 42.5 cf +1.00' Row Adjustment x 6.07 sf x 2 Rows = 437.2 cf Chamber Storage

1,259.8 cf Field - 437.2 cf Chambers = 822.6 cf Stone x 40.0% Voids = 329.1 cf Stone Storage

Chamber Storage + Stone Storage = 766.2 cf = 0.018 af Overall Storage Efficiency = 60.8% Overall System Size = 38.00' x 10.33' x 3.21'

10 Chambers 46.7 cy Field 30.5 cy Stone



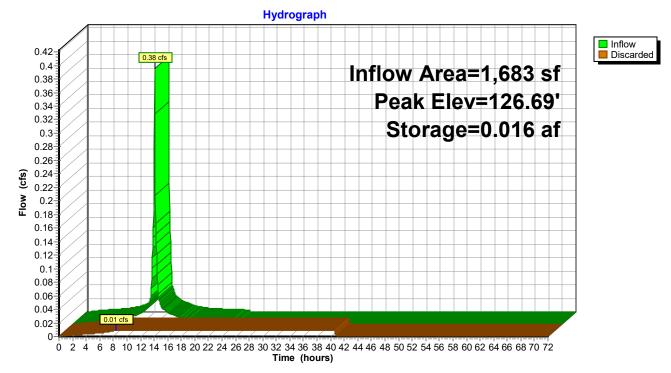


127 Clark St - Proposed No Min Tc **127 Clark Street - Proposed rev08032021** *Type III 24-hr Design Storm Newton Rainfall=8.78"* 

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### Pond 1P: Driveway Infiltration Chambers



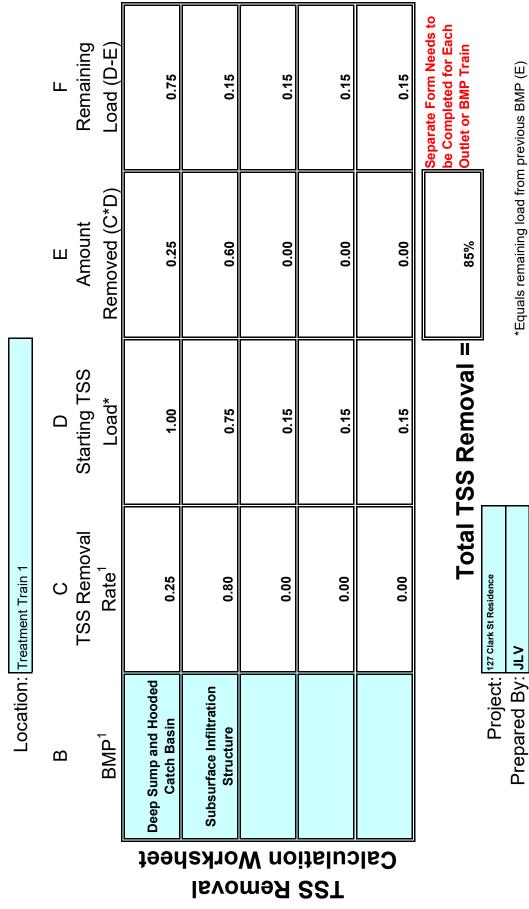
TSS Removal Worksheet

INSTRUCTIONS: 1. In BMP Column. click on Blue Cell to

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu

2. Select BMP from Drop Down Menu

3. After BMP is selected, TSS Removal and other Columns are automatically completed.



which enters the BMP

Date: 1/12/2021

1. From MassDEP Stormwater Handbook Vol. 1

must be used if Proprietary BMP Proposed

Non-automated TSS Calculation Sheet

Version 1, Automated: Mar. 4, 2008



Appendix E Construction Period Pollution Prevention Plan



mundenengineering.com info@mundenengineering.com 781-302-6099

# CONSTRUCTION PERIOD POLLUTION PREVENTION PLAN

#### 127 CLARK ST NEWTON MA

April 2021





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# Appendices

A Stormwater Construction Site Inspection Log



# **Section 1 Introduction**

Standard 8 of the Massachusetts Standards requires:

"A plan to control construction-related impacts including erosion, sedimentation and other pollutant sources during construction and land disturbance activities (construction period erosion, sedimentation, and pollution prevention plan) shall be developed and implemented".

The following Construction Period Pollution Prevention Plan (CPPPP) outlines the requirements to comply with Standard 8.

# **Section 2 Project Information**

# 2.1 **Responsible Parties**

This Construction Period Pollution Prevention Plan has been prepared for the construction phase activity for the redevelopment of a single-family dwelling located at 127 Clark St in Newton, Massachusetts. The property is owned by Tim and Lauren Fagerberg. During construction, the contractor will be responsible for pollution prevention and erosion and sediments controls as follows below.

# 2.2 General Description of Project

The project site activities include the redevelopment of a single-family dwelling located at 127 Clark St in the City of Newton, Massachusetts. The Site currently consists of a 2-story single family dwelling with a covered porch, driveway, brick walkways, and retaining walls. The proposed project includes the construction of a 2-story addition off the rear side of the existing dwelling and extending the existing driveway. Under proposed conditions, the total impervious surface will result in a net increase of 1,410 sq. ft.

The property is located on the West side of Clark Street and abuts residential properties to the North, West, and South. On the East side of the property, the lot has a 60-foot frontage along Clark Street.

# 2.3 Stormwater Management and Erosion Control Plan

A Stormwater Management and Erosion Control Plan (SMECP) is provided on sheet 5 of the Site Plans. The SMECP outlines the minimum requirements for the prevention of erosion and sedimentation due to construction impacts. The SMECP provides locations of the perimeter controls, anti-tracking pads, and inlet protection.

# **Section 3 Erosion and Sediment Controls**

The Contractor shall comply with the following temporary erosion and sediment controls to minimize the discharge of pollutants in stormwater from construction activities.

# **3.1** Perimeter Controls

<u>Description</u>: The erosion control barriers will consist of silt socks placed in a manner that restricts the contractor to the areas necessary to perform the work. The perimeter controls will generally define the limits of work.

<u>Installation</u>: The temporary erosion control measures shall be installed before construction begins at the site and around soil stockpiles once they have been established. Silt socks will be installed per the details shown on the plans and per manufacturer requirements. Perform work in accordance with the ESCP.

<u>Maintenance Requirements</u>: Perimeter controls will be inspected weekly and immediately after storm events to ensure it is intact and that there are no gaps where the fence meets the ground or tears along the length of the fence. If gaps or tears are found during the inspection, the fabric will be repaired or replaced immediately. Accumulated sediment will be removed from the base if it reaches one-third the height of the silt sock and properly disposed off-site. If accumulated sediment is creating noticeable strain on the fabric and might fail from a sudden storm event, the sediment will be removed more frequently. Before the silt socks are removed from the project area, the sediment will be removed. The erosion control barriers will be removed and properly disposed offsite following the stabilization of disturbed areas. The silt socks will likely need to be replaced is the construction extends over 6 months.

# **3.2** Inlet Protection

<u>Description:</u> Storm drain inlets will be protected from sediment by commercially available catch basin inserts as shown on the plan set for the proposed development. The catch basin inserts will be removed once the construction site has been permanently stabilized.

<u>Installation</u>: Inserts should be installed at all catch basins prior to construction or immediately after catch basins have been constructed until project completion.

<u>Maintenance Requirements:</u> The catch basin inserts will be inspected weekly and immediately after storm events. If the basin insert becomes clogged with sediment, the insert will be removed and cleaned or replaced per the manufacturer's recommendations.

# **Section 4 Pollution Prevention**

A typical construction site generates pollutants through construction activities. The following identifies preventative measures to reduce the opportunity for pollutants to enter the stormwater runoff stream.

# 4.1 Waste Management

<u>Description</u>: All waste materials will be collected and disposed of into one metal trash dumpster on site. Only trash and construction debris from the site will be deposited in the dumpster. No construction materials will be buried on-site. All personnel will be instructed, during tailgate training sessions, regarding the correct disposal of trash and construction debris. Notices that state these practices will be posted on site and the individual who manages day-today site operations will be responsible for seeing that these practices are followed.

Installation: Trash dumpsters will be installed prior to construction.

<u>Maintenance Requirements</u>: The dumpsters will be inspected weekly and immediately after storm events. The dumpster will be emptied weekly and taken to a landfill. If trash and construction debris are exceeding the dumpster's capacity, the dumpsters will be emptied more frequently.



# 4.1.2 Hazardous or Toxic Waste

Hazardous waste materials such as oil filters, petroleum products, paint, and equipment maintenance fluids shall not be placed in the dumpster and disposed of daily accordingly with local, state and federal regulations.



Appendix A Stormwater Construction Site Inspection Log

# Stormwater Construction Site Inspection Form

	General Information						
Project Name							
Project Location							
Date of Inspection	Start/End Time						
Inspector's Name(s)							
Inspector's Title(s)							
Inspector's Contact Information							
Describe present phase of construction							
Type of Inspection:Image: RegularImage: Pre-storm	event 🗆 During storm event 🗆 Post-storm event						
	Weather Information						
Amount of rainfall since last	inspection (inches):						
Weather at time of this inspe □ Clear □ Cloudy □ Rair							
□ Other:	Temperature:						
Have any discharges occurre If yes, describe:	d since the last inspection?   Yes  No						
Are there any discharges at t If yes, describe:	Are there any discharges at the time of inspection?  Yes No If yes, describe:						
Field Observations							
Description of Work Accompl	ished:						

#### Site-specific BMPs

	ВМР	BMP Installed or Required	BMP Maintenance Required?	Corrective Action Needed and Notes
1	Perimeter Controls	□Yes □No	□Yes □No	
2	Sediment track out	□Yes □No	□Yes □No	
3	Inlet protection	□Yes □No	□Yes □No	

# Stormwater Construction Site Inspection Form

0	verall Site Issues			
	BMP/activity	Implemented?	Maintenance	Corrective Action Needed and Notes
			Required?	
1	Are all slopes and disturbed areas not actively being worked properly stabilized?	□Yes □No	□Yes □No	
2	Are natural resource areas (e.g., streams, wetlands, mature trees, etc.) protected with barriers or similar BMPs?	□Yes □No	□Yes □No	
3	Are perimeter controls and sediment barriers adequately installed (keyed into substrate) and maintained?	□Yes □No	□Yes □No	
4	Are discharge points and receiving waters free of any sediment deposits?	□Yes □No	□Yes □No	
5	Is the construction sediment track out procedures preventing sediment from being tracked into the street?	□Yes □No	□Yes □No	
6	Are temporary stockpiles on site which will remain or have remained for more than 7 days have erosion controls?	□Yes □No	□Yes □No	
7	Are dust control measures being utilized as to prevent the migration of dust from the site and are the effective?	□Yes □No	□Yes □No	
8	Have areas adjacent to the site work been disturbed, which has resulted in disruption of topsoil outside of the limits of work?	□Yes □No	□Yes □No	
9	Is trash/litter from work areas	□Yes □No	□Yes □No	

# **Stormwater Construction Site Inspection Form**

				1
	collected and placed			
	in covered			
	dumpsters?			
10	Are washout	□Yes □No	□Yes □No	
	facilities (e.g.,			
	paint, stucco,			
	concrete) available,			
	clearly marked, and			
	maintained?			
11	Are vehicle and	□Yes □No	□Yes □No	
	equipment fueling,			
	cleaning, and			
	maintenance areas			
	free of spills, leaks,			
	or any other			
	deleterious			
10	material?			
12	Are materials that	□Yes □No	□Yes □No	
	are potential			
	stormwater			
	contaminants			
	stored inside or			
12	under cover?			
13	Are non-stormwater	□Yes □No	□Yes □No	
	discharges (e.g.,			
	wash water,			
	dewatering)			
	properly controlled?	l		

#### Non-Compliance

Describe any incidents of non-compliance not described above:

#### **CERTIFICATION STATEMENT**

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."

#### Print name and title: \_\_\_\_\_

Signature: \_\_\_\_\_

Date: \_\_\_\_\_



Appendix F Long-Term Operation and Maintenance Plan



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#### LONG-TERM OPERATION AND MAINTENANCE PLAN

#### 127 CLARK ST NEWTON MA

April 2021





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# **Section 1 Introduction and Purpose**

The following Long-Term Operation and Maintenance Plan (O&M) Plan has been prepared for the stormwater management system for the proposed redevelopment project located at 127 Clark St in the City of Newton, Massachusetts. The purpose of this plan is to provide guidance and procedures for the maintenance of the stormwater management system following construction.

The proposed project has been designed in compliance with the Massachusetts Department of Environmental Protection (MassDEP) Stormwater Standard and the City of Newton Stormwater Regulations to maintain or improve stormwater runoff quality and quantity. The stormwater management system components shall be maintained as recommended in the Massachusetts Stormwater Handbook.

# **Section 2 Responsible Parties**

The property owners, Tim and Lauren Fagerberg, are responsible for maintaining and servicing the proposed design and stormwater management system post construction. During construction, the contractor will be responsible for stormwater management maintenance.

# Section 3 Stormwater Management System

The on-site stormwater management system is comprised of a deep-sump hooded catch basin, conveyance piping and a series of eight underground infiltration chambers. In general, the majority of runoff from the proposed project area, including rooftops, paved driveway and unpaved areas, is conveyed to the backyard either by sheet flow or through the stormwater management system. A minimal amount of discharge from the paved driveway is conveyed to the public road by sheet flow.

# 3.1 Permanent Erosion Control Measures

Permanent stabilization will be done immediately after the final design grades are achieved, but no later than 14 days after construction ceases. All disturbed areas within the limit of work will be reseeded with grass. Portions of the site where construction activities have permanently ceased will be stabilized, as soon as possible but no later than 14 days after construction ceases.

All seeded areas will be inspected weekly during construction activities for failure and after storm events until a dense cover of vegetation has been established. If failure is noticed at the seeded area, the area will be reseeded, fertilized, and mulched immediately. After construction is completed at the site, permanently stabilized areas will be monitored until final stabilization is reached.

# 3.2 Inspections

Inspections will be performed in accordance with the Massachusetts Department of Environmental Protection (MassDEP) Stormwater Handbook. The Site Plans identify the location of each BMP to be inspected and maintained as described in this Section.

The following stormwater management system features will be evaluated during each inspection:

#### **3.2.1** Vegetated Surfaces

Inspection Frequency: Bi-annually in Summer and Winter

#### Special Inspection Event(s): Spring Snow Melt

All vegetative surfaces will be observed to identify locations of settlement, erosion and other impacts from the proposed redevelopment.

### **3.2.2** Stormwater Infiltration Chambers

#### Inspection Frequency: Every 3 months

#### Special Inspection Event(s): After every major storm event

Subsurface infiltration management systems should be inspected quarterly for sediment accumulation, blockages, structural deficiencies, and any other defects that may impair their intended function. The systems should be inspected via the inspection ports provided and should include the inlet and outlet to the system. Quarterly inspections should include one inspection following each the foliage and snow removal seasons.

In addition, Cultec Stormwater Chambers should be inspected 2 years after commissioning, and every 9 years following per manufacturers recommendations. Inspect the interior of the stormwater management chambers through the inspection port for deficiencies using CCTV or comparable technique. If any debris is present, clean stormwater management chambers and feed connectors. Maintenance of the stormwater management system shall be performed by qualified workers and shall follow applicable occupational health and safety requirements. Debris removed from the stormwater management system shall be disposed of in accordance with applicable laws and regulations.

The stormwater infiltration chambers must be inspected 48-hours after every major storm for signs of blockage and excess sedimentation.

# 3.2.3 Deep Sump Catch Basin

Inspection Frequency: Every 3 months

Special Inspection Event(s): After every major storm event

Deep sump catch basins must be cleaned out and sediment must be removed every 6 months or as required. Typically, a catch basin should be cleaned if the depth of the deposits is equal to ir greater than 3 ft. (one-half of the sump depth) from the invert.



# Section 4 Operation and Maintenance Log Form

Date:	
Person conducting Inspection:	
Reason for Inspection (Routine / Significant Rainfall):	

## Stormwater Management System Components:

## **Vegetated Surface**

Component inspected during this inspection

Any Repair Necessary\_\_\_\_\_

Other Comments\_\_\_\_\_

#### **Stormwater Infiltration Chambers**

Component inspected during this inspection

Any Repair Necessary\_\_\_\_\_

Other Comments\_\_\_\_\_

### **Deep Sump Catch Basin**

Component inspected during this inspection

Any Repair Necessary\_\_\_\_\_



Other Comments\_\_\_\_\_